

## Alabama Statewide Airport Pavement Management Program Update

Ozark Airport – Blackwell Field (71J)

**Final Report** 

February 2022









Submitted to

Alabama Aeronautics Bureau

Submitted by





Pavement Management - Evaluation - Testing - Design

# ALABAMA STATEWIDE AIRPORT PAVEMENT MANAGEMENT PROGRAM UPDATE

Ozark Airport – Blackwell Field, Ozark (71J)

**FINAL REPORT** 

Prepared For:

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February 2022



## **Executive Summary**

The Jviation Inc. team, which included All About Pavements, Inc., (API) was awarded a contract by the Alabama Department of Transportation's Aeronautics Bureau (ALDOT) in 2018 to update the existing Alabama Statewide Airport Pavement Management Program (APMP). The scope of this project includes the airside pavement network at Ozark Airport – Blackwell Field (71J).

The following APMP tasks were completed to achieve the project objectives at 71J:

- Update the PAVER work history with records review information provided by ALDOT
- Conduct a visual pavement condition survey of the airfield pavements
- Update the PAVER database with inventory and condition data
- Update Maintenance and Rehabilitation (M&R) policies and unit costs
- > Develop a 7-Year Pavement Capital Improvement Program (PCIP) with associated cost estimates

#### **ES.1 Pavement Inventory**

There are 15 branches and 17 sections within 71J's pavement network with a total surface area of approximately 1.12 million square feet (sf). Figure ES-1 shows the distribution of the pavement network by surface type and branch use.

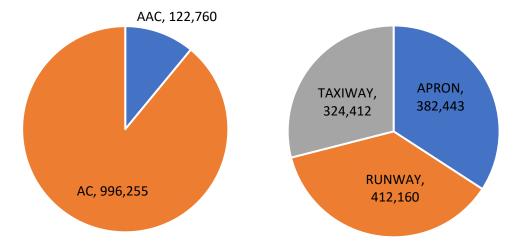


Figure ES-1: Pavement Area (sf) by Surface Type and Branch Use.

#### **ES.2 Pavement Condition**

Visual pavement inspections were conducted in November 2019 using the Pavement Condition Index (PCI) method as specified in ASTM D5340-12 and FAA AC 150/5380-6C. The PCI is a numerical rating scale from 0 to 100 that provides a measure of the pavement's functional surface condition. The overall

API



area-weighted network PCI (AW PCI) for the 71J pavement network is 72, representing a "Satisfactory" condition. The network area-weighted pavement age (AW Age) is 20 years.

Table ES-1 is a listing of the section PCI values and ratings.

Table ES-1: 71J Section PCI Values and Ratings.

Branch ID	Name	Section ID	Surface	Area, sf	PCI	PCI Category
A01	Apron 01	01	AC	78,123	66	Fair
A01	Apron 01	02	AC	53,187	22	Serious
A02	Apron 02	01	AC	48,373	62	Fair
A03	Apron 03	01	AC	80,000	94	Good
A04	Apron 04	01	AAC	122,760	68	Fair
R1331	Runway 13-31	01	AC	412,160	73	Satisfactory
TA	Taxiway A	01	AC	188,480	78	Satisfactory
TA1	Taxiway A1	01	AC	11,139	78	Satisfactory
TA2	Taxiway A2	01	AC	10,910	79	Satisfactory
ТВ	Taxiway B	01	AC	41,232	92	Good
TC	Taxiway C	01	AC	7,359	71	Satisfactory
TC01	Taxiway Connector 01	01	AC	2,903	79	Satisfactory
TC02	Taxiway Connector 02	01	AC	6,215	38	Very Poor
TC03	Taxiway Connector 03	01	AC	6,819	16	Serious
THANG01	Taxiway Hangar 01	01	AC	9,098	68	Fair
THANG01	Taxiway Hangar 01	02	AC	9,184	26	Very Poor
THANG02	Taxiway Hangar 02	01	AC	31,073	100	Good

## ES.3 Pavement Maintenance and Repair Funding Levels

The PAVER database was updated with 2019 condition data, maintenance and repair (M&R) policies, and unit costs; which were then used to evaluate the effect of multiple funding levels on the overall future pavement condition. Figure ES-2 presents the forecasted 71J network PCI values for each funding level.

#### ES.4 Pavement Capital Improvement Program (PCIP)

The analysis output from the unlimited funding budget scenario was used as a starting point in developing the PCIP. For this scenario, sections were grouped into projects to allow for a logical construction sequence. Table ES-2 summarizes the 7-year PCIP, which has an estimated total cost of approximately \$3.3 million. These recommendations are based on a network-level evaluation. Project-level evaluations should be conducted prior to developing design and bid package documents.



In addition to the major rehabilitation needs that are identified in the PCIP, PAVER was used to develop maintenance activities to repair specific PCI distresses in Year 1. The estimated costs for these maintenance activities are \$25,498 as summarized in Table ES-3.

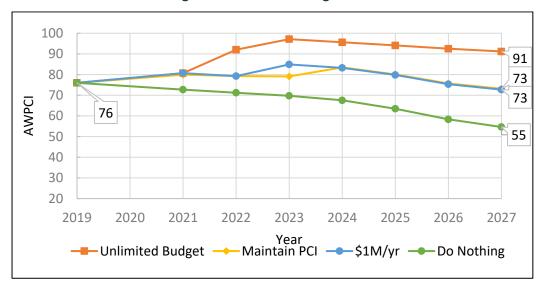


Figure ES-2: M&R Funding Levels.

Table ES-2: Summary of Pavement Capital Improvement Program.

Project Year	CIP Project	Total Project Cost	Total Project Area, sf	AWPCI Before	AWPCI After
2021	71J_21-01_Taxiway A Preservation	\$187,090	213,432	75	81
2022	71J_22-01_Runway 13-31 Rehabilitation	\$1,737,720	419,519	70	100
2022	71J_22-02_Apron 02 Rehabilitation	\$200,369	48,373	56	100
2023	71J_23-01_Taxiway B and Apron 03 Preservation	\$112,741	121,232	85	92
2024	71J_24-01_Apron 04 Rehabilitation	\$674,133	141,042	54	100
	71J_25-01_Apron 01 Rehabilitation	See Note	144,344	33	100
2025	71J_25-02_Runway 13-31 Surface Treatment	\$275,005	419,519	96	99
	71J_25-03_Apron 02 Surface Treatment	\$31,710	48,373	93	98
2027	71J_27-01_Taxiway Hangar 02 Preservation	\$32,524	31,073	85	93
2027	71J_27-02_Apron 04 Surface Treatment	\$85,373	122,760	93	98
	Total	\$3,336,665			

Cost excluded from PCIP as directed by ALDOT

Table ES-3: Summary of Localized Maintenance Plan.

Policy	Work Description	Work Quantity	Work Unit	Work Cost
Preventive	Crack Sealing - AC	6,414	Ft	\$25,336
	Patching - AC Partial-Depth	10	SqFt	\$163
			Total	\$25,498



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F1: Section Forecasted Pavement Condition Rating

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• Final Report in PDF format

Geo-referenced Field Photos



#### 1 Introduction

#### 1.1. Overview

The Alabama Department of Transportation's Aeronautics Bureau (ALDOT) is responsible for preserving and enhancing Alabama's air transportation system, which consists of 74 general aviation airports throughout the State. ALDOT implemented an Airport Pavement Management Program (APMP) in 2008 using the PAVER system. ALDOT awarded a project in 2018 to Jviation Inc. (Jviation) to update the System Plan and conduct an Economic Analysis for the Alabama airports. The scope of work also included an update of the APMP for 59 general aviation airports, which was conducted by All About Pavements, Inc., (API), a Jviation team member.

With this update of the APMP, the Alabama airports continue to be eligible for FAA funding for major pavement rehabilitation work under the Airport Improvement Program (AIP) since an APMP meets the pavement maintenance management requirements described in Appendix A of AC 150/5380-6C.

This report discusses the evaluation of the airside pavements at Ozark Airport – Blackwell Field (71J), the current and forecasted pavement condition, and the development of the Pavement Capital Improvement Program (PCIP).

## 1.2. Work Scope

The goals of the Alabama Statewide Airport Pavement Management Update program are as follows:

- Conduct a visual pavement inspection of the asphalt surfaced pavements for 59 of the 74 general aviation airports in Alabama.
- Based on the visual inspection analysis results, develop a 7-year PCIP for each airport.

The scope of work is as shown below:

- Conduct a Records Review
- Update Pavement Network Definition
- Conduct Pavement Condition Surveys
- Update and customize existing APMP PAVER database
- Develop PCIP and associated project cost estimates
- Prepare Draft and Final Reports
- Develop a web-based viewer for reporting APMP data

As required in the Scope of Work, a detailed pavement condition survey was not conducted for any Portland Cement Concrete (PCC) aprons and PCC taxiways longer than 2,000 ft. Instead, a condition rating of "Good", "Fair", or "Poor" was assigned based on the overall pavement condition.

The deliverable products include a PAVER 7.0 database, individual airport evaluation reports, a statewide summary report, and the web viewer. The 71J report will be one of the 59 individual airport reports that will be available on ALDOT's website.





#### 1.3. Pavement Management Concept

An APMP provides an integrated framework for comprehensive evaluation and decision making for managing airfield pavements. The essential components of an effective APMP provide for an objective evaluation of the condition of existing pavements, identification of short-term and long-range major rehabilitation work, necessary improvements in the pavement structural capacity, and the recurring maintenance work that should be completed each year. The APMP will also provide a budget for each of these types of pavement construction.

Historically, most organizations have made maintenance decisions based on past experience, without the benefit of documented data or analysis. This practice does not encourage life cycle cost analysis, nor the evaluation of cost effectiveness of alternate scenarios, and can lead to the inefficient use of funds. With limited allocated funding for Maintenance and Repair (M&R) Program projects, a defined procedure for setting priorities and schedules that will maximize the funds available is more important than ever.

In examining the lifespan of a 20-year pavement, a "Good" to "Fair" condition rating may last only 5 to 15 years. After that point, the rate of deterioration of pavements accelerates sharply as the age of the pavement increases, and within five years, the pavement may deteriorate to the point of failure. In order to extend pavement life, maintenance and repairs need to be scheduled and performed before the pavement surface declines to a "fair" condition. The point at which rehabilitation can be done before the steep decline occurs is called the "critical PCI", and is generally considered to occur when the Pavement Condition Index (PCI) is between 60 and 70 for general aviation airports. If the work is done before deterioration accelerates, the cost of rehabilitation can be reduced as shown in Figure 1.1.

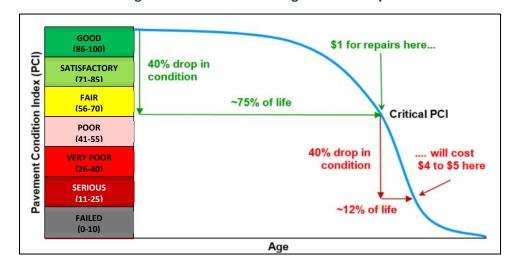


Figure 1.1: Pavement Management Concept.



## 2 Airfield Pavement Inventory

#### 2.1. Introduction

71J is a General Aviation (GA) airport located approximately 2 miles south east of Ozark. The airport was activated in June 1956 and is owned and operated by the Town of Ozark. Figure 2.1 shows an aerial image of the airport.



Figure 2.1: Ozark Airport – Blackwell Field.

(Source: Google Earth)

## 2.2. Pavement Inventory

71J consists of one runway, a parallel taxiway, two connector taxiways, and multiple aprons. The total pavement area is approximately 1.12 million square feet. Pavement surfaces at 71J include Asphalt Concrete (AC) and Asphalt Overlay on AC (AAC). A complete listing of the pavement sections is included in Appendix A. Runway 13-31 is 5,152 ft. long and 80 ft. wide.

A records search was undertaken to identify any preservation or rehabilitation work that has occurred at 71J since the last APMP update in 2009. The following records that were provided by ALDOT were reviewed, and the PAVER database was updated with work history information:

- > Taxiway Construction, 2015
- > Apron Construction, 2016
- Runway 13-31 Crackseal, 2018

#### 2.3. Climatic Conditions

Table 3.1 provides a summary of the climatic data for the geographic region that includes 71J. As the table shows, the pavements at 71J are not exposed to any freeze-thaw cycles. The mean air





temperature for January ranges from an average low of 39 degrees °F to an average high of 60 degrees °F. The average annual rainfall at 71J is near 56 inches.

Table 2.1: Average Annual Temperatures and Rainfall for 71J.

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
High Temp (°F)	60	65	72	78	84	90	91	91	87	79	70	62
Low Temp (°F)	39	42	48	54	62	68	70	70	66	56	48	42
Precip. (in)	5.9	5	6.9	4.1	3.6	4.8	5.8	3.6	4.5	2.7	4.7	4.7

Source: www.intellicast.com

#### 2.4. Pavement Network Definition

A key element in developing an APMP system is defining the pavement network, which is the process of dividing an agency's pavements into a hierarchical order that facilitates inspection and M&R planning. The 71J network (e.g. all airside pavements) is then divided into branches, which are a readily identifiable part of the pavement system and have distinct functions. For airports, branches typically consist of individual runways, taxiways and aprons. Figure B1A in Appendix B shows the branches at 71J.

Once branches have been defined, pavement evaluation and analysis techniques require the airfield pavement system to be broken up into discrete sections. A pavement "section" is the smallest management unit that is used when considering the application and selection of maintenance and rehabilitation (M&R) treatments, and is defined in Section 2.1.8 of ASTM D 5340-12 as "a contiguous pavement area having uniform construction, maintenance, usage history, and condition. A section should also have the same traffic volume and load intensity." A complete list of the pavement inventory and the corresponding section designations are included in Appendix A. Figure B1B presents the section layout.

To facilitate the visual survey of the airside pavement, each section is further subdivided into conveniently defined sub-section areas, or sample units. Similar sizing is critical as studies have found that maintaining the size of the sample units to within 40 percent of the established norm may reduce the standard error of the average PCI values. To meet that criteria, ASTM recommends that sample units for asphalt pavements be 5,000 square feet (± 2,000).

Table 2.2 was used as a guideline in developing sampling rates that reflect typical rates that are used for other large pavement networks. In general, this sampling rate will not provide a 95% confidence level with a standard error of 5 PCI points. A higher level of sampling is recommended before a project-level rehabilitation design is developed for a pavement section or facility.

Sample units that include a one-time occurrence of a distress (i.e. a large patch) or an unusual severity or quantity of a distress seen elsewhere, were designated as "additional" sample units as described in



the ASTM D5340 PCI procedure. This allows the PCI to be calculated without extrapolating the aberrant distress throughout the section as a whole. In Appendix B, Figure B1C shows the sample unit layout for 71J.

**Table 2.2: PCI Sampling Rate for AC Surfaces.** 

Total Samples	Samples to Inspect
1	1
2	2
3 – 6	3
7 – 13	4
14 – 39	5
> 39	15 percent, but less than 12

## 2.5. Inventory Summary

There are 15 branches (facilities) at 71J that include 17 pavement sections and a total area of approximately 1.12 million square feet of paved surfaces, as shown in Table 2.3.

Table 2.3: 71J Pavement Branches.

Branch ID	Branch Name	Branch Use	Area, sf	Number of Sections
A01	Apron 01	APRON	131,310	2
A02	Apron 02	APRON	48,373	1
A03	Apron 03	APRON	80,000	1
A04	Apron 04	APRON	122,760	1
R1331	Runway 13-31	RUNWAY	412,160	1
TA	Taxiway A	TAXIWAY	188,480	1
TA1	Taxiway A1	TAXIWAY	11,139	1
TA2	Taxiway A2	TAXIWAY	10,910	1
ТВ	Taxiway B	TAXIWAY	41,232	1
TC	Taxiway C	TAXIWAY	7,359	1
TC01	Taxiway Connector 01	TAXIWAY	2,903	1
TC02	Taxiway Connector 02	TAXIWAY	6,215	1
TC03	Taxiway Connector 03	TAXIWAY	6,819	1
THANG01	Taxiway Hangar 01	TAXIWAY	18,282	2
THANG02	Taxiway Hangar 02	TAXIWAY	31,073	1
		Total	1,119,015	17

Table 2.4 shows the distribution of airfield pavement by age with the area-weighted age being 20 years for all airside pavements at 71J.





Table 2.4: 71J Pavement Age.

Age (Years)	Number of Sections	Percent of Area	Area, sf
0 – 5	3	14	152,305
6 – 10	0	0	0
11 – 15	6	73	823,572
16 – 20	0	0	0
> 20	8	13	143,138

Figure 2.2 shows the distribution by surface type. Figure 2.3 presents the distribution by pavement use (e.g. runway, taxiway, and apron).

1,000,000 800,000 400,000 200,000

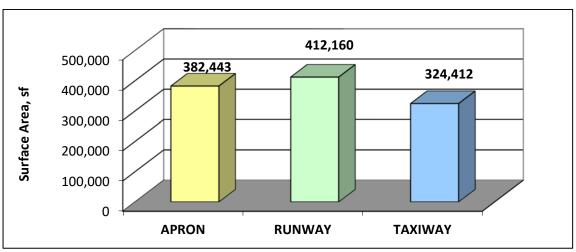
Figure 2.2: 71J Pavement Area by Surface Type.



AC

AAC

0



Maps B1D, B1E, and B1F show the pavement type, branch use, and pavement age, respectively.



## 3 Pavement Condition

#### 3.1. Introduction

A visual PCI survey of the airside pavements at 71J was conducted in order to assist in the development of a realistic PCIP. The PCI survey measures and records pavement distresses that exist within each of the inspected sample units. This survey was conducted in November 2019 by a 2-person team. The survey was performed in accordance with the methods described in ASTM D 5340-12 and FAA AC 150/5380-7B, using the sampling rates from Chapter 2 of this API report.

During the pavement survey, Quality Control (QC) and data verification were performed on both the individual distresses and the calculated section PCI values. QC included the following activities;

- Review of distress quantities to identify data entry errors (100% review at the sample unit level). General guidance was used from ASTM D5340-12, section 13, which addresses the precision of distress quantities that are recorded during PCI surveys.
- > Duplicate surveys were performed to ensure consistency between each of the inspectors in a 2-person PCI survey team.

## 3.2. Pavement Condition Rating Methodology

The PCI is a measure of the pavement's functional surface condition. It provides insight into the causes of each distress, and whether the distress is primarily caused by load, climatic conditions, and other material related deficiencies. The PCI is a numerical rating (on a scale of 0 to 100) that is based on the type, severity and quantity of each distress that is found in an inspected sample unit.

The PCI survey results are displayed using seven categories and ratings in accordance with the ASTM, but can also be presented using a simplified 3-category rating system for use in comparing with other distress related indices, as shown in Table 3.1.





**Table 3.1: Pavement Condition Index Rating Scale.** 

	Simplified PCI	ASTM PCI Color	PCI	DCI Datings and Definition
	Color Legend	Legend	Range	PCI Ratings and Definition
G00D			86-100	GOOD: Pavement has minor or no distresses and should require only routine maintenance.
09			71-85	SATISFACTORY: Pavement has scattered low-severity distresses that should require only routine maintenance.
FAIR			56-70	<u>FAIR</u> : Pavement has a combination of generally low- and medium-severity distresses. Near-term maintenance and repair needs may range from routine to major.
			41-55	<u>POOR</u> : Pavement has low-, medium-, and high-severity distresses that probably cause some operational problems. Near-term M&R needs range from routine to major. requirement for
OR			26-40	<u>VERY POOR</u> : Pavement has predominantly medium- and high- severity distresses that cause considerable maintenance & operational problems. Near-term M&R needs will be major.
POOR			11-25	SERIOUS: Pavement has mainly high-severity distresses that cause operational restrictions; immediate repairs are needed.
			0-10	<u>FAILED</u> : Pavement deterioration has progressed to the point that safe aircraft operations are no longer possible; complete reconstruction is required.

## 3.3. Distress Types

The ASTM D5340 standard considers 17 distresses, which tend to fall into one of the following four cause categories:

- ➤ <u>Load related</u>: AC distresses include alligator cracking, corrugation, depression, polished aggregate, rutting and slippage cracking; PCC distresses include corner breaks, longitudinal cracking, divided slabs, polished aggregate, pumping and joint spalling.
- Climate and durability related: AC distresses include bleeding, block cracking, joint reflection cracking, longitudinal and transverse (L&T) cracking, swelling, raveling, and weathering; PCC distresses include blow-ups, "D" cracking, longitudinal cracking, pop-outs, pumping, scaling, shrinkage cracks, and joint and corner spalling.
- Moisture & Drainage related: AC distresses include alligator cracking, depressions, potholes and swelling; PCC distresses include corner breaks, divided slabs and pumping.
- Other factors: Oil spillage, jet blast erosion, bleeding, patching and concrete slab joint faulting.



As described above, distress may have more than one cause. For example, depressions may be caused by incorrect compaction during construction, or by subgrade softening due to environmental factors. In addition, a distress may be initiated by one cause but may progress to a distress of higher severity by another cause. Therefore, engineering judgment is critical in analyzing the actual causes of the distress.

Distress descriptions provided in Appendix C were taken from the "PCI Field Manual," developed by the U.S. Army Construction Engineering Research Lab (CERL), latest edition. Appendix C provides a detailed explanation of each type of AC and PCC surface distress.

#### 3.4. Additional PCI-based Indices

The distress data used to compute PCI can also be used to calculate additional indices that are helpful in understanding the condition of the pavement and developing PCIP recommendations. One additional index that was computed is the Foreign Object Damage (FOD) potential index.

The FOD index was developed by the US Air Force and is described in detail in the US Army Corp of Engineers Engineering Technical Letter (ETL) 04-09, Pavement Engineering Assessment (EA) Standards. Loose objects on an airfield pavement surface resulting from pavement distresses can be detrimental to aircraft engines, specifically engines that are low to the ground. The objects are ingested into the engines causing costly damage and presenting a safety hazard. Not all pavement distresses create a FOD potential. Therefore, an additional index was identified that uses the results of the PCI distress survey. As shown in Figure 3.1, the scale ranges from 0 to 100 with 0 being no FOD potential. Note that the FOD index uses a simplified three color scale.

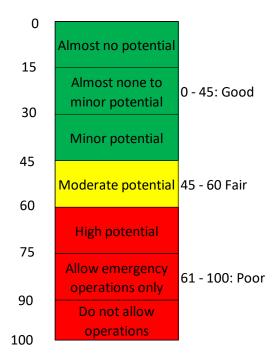


Figure 3.1: FOD Potential Rating Scale.





## 3.5. PCI Survey Results

The airside pavements at 71J include 17 sections with 229 sample units. The sample number of sample units that were surveyed in the field is 60, which is 26 percent of the total samples. Data from the inspected sample units were input into the PAVER database and a resultant PCI for each section was computed.

Figure 3.2 presents the area-weighted PCI by use and the overall airside network.

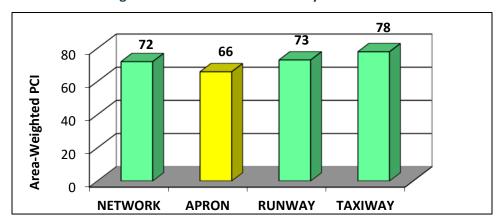


Figure 3.2: Pavement Condition by Branch Use.

Figure 3.3 shows the distribution of the 71J pavement network by condition. Approximately 6 percent of the network is in "Poor" or worse condition.

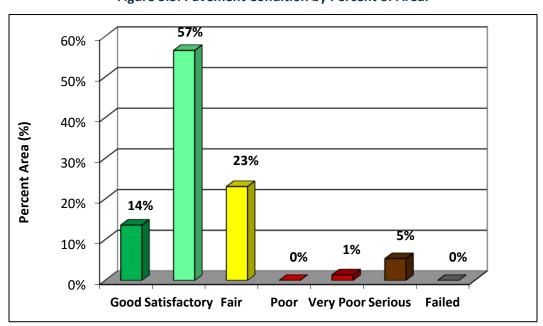


Figure 3.3: Pavement Condition by Percent of Area.



Table 3.2 is a listing of the section PCI.

Table 3.2: Section PCI.

Branch ID	Name	Section ID	Surface	Area, sf	PCI	PCI Category	FOD
A01	Apron 01	01	AC	78,123	66	Fair	48
A01	Apron 01	02	AC	53,187	22	Serious	88
A02	Apron 02	01	AC	48,373	62	Fair	52
A03	Apron 03	01	AC	80,000	94	Good	15
A04	Apron 04	01	AAC	122,760	68	Fair	46
R1331	Runway 13-31	01	AC	412,160	73	Satisfactory	40
TA	Taxiway A	01	AC	188,480	78	Satisfactory	34
TA1	Taxiway A1	01	AC	11,139	78	Satisfactory	34
TA2	Taxiway A2	01	AC	10,910	79	Satisfactory	33
ТВ	Taxiway B	01	AC	41,232	92	Good	18
TC	Taxiway C	01	AC	7,359	71	Satisfactory	42
TC01	Taxiway Connector 01	01	AC	2,903	79	Satisfactory	33
TC02	Taxiway Connector 02	01	AC	6,215	38	Very Poor	76
TC03	Taxiway Connector 03	01	AC	6,819	16	Serious	90
THANG01	Taxiway Hangar 01	01	AC	9,098	68	Fair	46
THANG01	Taxiway Hangar 01	02	AC	9,184	26	Very Poor	85
THANG02	Taxiway Hangar 02	01	AC	31,073	100	Good	0

Figure B2A and B2B in Appendix B are maps of the section PCI in 7- and 3-scale categories, respectively. Figures B2C is a map of the FOD rating. Appendix D contains a detailed report of the PCI values and distress type, quantity, and severity data for each sample unit that was surveyed in a section. Appendix E is a summary report of the extrapolated distress data at the section level.

Appendix F contains current section and branch PCI data and forecasted section PCI values. FOD values by section and branch are also presented. Figure B2D in Appendix B shows the locations of the photos that were taken during the survey. Photos are included in Appendix J.

#### 3.6. PCC Pavements

As stated earlier, the project scope did not include a detailed pavement condition survey for any Portland Cement Concrete (PCC) aprons. For these pavements, a rating of "Good", "Fair", or "Poor" was assigned based on the overall pavement condition. There are no PCC aprons at 71J.





## 4 Pavement Capital Improvement Program

#### 4.1. Introduction

PCI data were collected and entered into the PAVER database. In addition, the database customization included the following components, which are described in detail in this chapter.

- 1. Performance Modeling
- 2. Maintenance & Repair (M&R) Triggers (Critical PCI)
- 3. M&R Policies
- 4. Unit Costs

Once the database was customized, it was used to run budget analysis scenarios and develop a 7-year PCIP.

#### 4.2. Performance Modeling

To determine long-term M&R needs, a APMP must be able to predict future pavement condition. Future pavement condition is predicted using equation models that are generated from current and historical PCI data. Equation models are developed by grouping pavements based on similar performance characteristics such as region, construction history, surface type, traffic, priority and use. Mathematical techniques such as straight-line extrapolation and regression that include boundary and outlier filters are used to develop models that provide the best fit equation for the pavement condition data. PAVER's Prediction Modeling module was used to develop pavement performance models that are commonly referred to as 'Family Curves'.

Prediction models are used at the section level to compute future conditions based on the typical performance of the pavement sections that are included in each model. Future condition is computed by defining its position relative to the prediction model. The section prediction curve, or equation, is drawn through the current PCI-age point for each specific section. Since the shifted curve will run parallel to the computed prediction model, the predicted condition can be computed for any future age. Figure 4.1 is an illustration of this process.

Prediction models provide an effective way to compute future pavement performance based on past and current conditions, and pavement maintenance and rehabilitation practices. As new PCI inspection surveys are conducted, these models should be updated accordingly. In the case of the Alabama statewide airport pavement network, the best fit family curves were developed for each region by grouping pavements according to branch use (e.g. runway, taxiway) and surface type (e.g. AC, AAC, and APC). The family curves for ALDOT were developed based on branch use and are presented in Figure 4.2.



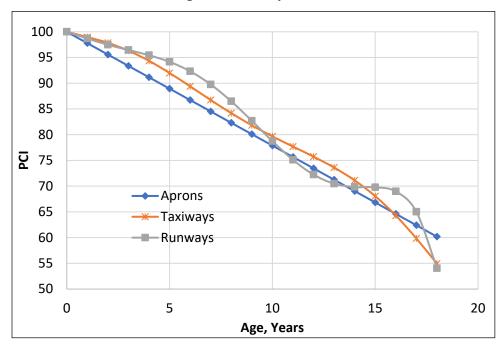


Present PCI-Age Point Modified Prediction Curve Prediction Curve 

Figure 4.1: PCI Forecasting.



Age, Years





#### 4.3. Critical PCI Values

The Critical PCI value is defined as "the PCI value at which the rate of PCI loss increases with time, or the cost of applying localized preventive maintenance increases significantly." This definition is incorporated into PAVER in defining and measuring the critical PCI values. These values, or M&R triggers, are assigned for each prediction model. As such, the critical PCI values are directly related to the branch use.

These critical PCI levels are selected based on several factors including a review of performance models; experience; other airport triggers; and acknowledge that time is required for funding approval and design. Note that preventive maintenance is recommended, and it should generally be performed above the critical PCI (trigger) values and Major M&R is generally performed below them. The critical PCI (CP) values were set at 70 for runways and taxiways, and 65 for other pavements.

#### 4.4. M&R Policies and Unit Costs

M&R policies refer to the activities that are applied at different condition levels to maintain and repair a pavement section.

Maintenance activities are localized activities which are typically assigned in the first year of the M&R plan based on the observed distresses. Safety (stopgap) maintenance addresses distresses that would affect operational safety if left unrepaired and is applied to pavements below the critical PCI. Preventive maintenance activities are aimed at slowing the rate of deterioration through consistent maintenance of existing pavements and are generally applied to pavements above the critical PCI. Appendix G presents the policies for preventive and safety maintenance.

Repair activities are conducted for larger areas, typically at the section level and are assigned based on the critical PCI. Repair activities broadly consist of three categories: preservation, rehabilitation, and reconstruction. Pavement preservation involves activities like surface treatments that are used to extend pavement service life and to delay more expensive rehabilitation work. These are applied when the pavement is in relatively good condition and does not exhibit any structural distress. Rehabilitation activities are used to repair pavements below or around the critical PCI and typically include mill and overlay. Reconstruction is recommended when the pavement has deteriorated to a level where rehabilitation is no longer cost effective.

Table 4.1 lists the pavement activity types, the individual activities within each type, and their associated 2020 unit costs. A more detailed description of the M&R activities and the development of the M&R unit costs is presented in Appendix H.

In accordance with ALDOT's focus on preservation, surface treatment is applied to all resurfaced and reconstructed runways, taxiways, and aprons three years after construction work is complete. Taxilanes and T-Hangar pavements are excluded from this requirement. This policy is applicable for projects in the PCIP between 2021 and 2024. For cost estimating, this surface treatment is assumed to have the same cost as the runway surface treatment.





Table 4.1: M&R Activities and Unit Costs.

Activity Type	PCI	Activity	Cost/sf
		Seal Cracks – AC (\$/If)	\$3.95
Maintenance	Note 1	AC Full-Depth Patching	\$25.05
		AC Partial-Depth Patching	\$16.28
Preservation	75.00	Runway Surface Treatment	\$0.57
Preservation	75-90	Taxiway and Apron Surface Treatment	\$0.85
	> CP	2" AC OL <sup>2</sup>	\$3.54
Rehabilitation	55 - CP	Mill 2" & 2" AC OL	\$3.90
	45 - 55	Mill 2" & 2" AC OLP (With Pre-Overlay Repairs)	\$4.82
Reconstruction	0 - 45	AC Reconstruction	\$8.25

<sup>&</sup>lt;sup>1</sup> Preventive > CP; Safety (Stopgap) < CP

## 4.5. Pavement CIP Development

The PAVER database, updated with condition data and customized with condition performance priorities, policies, and costs; was used to evaluate the effect of multiple funding levels on the overall future pavement condition. This output was further used to develop the PCIP. Figure 4.3 illustrates the process that PAVER uses in the funding analysis.

The following M&R funding levels were used for the 71J pavement network to help establish the 7-Year PCIP. Figure 4.4 presents the network area-weighted average PCI for each of the following funding scenarios at the end of the analysis period:

- Unlimited Funding: Unlimited funding is available for all pavement needs. The PCI increases to 91 by 2027.
- ➤ Maintain PCI: Maintain existing PCI of 73.
- Constrained Funding: This scenario constrains the funding to \$1 million each year (total of \$7 million). The PCI decreases to 73 in 2027.
- ➤ <u>Do Nothing</u>: Performing no M&R would reduce the network PCI from 76 to 55 by 2027.



<sup>&</sup>lt;sup>2</sup> For sections with structural distress and PCI > CP

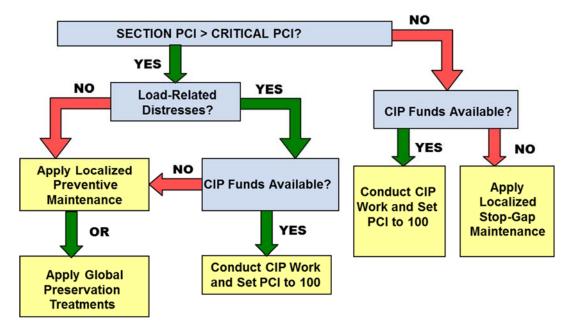


Figure 4.3: Budget Analysis Process.



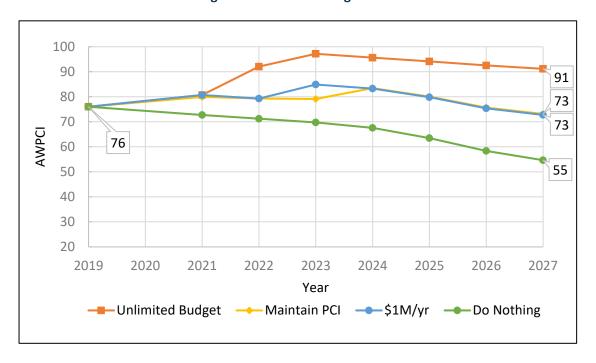


Table 4.2 summarizes the annual funding required for the above analyses. For the unlimited analysis, all pavement needs are funded in the year they are required. Therefore, the unfunded costs are zero. The total funded amount over the 7-year period is approximately \$3.5 million. For the annual funding level of \$1 million per year, funding is prioritized based on the prioritization matrix. When the needs exceed the funding for any year, the remaining sections are transferred to the succeeding year and the amount





for these activities are represented as "unfunded". The "unfunded" repairs in 2027 for this funding level is approximately \$2.3 million.

Constrained Year Unlimited Maintain PCI Do Nothing \$1M/year 2021 \$844,000 \$771,000 \$844,000 \$0 2022 \$90,000 \$12,000 \$0 \$1,664,000 2023 \$939,000 \$165,000 \$943,000 \$0 2024 \$5,000 \$812,000 \$9,000 \$0 2025 \$0 \$7,000 \$13,000 \$13,000 2026 \$9,000 \$18,000 \$18,000 \$0 2027 \$75,000 \$86,000 \$86,000 \$0 Total \$3,542,000 \$1,956,000 \$1,927,000 \$0 2027 Backlog \$2,346,000 \$2,346,000 \$4,972,000

Table 4.2: Summary of M&R Funding Level Analyses.

Map B3A in Appendix B presents the 2027 forecasted PCI by section when the M&R activities recommended in the CIP are not conducted.

## 4.6. Pavement Capital Improvement Program

The unlimited funding analysis contains rehabilitation activities for sections from the same branch spread out over the seven-year period, which is not always operationally feasible to construct. The analysis output was treated as a starting point in developing the CIP. Sections were often integrated together to account for construction feasibility and other factors, resulting in larger projects which were more realistic. In addition, each project could contain sections whose condition did not trigger rehabilitation but were included to provide a logical plan which would avoid creating "islands" of newer pavement within a particular feature. For example, if the PAVER analysis showed rehabilitation was required for eight out of 10 sections on a runway, the entire runway would be recommended for rehabilitation to provide a continuous new pavement surface.

Table 4.3 shows the projects and the associated costs for the recommended 7-year PCIP. Table 4.4 is a more detailed view of the PCIP. This table lists the individual pavement section, section level M&R work, section repair cost, surface area and the PCI before the M&R is applied. The costs that are presented represent an annual escalation rate of 3% for the unit costs. The total 7-year PCIP cost is approximately \$3.3 million. Map B3B shows the recommended repair types, while Map B3C presents the recommended projects and activities in the PCIP. Appendix I1 presents a summary of the recommended activities and cost by year for each section at 71J.



Table 4.3: Summary of 7-Year PCIP by Project.

Project Year	CIP Project	Total Project Cost	Total Project Area, sf	AWPCI Before	AWPCI After
2021	71J_21-01_Taxiway A Preservation	\$187,090	213,432	75	81
2022	71J_22-01_Runway 13-31 Rehabilitation	\$1,737,720	419,519	70	100
2022	71J_22-02_Apron 02 Rehabilitation	\$200,369	48,373	56	100
2023	71J_23-01_Taxiway B and Apron 03 Preservation	\$112,741	121,232	85	92
2024	71J_24-01_Apron 04 Rehabilitation	\$674,133	141,042	54	100
	71J_25-01_Apron 01 Rehabilitation	See Note	144,344	33	100
2025	71J_25-02_Runway 13-31 Surface Treatment	\$275,005	419,519	96	99
	71J_25-03_Apron 02 Surface Treatment	\$31,710	48,373	93	98
2027	71J_27-01_Taxiway Hangar 02 Preservation	\$32,524	31,073	85	93
2027	71J_27-02_Apron 04 Surface Treatment	\$85,373	122,760	93	98
	Total	\$3,336,665			

Cost excluded from PCIP as directed by ALDOT

Table 4.4: Summary of 7-Year PCIP by Project and Section.

Branch	Section	Area, sf	PCI Before Rehab	Activity	Activity Type	Cost
71J_21-01_Taxiway A Preservation						\$187,090
TA	01	188,480	76	Taxiway & Apron Surface Treatment	Preservation	\$165,218
TA1	01	11,139	76	Taxiway & Apron Surface Treatment	Preservation	\$9,764
TA2	01	10,910	77	Taxiway & Apron Surface Treatment	Preservation	\$9,563
TC01	01	2,903	77	Taxiway & Apron Surface Treatment	Preservation	\$2,545
71J_22-01_	Runway 1	3-31 Rehab	ilitation			\$1,737,720
R1331	01	412,160	70	Mill 2" & 2" AC OL	Rehabilitation	\$1,707,237
TC	01	7,359	63	Mill 2" & 2" AC OL	Mill 2" & 2" AC OL Rehabilitation	
71J_22-02_	Apron 02	Rehabilitati	on			\$200,369
A02	.02 01 48,373 57 Mill 2" & 2" AC OL Rehabilitation		\$200,369			
71J_23-01_	71J_23-01_Taxiway B and Apron 03 Preservation					
A03	01	80,000	87	Taxiway & Apron Surface Treatment Preservation		\$74,397
ТВ	01	41,232	84	Taxiway & Apron Surface Treatment Preservation		\$38,344
71J_24-01_	71J_24-01_Apron 04 Rehabilitation					
A04	01	122,760	59	Mill 2" & 2" AC OL Rehabilit		\$539,460
THANG01	01	9,098	49	Mill 2" & 2" AC OLP Rehabilita		\$49,357
THANG01	02	9,184	11	AC Reconstruction	Reconstruction	\$85,316
71J_25-01_Apron 01 Rehabilitation					See Note	
A01	01	78,123	55	Mill 2" & 2" AC OLP Rehabilitation		\$0
A01	02	53,187	11	AC Reconstruction Reconstruction		\$0
TC02	01	6,215	20	AC Reconstruction Reconstruction		\$0





Branch	Section	Area, sf	PCI Before Rehab	Activity	Activity Type	Cost
TC03	01	6,819	0	AC Reconstruction Reconstruction		\$0
71J_25-01_	Runway 1	3-31 Surfac	e Treatme	ent		\$275,005
R1331	01	412,160	-	Surface Treatment	Preservation	\$270,181
TC	01	7,359	-	Surface Treatment	\$4,824	
71J_25-03_Apron 02 Surface Treatment						\$31,710
A02	01	48,373	-	Surface Treatment Preservation		\$31,710
71J_27-01_Taxiway Hangar 02 Preservation						\$32,524
THANG02	01	31,073	86	Taxiway & Apron Surface Treatment   Preservation		\$32,524
71J_27-02_Apron 04 Surface Treatment						\$85,373
A04	01	122,760	-	Surface Treatment Preservation		\$85,373
					Total	\$3,336,665

Cost excluded from PCIP as directed by ALDOT

The FAA, under the Airport Improvement Program (AIP) provides approximately 90 percent of eligible costs for planning and development of public-use airports included in the NPIAS as grants. The remaining 10 percent of costs are shared between ALDOT and the airport sponsor. The following is the distribution of the 7-yr PCIP cost of \$3.3 million for 71J:

➤ FAA (90%): \$2.9 million
 ➤ ALDOT (5%): \$0.2 million
 ➤ Airport Sponsor (5%): \$0.2 million

The recommendations within the PCIP are based on a network-level study and should be used for planning purposes only. A detailed project-level assessment should be conducted for each project to determine the appropriate repair activities and develop more accurate cost estimates.

Table 4.5 summarizes the maintenance activities that are recommended for Year 1 (2021). The estimated cost is approximately \$25,498. A complete listing of the maintenance activities by section is presented in Appendix I2. This may be used as a basis for establishing an annual maintenance budget for the 71J pavements.

Table 4.5: Summary of Year-1 Maintenance Plan.

Policy	Work Description	Work Quantity	Work Unit	Work Cost
Preventive	Crack Sealing - AC	6,414	Ft	\$25,336
	Patching - AC Partial-Depth	10	SqFt	\$163
			Total	\$25,498





## Appendix A Pavement Inventory Report

Ozark Airport - Blackwell Field (71J)

Branch ID	Name	Branch Use	Section ID	Rank <sup>1</sup>	Length (ft)	Width (ft)	Area (sf)	LCD <sup>2</sup>	Surface <sup>3</sup>
A01	Apron 01 Ozark	APRON	01	S	392	291	78,123	1/1/2005	AC
A01	Apron 01 Ozark	APRON	02	S	905	100	53,187	1/1/1956	AC
A02	Apron 02 Ozark	APRON	01	S	324	158	48,373	1/1/1956	AC
A03	Apron 03 Ozark	APRON	01	S	400	200	80,000	1/2/2016	AC
A04	Apron 04 Ozark	APRON	01	S	660	186	122,760	10/2/2008	AAC
R1331	Runway 13-31 Ozark	RUNWAY	01	Р	5,152	80	412,160	1/1/2005	AC
TA	Taxiway A Ozark	TAXIWAY	01	Р	5,421	35	188,480	1/1/2005	AC
TA1	Taxiway A1 Ozark	TAXIWAY	01	S	184	42	11,139	1/1/2005	AC
TA2	Taxiway A2 Ozark	TAXIWAY	01	S	183	37	10,910	1/1/2005	AC
ТВ	Taxiway B Ozark	TAXIWAY	01	Р	1,100	35	41,232	1/2/2016	AC
TC	Taxiway C Ozark	TAXIWAY	01	S	160	35	7,359	1/1/1956	AC
TC01	Taxiway Connector 01 Ozark	TAXIWAY	01	S	61	30	2,903	1/1/1956	AC
TC02	Taxiway Connector 02 Ozark	TAXIWAY	01	S	110	35	6,215	1/1/1997	AC
TC03	Taxiway Connector 03 Ozark	TAXIWAY	01	S	100	40	6,819	10/19/1990	AC
THANG01	Taxiway Hangar 01 Ozark	TAXIWAY	01	Т	182	50	9,098	1/1/1956	AC
THANG01	Taxiway Hangar 01 Ozark	TAXIWAY	02	Т	206	44	9,184	1/1/1956	AC
THANG02	Taxiway Hangar 02 Ozark	TAXIWAY	01	Т	556	49	31,073	6/1/2018	AC

<sup>&</sup>lt;sup>1</sup> P = Primary pavement, S = Secondary pavement, T = Tertiary pavement

<sup>&</sup>lt;sup>2</sup> LCD = Last construction date. The date of the last major pavement rehabilitation (e.g. AC overlay)

<sup>&</sup>lt;sup>3</sup> AC = Asphalt Cement Concrete, AAC = Aphalt Overlay AC, PCC = Portland cement Concrete, APC = Asphalt Overlay PCC

## **APPENDIX B**

## **PMP Maps**

**B1: Inventory Maps** 

B1A: Branch Identification B1B: Section Identification B1C: Sample Unit Layout

B1D: Pavement Type

B1E: Branch Use

B1F: Pavement Age

**B2: Surface Condition Maps** 

B2A: 7-Color PCI B2B: 3-Color PCI

B2C: FOD Rating

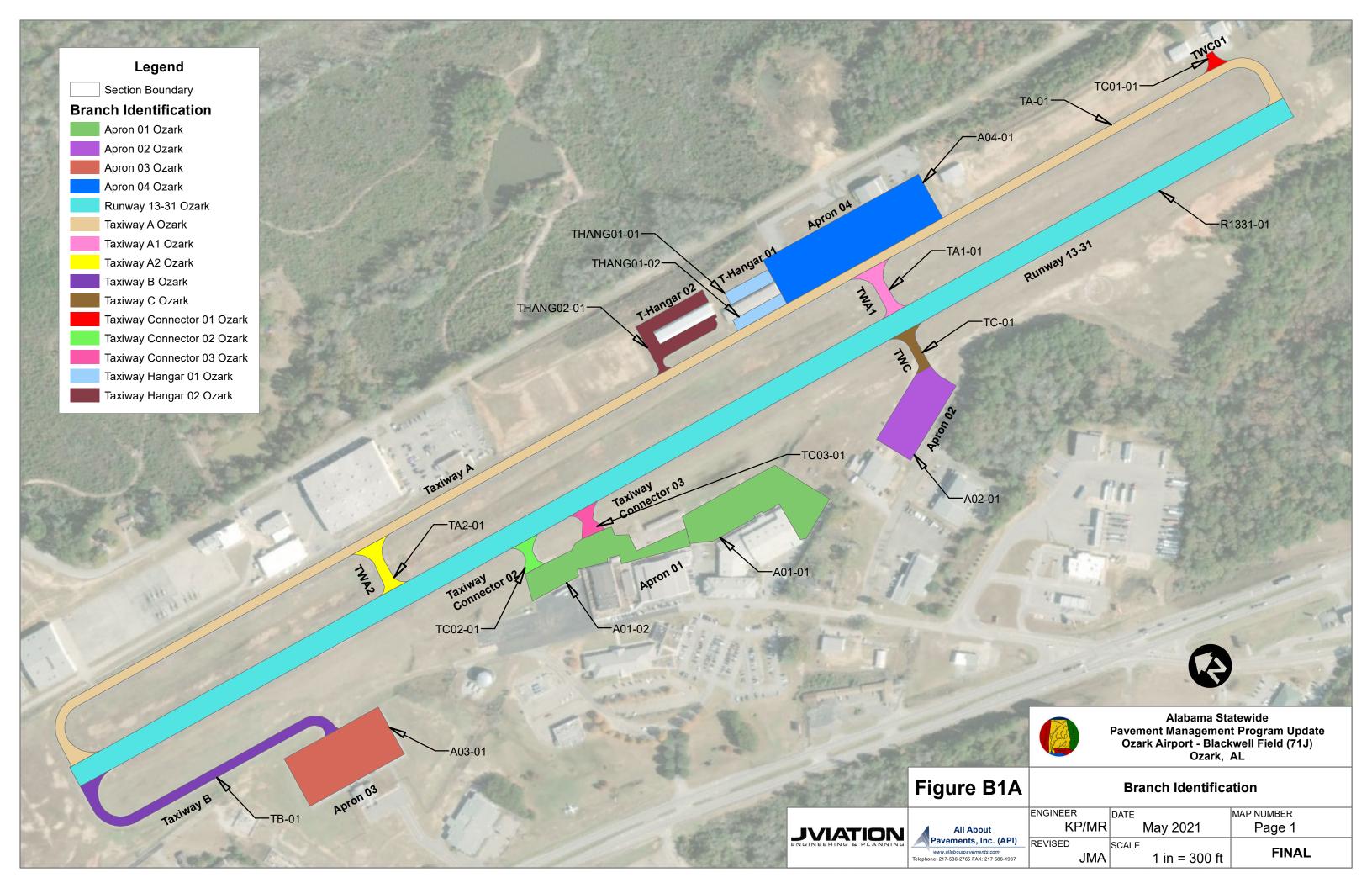
**B2D: Survey Photo Locations** 

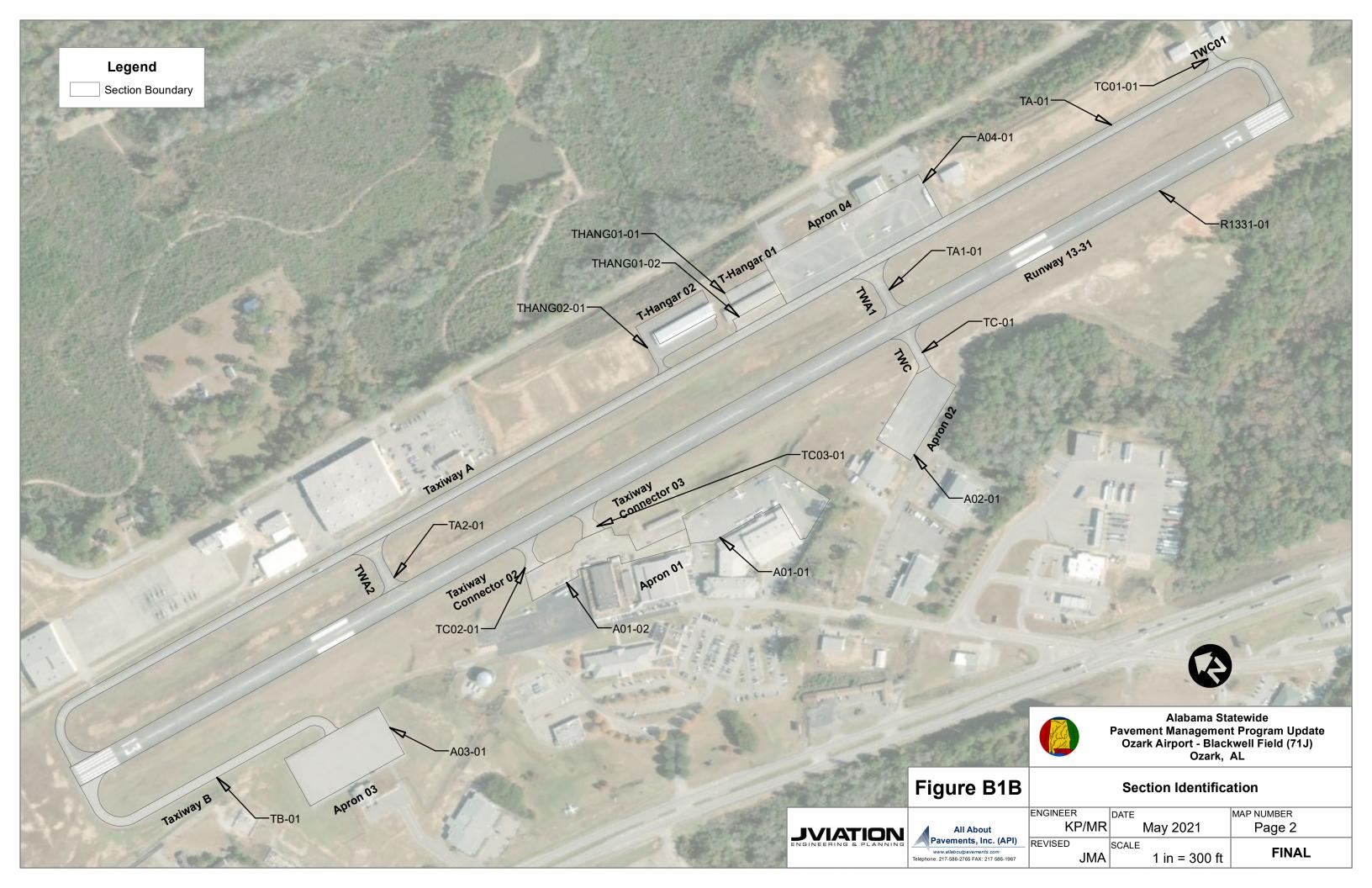
B3: Pavement Capital Improvement Plan (PCIP) Maps

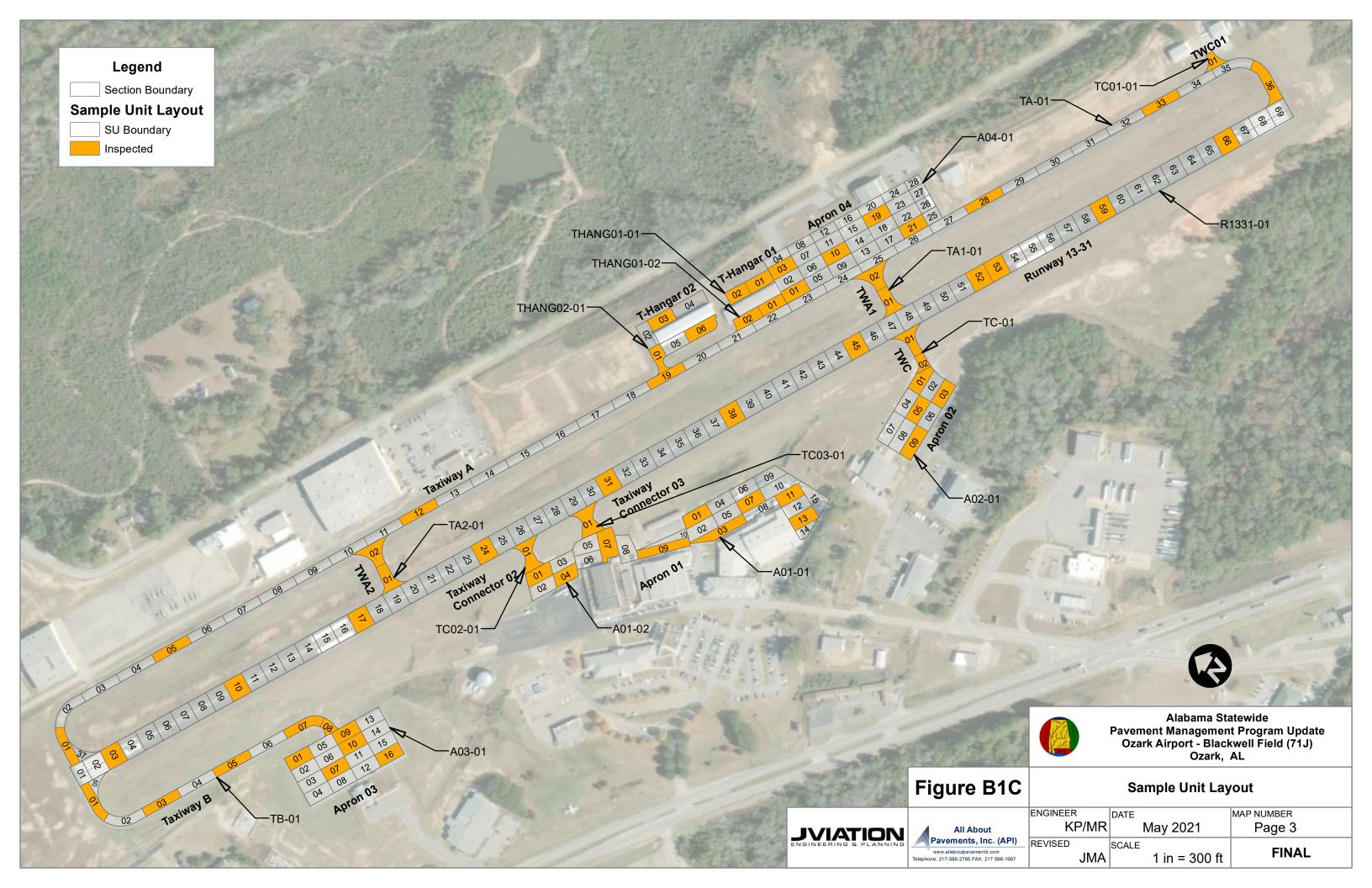
B3A: 2027 Forecasted PCI without PCIP

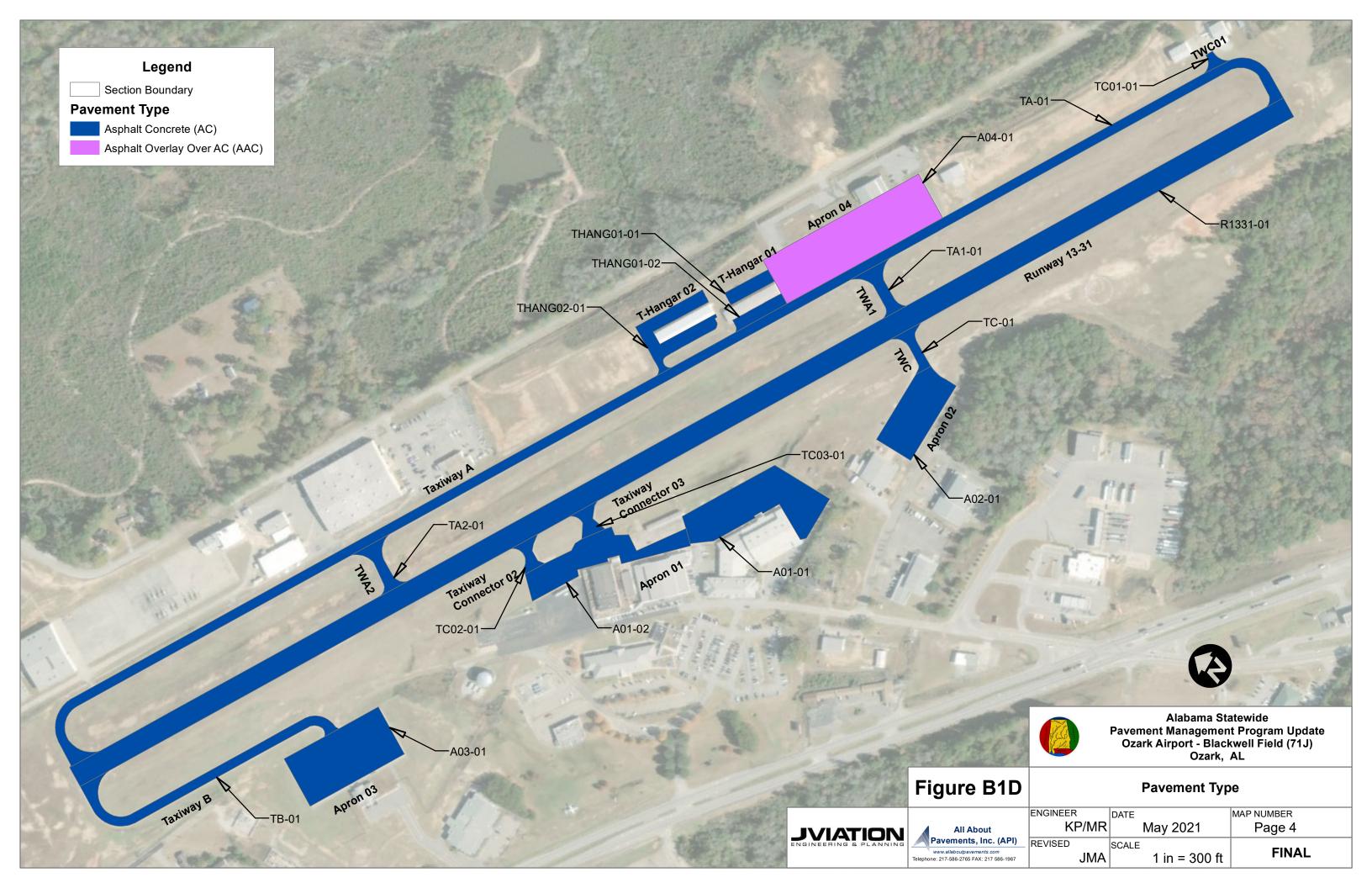
B3B: M&R Needs

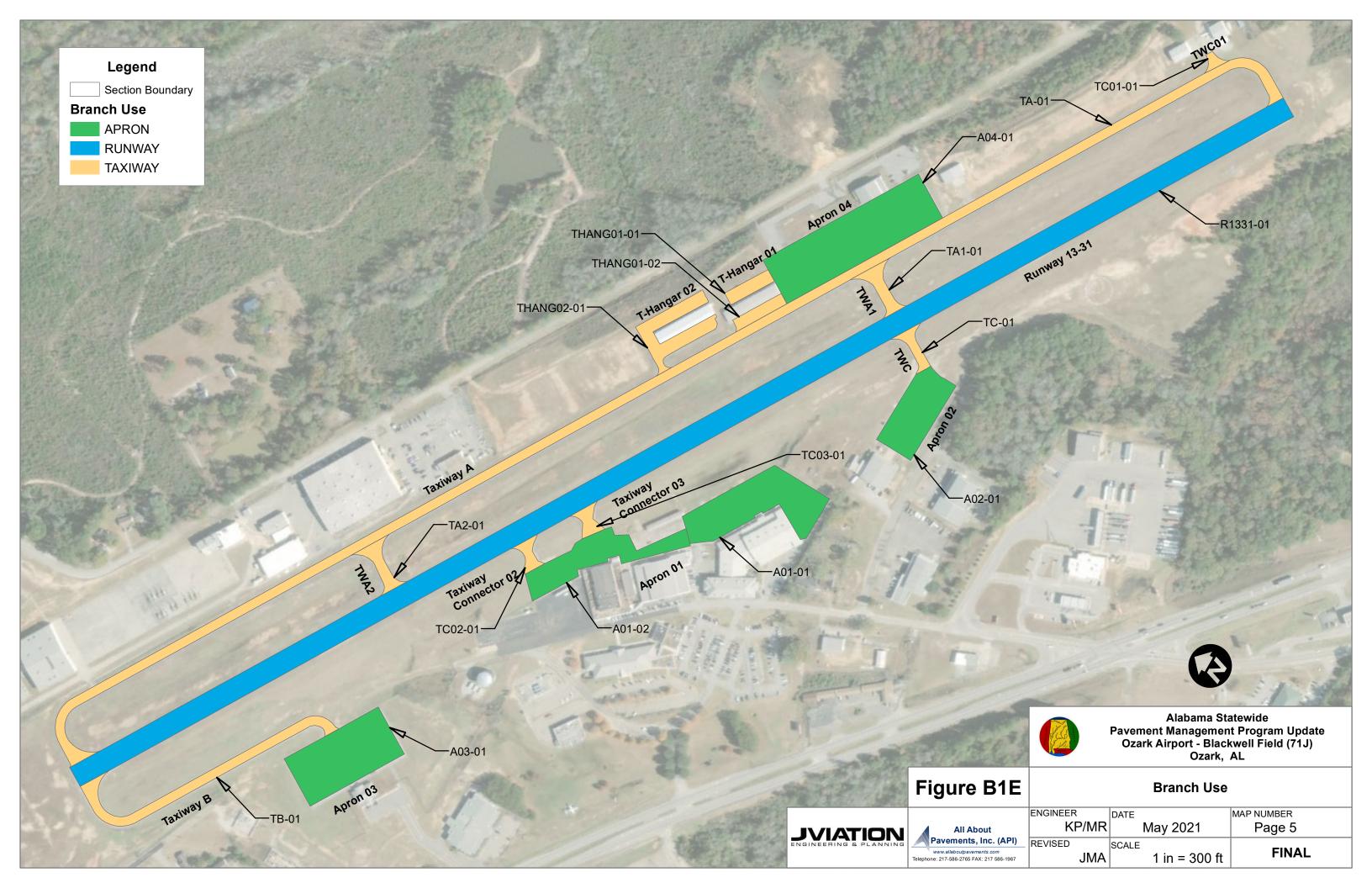
**B3C: PCIP Recommendations** 

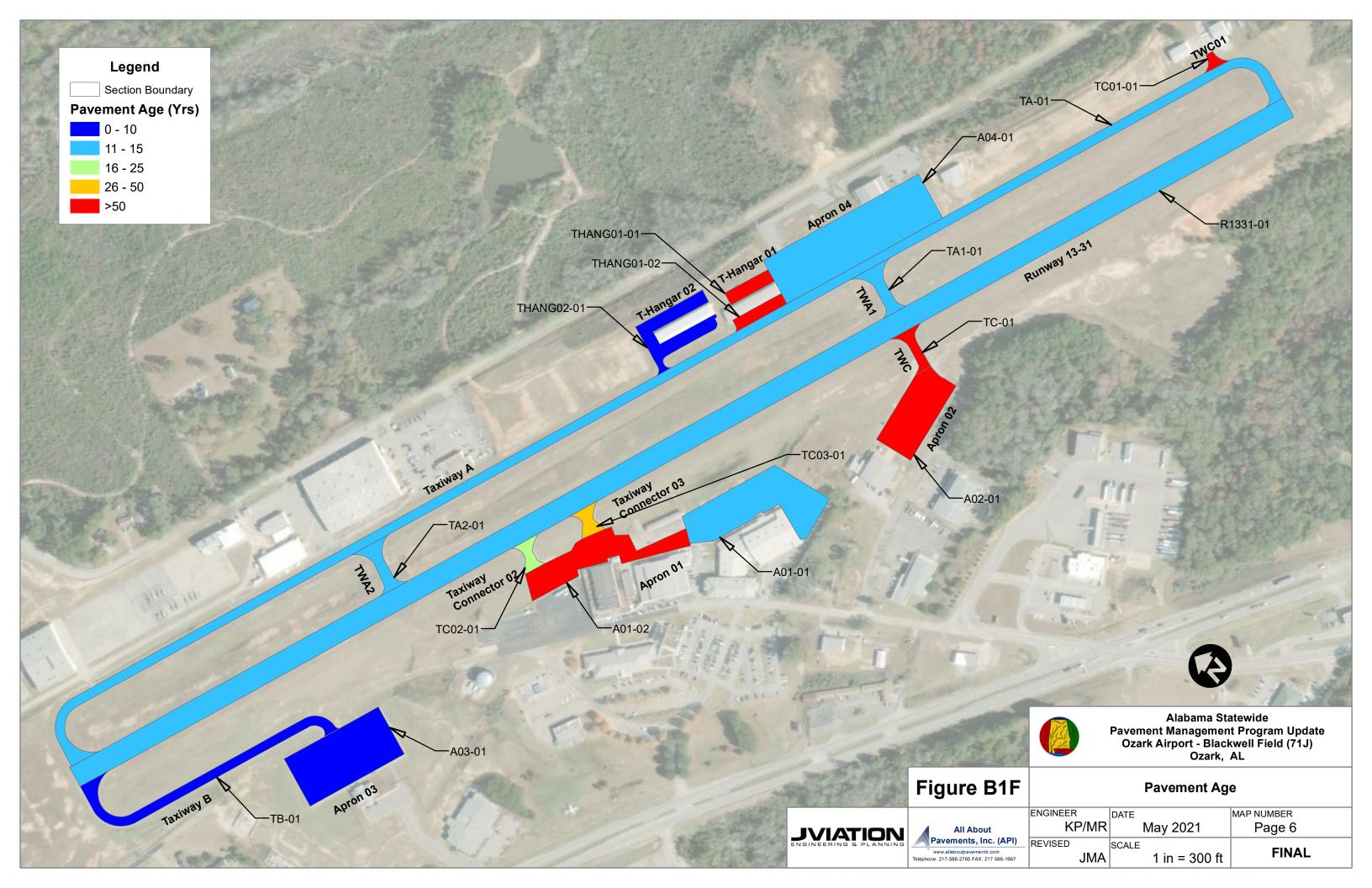


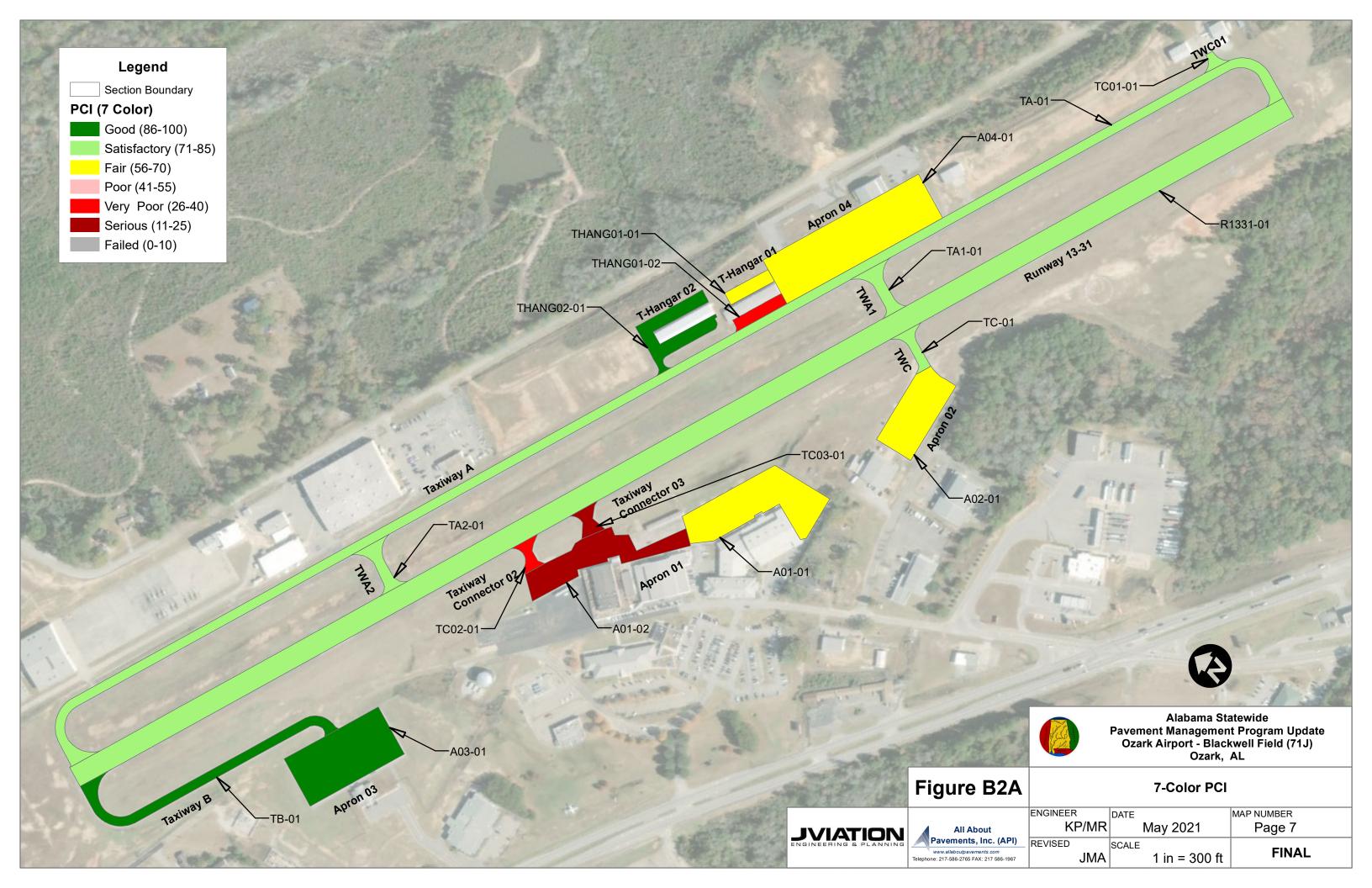


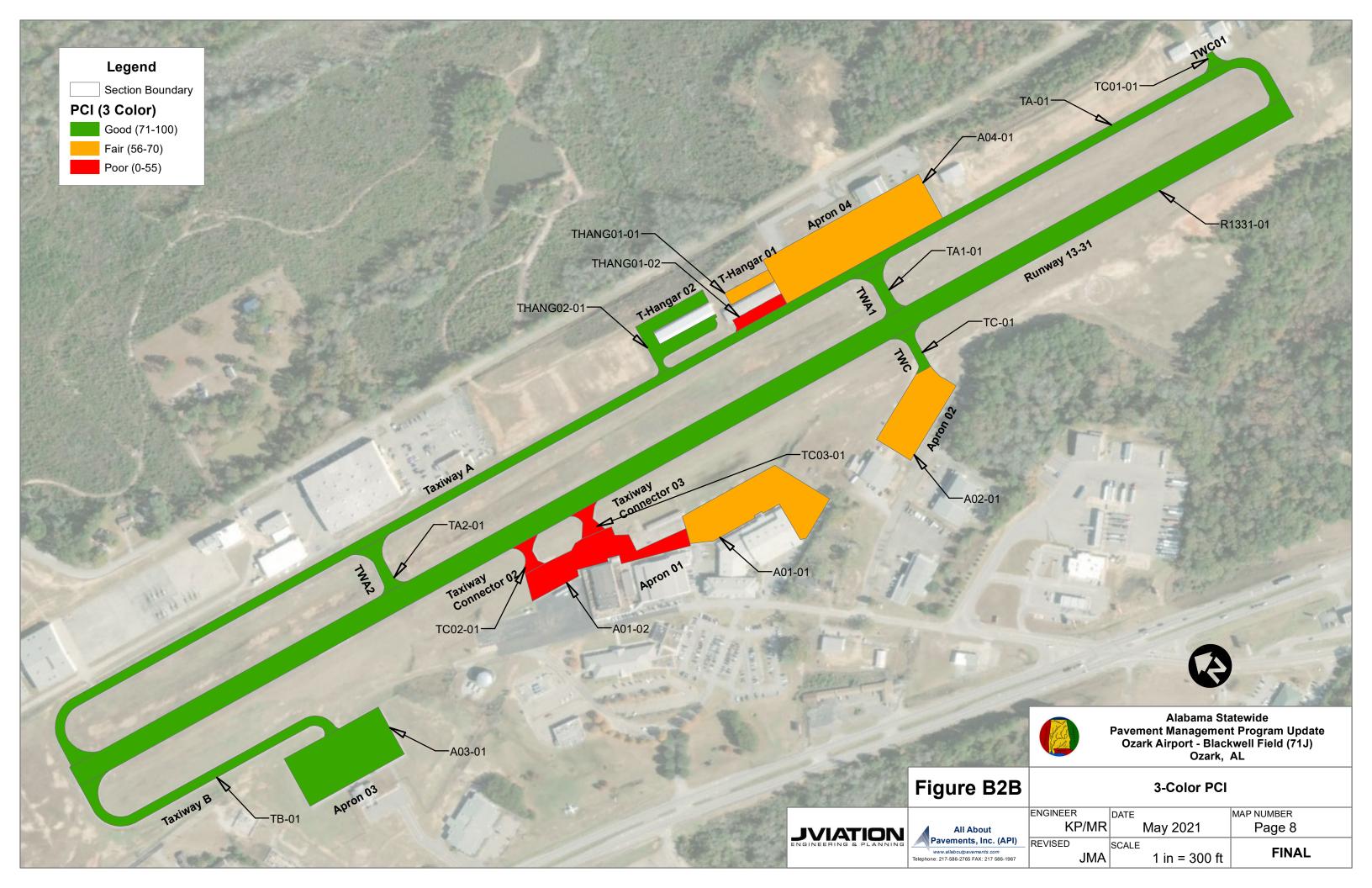


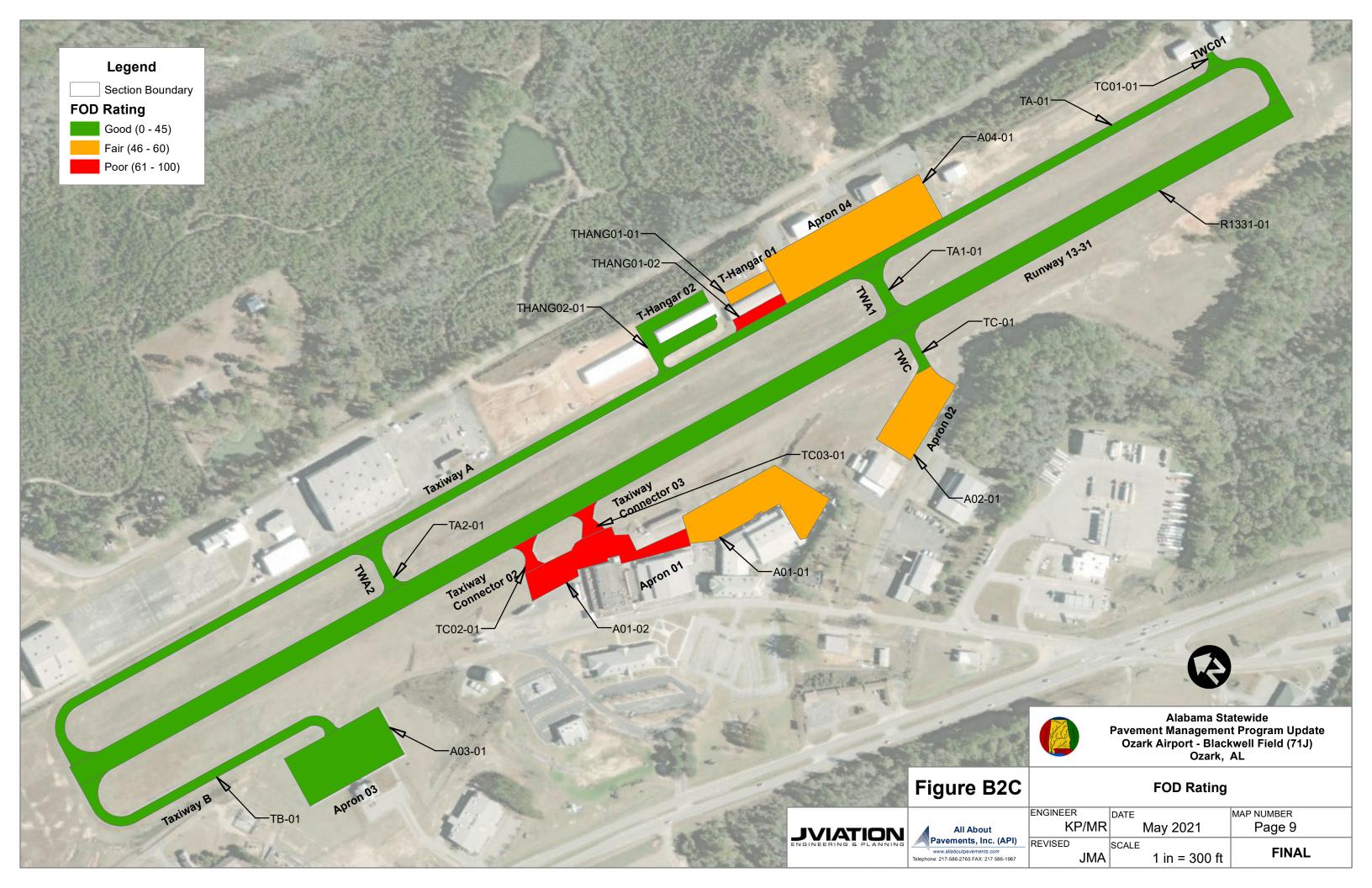


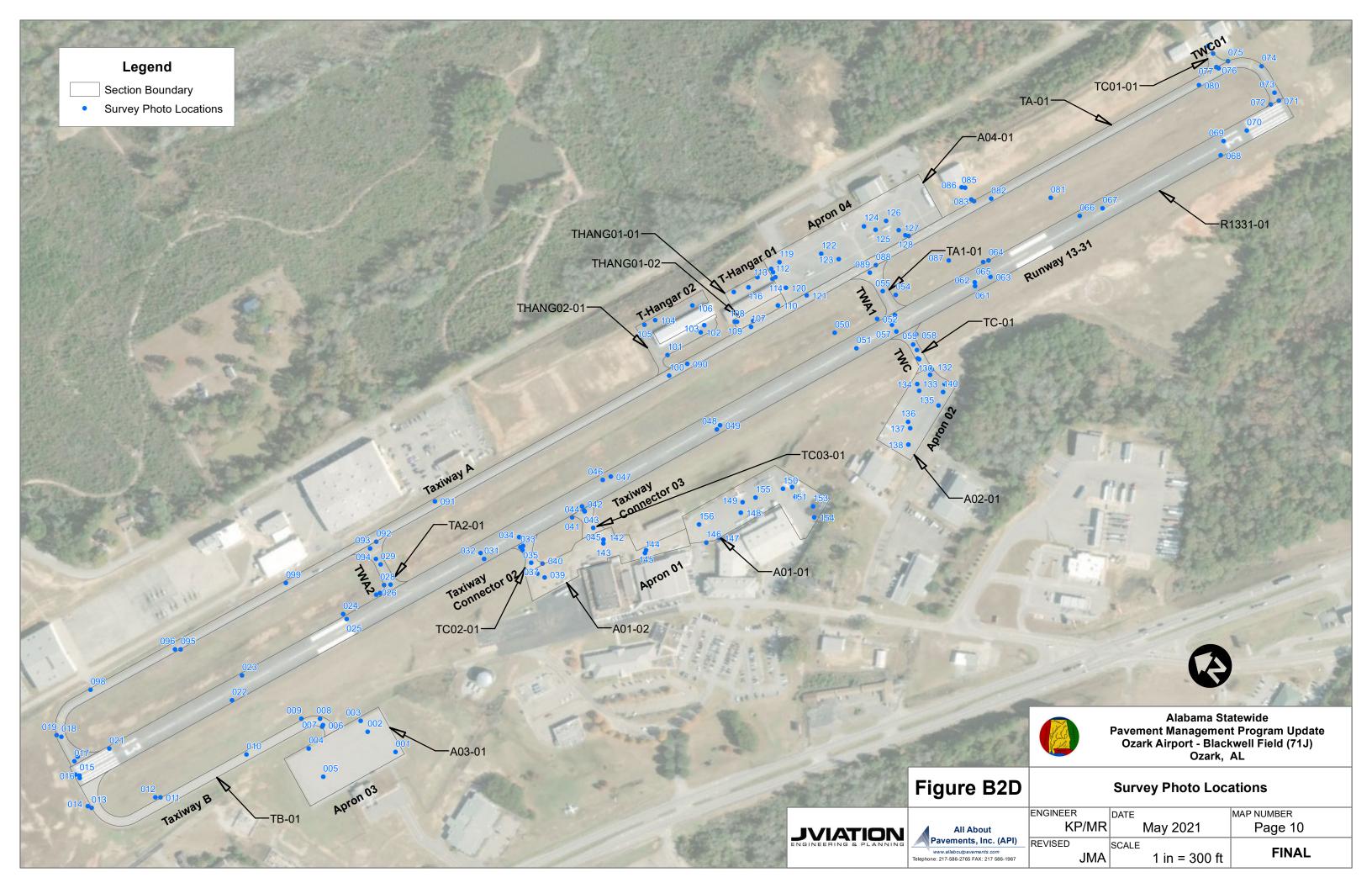


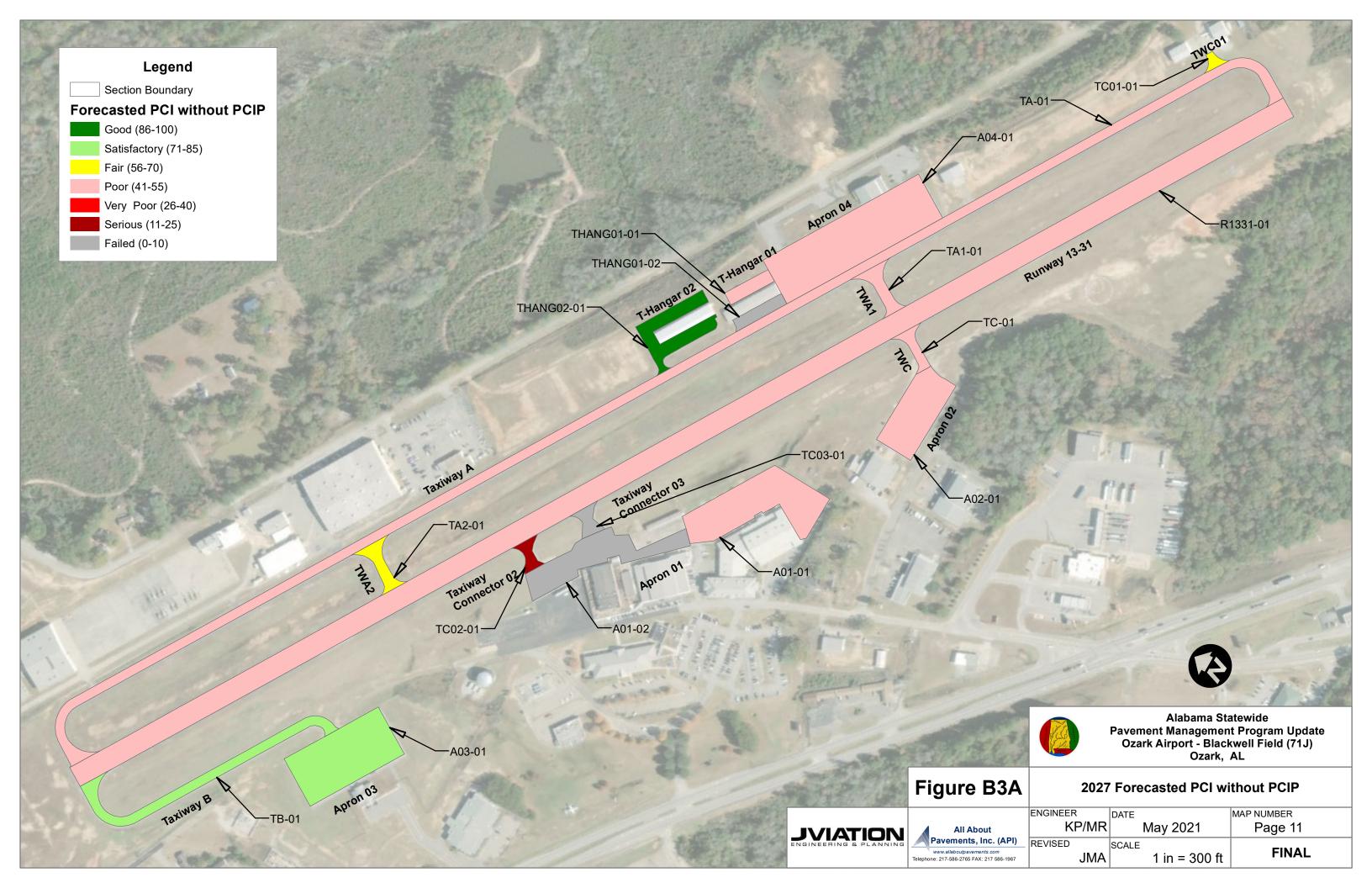


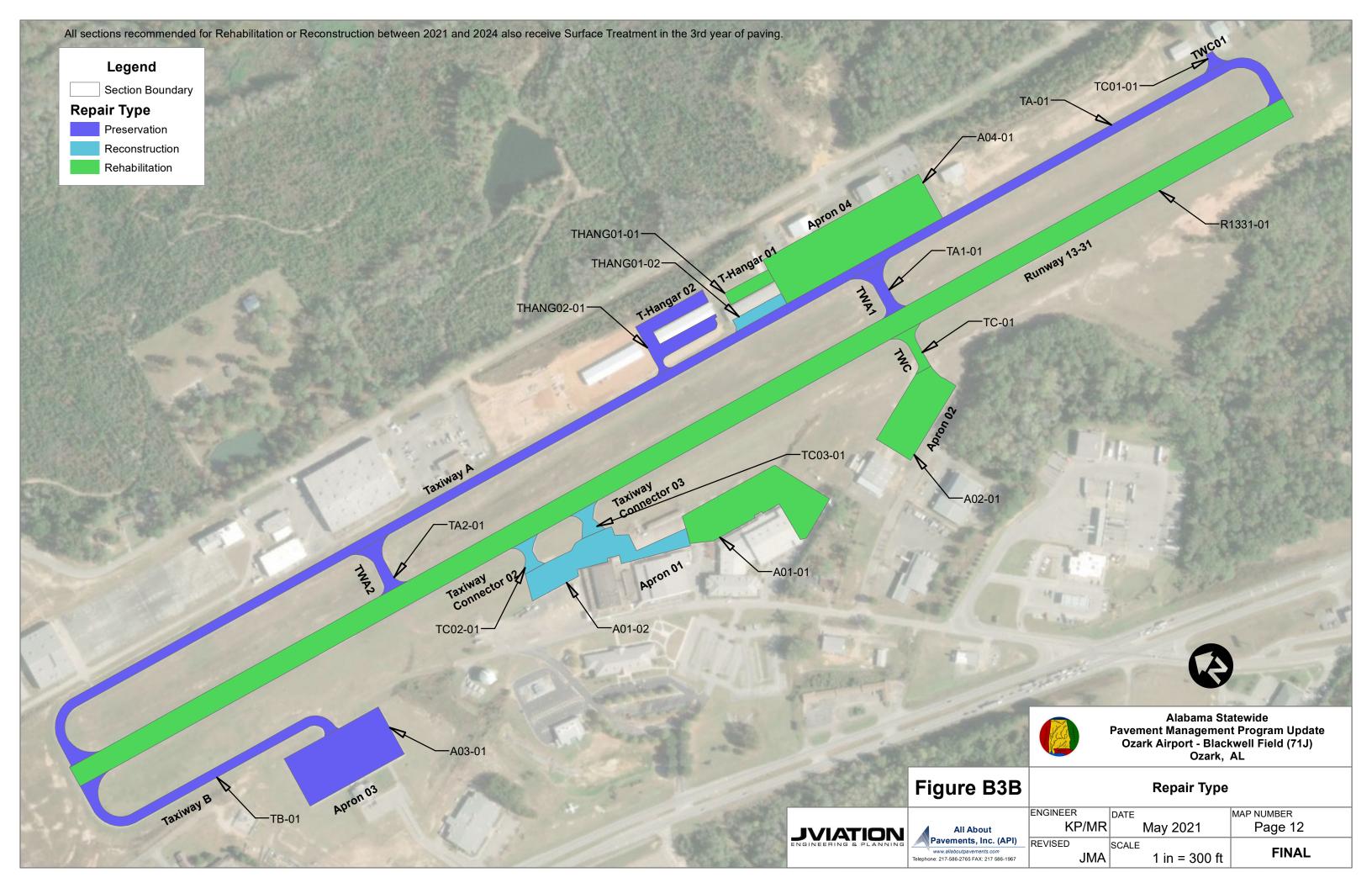


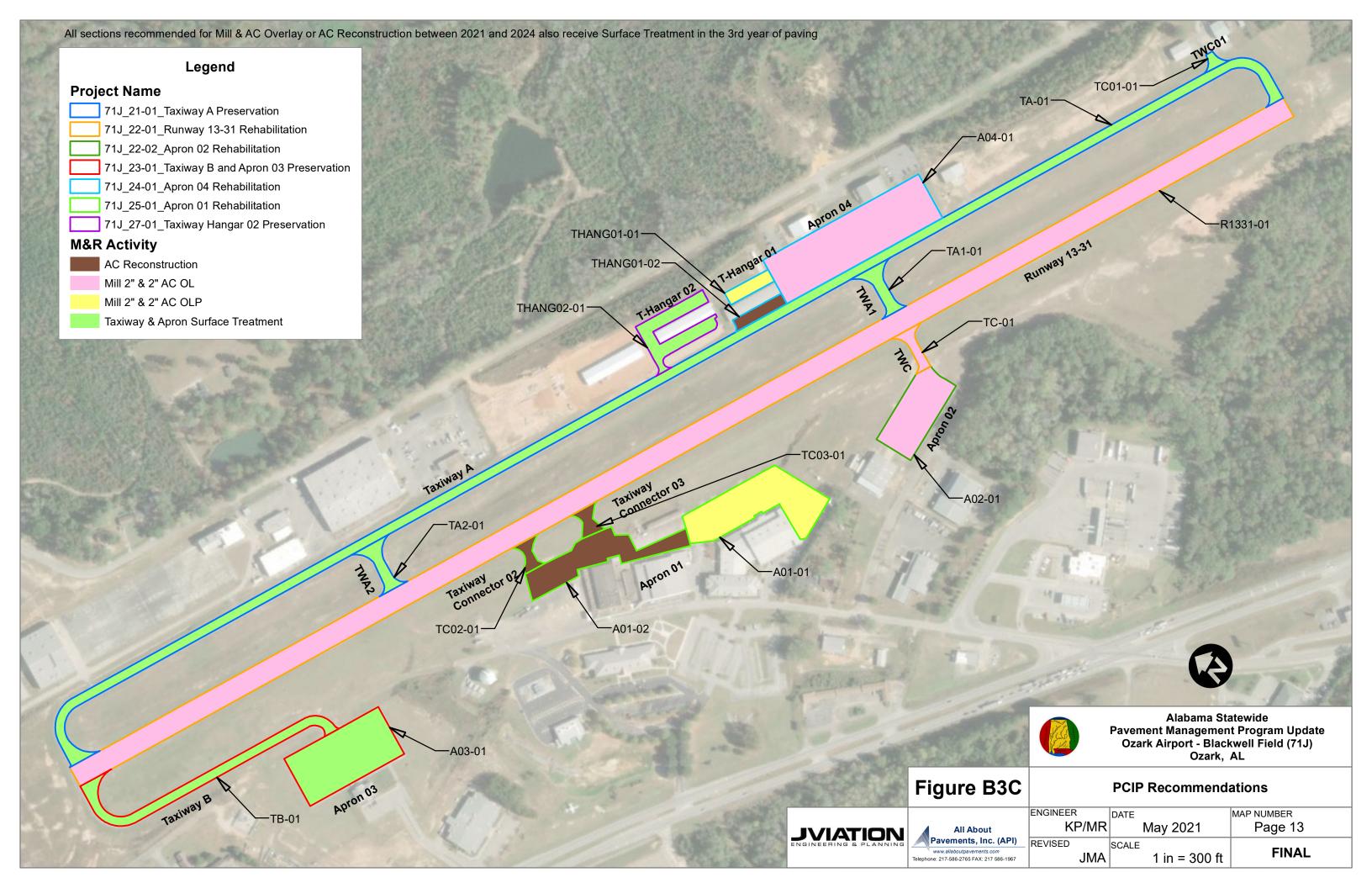


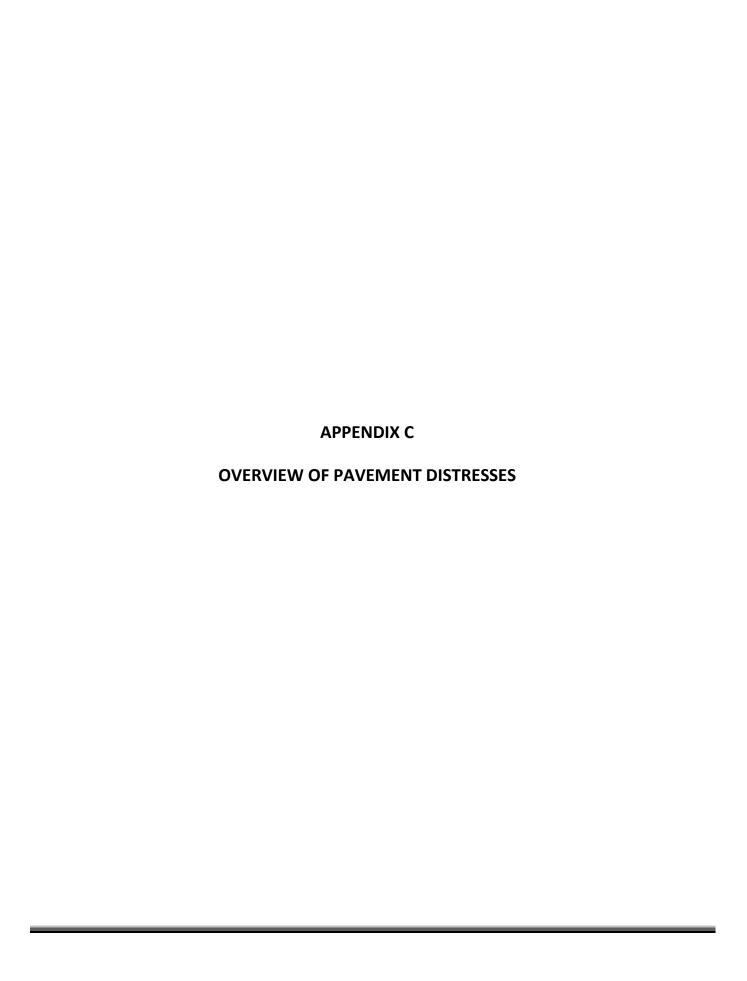












### 1. Alligator Cracking (AC)

Alligator cracking is a series of interconnecting cracks caused by fatigue failure of the asphalt concrete surface where tensile stress and strain is highest under wheel loads. The cracks propagate to the surface initially as a series of parallel cracks. After repeated traffic loading the cracks connect, forming many sided, sharp angled pieces that develop a pattern resembling chicken wire or the skin of an alligator. The pieces are less than 2 feet long on the longest side. Alligator cracking occurs only in areas that are subjected to repeated traffic loading, such as wheel paths, and is considered a major structural distress.

#### Severities:

- Low made up of fine, hair-like cracks running parallel to each other with none or only a few inter-connecting cracks. The cracks are not spalled;
- Medium Further development of light alligator cracking into a pattern or network of cracks that may be lightly spalled. Medium-severity alligator cracking is defined by a well-defined pattern of interconnecting cracks, where all pieces are securely held in place (good aggregate interlock between pieces);
- High has progressed so that the pieces are well defined and spalled at the edges. Some of the pieces may rock under traffic and may cause FOD potential.

- Low No action, surface seal or overlay for low severity distress;
- Medium partial or full depth patch, overlay or reconstruct;
- High partial or full depth patch, overlay or reconstruct.



### 2. Bleeding (AC)

Bleeding is a film of bituminous material on the pavement surface that creates a shiny, glass-like, reflecting surface that usually becomes quite sticky. Bleeding is caused by excessive amounts of asphaltic cement or tars in the mix or low-air void content, or both. It occurs when asphalt fills the voids of the mix during hot weather and then expands out onto the surface of the pavement. Since the bleeding process is not reversible during cold weather, asphalt or tar will accumulate on the surface.

<u>Severities</u>: No degrees of severity are defined. Bleeding should be noted when it is extensive enough to reduce skid resistance.

<u>Repair Policies</u>: Do nothing; sand blot the distressed area by applying heat and roll sand into the areas affected with bleeding, remove the excess material; patch.



### 3. Block Cracking (AC)

Block cracks are interconnected cracks that divide the pavement into rectangular shaped pieces. The blocks may range in size from 1 by 1 foot to 10 by 10 feet. Block cracking is caused mainly by shrinkage of the asphalt concrete and is not load associated. The occurrence of block cracking usually indicates that the asphalt has hardened significantly. Block cracking normally occurs over a large proportion of the pavement area, but will sometimes occur only in the non-traffic areas.

### Severities:

- Low defined by cracks that are at most lightly spalled, causing no foreign object damage (FOD) potential. Un-filled cracks have 1/4 inch or less mean width, and filled cracks have filler in satisfactory condition;
- Medium defined by cracks that are moderately spalled (some FOD potential), un-filled cracks that are at most lightly spalled, but have a mean width greater than 1/4 inch or filled cracks that are at most lightly spalled but have filler in unsatisfactory condition;
- High defined by cracks that are severely spalled, causing a definite FOD potential.

- ♦ Low No action:
- Medium seal cracks, apply rejuvenator, recycle surface or heat scarify and overlay;
- High recycle surface or heat scarify and overlay.

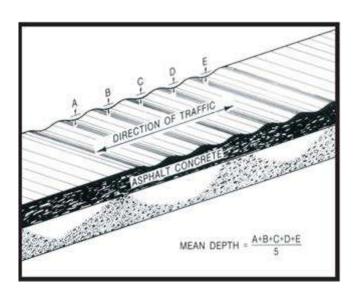


# 4. Corrugation (AC)

#### Description

Corrugation is a series of closely spaced ridges and valleys (ripples) occurring at fairly regular intervals, usually less than 5 feet (1.5 meters) along the pavement. The ridges are perpendicular to the traffic direction. Traffic action combined with an unstable pavement surface or base usually causes this type of distress.

- Corrugations are minor and do not significantly affect ride quality (see measurement criteria below).
- M Corrugations are noticeable and significantly affect ride quality (see measurement criteria below).
- Corrugations are easily noticed and severely affect ride quality (see measurement criteria below).



### 5. Depression (AC)

Depressions are localized pavement surface areas having elevations slightly lower than those of the surrounding pavement. In many instances, light depressions are not noticeable until after a rain, when ponding water creates "birdbath" areas; but the depressions can also be located without rain because of stains created by ponding of water. Depressions can be caused by settlement of the foundation soil or can be built during construction. Depressions cause roughness and, when filled with water of sufficient depth, could cause hydroplaning of aircraft.

#### Severities:

- ◆ Low Depression can be observed or located by stained areas, only slightly affects pavement riding quality, and may cause hydroplaning potential on runways. Maximum depth 1/8 to 1/2 inch for runways, 1/2 to 1 inch for taxiways and aprons;
- ♦ Medium The depression can be observed, moderately affects pavement riding quality, and causes hydroplaning potential on runways. Maximum depth 1/2 to 1 inch for runways, 1 to 2 inches for taxiways and aprons;
- High The depression can be readily observed, severely affects pavement riding quality, and causes definite hydroplaning potential; Depth greater than 1 inch for runways, greater than 2 inches for taxiways and aprons;.

- ♦ Low No action;
- Medium Shallow, partial or full depth patch;
- High Shallow, partial or full depth patch.



# 6. Jet Blast (AC)

# Description

Jet blast erosion causes darkened areas on the pavement surface when bituminous binder has been burned or carbonized; localized burned areas may vary in depth up to approximately 1/2 inch (13 millimeters).

# Severity Levels

No degrees of severity are defined. It is sufficient to indicate that jet blast erosion exists.



### 7. Joint Reflection Cracking (AC)

### **Description**

This distress occurs only on pavements having an asphalt or tar surface over a PCC slab. This category does not include reflection cracking from any other type of base (i. e., cement stabilized, lime stabilized); such cracks are listed as longitudinal and transverse cracks. Joint-reflection cracking is caused mainly by movement of the PCC slab beneath the AC surface because of thermal and moisture changes; it is not load related. However, traffic loading may cause a breakdown of the AC near the crack, resulting in spalling and FOD potential. If the pavement is fragmented along a crack, the crack is said to be spalled. A knowledge of slab dimensions beneath the AC surface will help to identify these cracks.

### Severity Levels

Cracks have only light spalling (little or no FOD potential) or no spalling and can be filled or non-filled. If non-filled, the cracks have a mean width of 1/4 inch (6 millimeters) or less. Filled cracks are of any width, but their filler material is in satisfactory condition.

One of the following conditions exists: (1) cracks are moderately spalled (some FOD potential) and can be either filled or non-filled of any width; (2) filled cracks are not spalled or are only lightly spalled, but the filler is in unsatisfactory condition; (3) non-filled cracks are not spalled or are only lightly spalled, but the mean crack width is greater than 1/4 inch (6 millimeters); or (4) light random cracking exists near the crack or at the corner of intersecting cracks.

Cracks are severely spalled (definite FOD potential) and can be either filled or non-filled of any width.



### 8. Longitudinal and Transverse Cracking (AC)

Longitudinal and transverse (L&T) cracks are parallel to the pavement's centerline or laydown direction. They may be caused by: 1) a poorly constructed paving lane joint, 2) shrinkage of the AC surface due to hardening of the asphalt, or 3) a reflective crack caused by cracks beneath the surface course. Transverse cracks extend across the pavement perpendicularly to the pavement centerline or laydown direction, and may be caused by items 2) or 3) as stated above. These types of cracks are not usually load related.

#### Severities:

- Low have either minor spalling or no spalling. The cracks can be filled or unfilled. Un-filled cracks have a mean width of 1/4 inch or less. Filled cracks are any width but their filler is in satisfactory condition;
- Medium one of the following conditions exists: 1) cracks are moderately spalled and can be either filled or un-filled of any width; 2) filled cracks are not spalled or only lightly spalled, but the filler is in unsatisfactory condition; 3) unfilled cracks are not spalled or only lightly spalled, but the crack width exceeds 1/4 inch; or 4) light random cracking exists near the crack or at the corner of the intersecting cracks;
- High severely spalled with a definite FOC potential. They can be either filled or un-filled.

- Low No action;
- Medium seal cracks;
- High seal cracks or perform a full depth patch.



# 9. Oil Spillage (AC)

Oil spillage is the deterioration or softening of the pavement surface caused by the spilling of oil, fuel, or other solvents.

<u>Severities</u>: No degrees of severity are defined. It is sufficient to indicate that oil spillage exists.

- ♦ Do nothing;
- Partial or full depth patch.



# 10. Patching

Repair patching and utility cut patching is considered a defect, regardless of how well it performs or was constructed.

### **Severities**:

- Low in good condition and is performing satisfactorily;
- Medium is somewhat deteriorated and affects riding quality to some extent;
- High is badly deteriorated and affects riding quality significantly or has high FOD potential.

- ♦ Low No action;
- Medium seal cracks, repair the distresses in the patch or replace the patch;
- High replace the patch.



Figure C.7: Asphalt Patching.

## 11. Polished Aggregate (AC)

## Description

Aggregate polishing is caused by repeated traffic applications. Polished aggregate is present when close examination of a pavement reveals that the portion of aggregate extending above the asphalt is either very small or there are no rough or angular aggregate particles to provide good skid resistance. Existence of this type of distress is also indicated when the number on a skid resistance rating test is low or has dropped significantly from previous ratings.

### Severity Levels

No degrees of severity are defined. However, the degree of polishing should be significant before it is included in the condition survey and rated as a defect.



### 12. Raveling (AC)

#### **Definition**

Raveling is the dislodging of coarse aggregate particles from the pavement surface.

### Dense Mix Severity Levels.

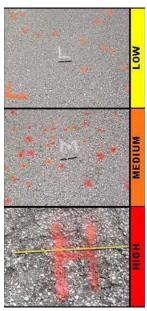
As used herein, coarse aggregate refers to predominant coarse aggregate sizes of the asphalt mix. Aggregate clusters refer to when more than one adjoining coarse aggregate piece is missing. If in doubt about a severity level, three representative areas of 1 square yard (1 square meter) each should be examined and the number of missing coarse aggregate particles counted.

Low severity occurs if any one of these conditions exist: (1) In a square yard (square meter) representative area, the number of coarse aggregate particles missing is between 5 and 20. (2) Missing aggregate clusters is less than 2 percent of the examined square yard (square meter) area. In low severity raveling, there is little or no FOD potential.

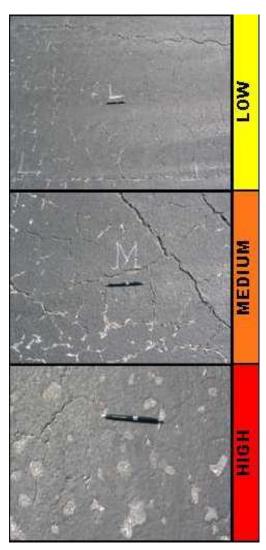
Medium severity occurs if any one of these conditions exist: (1) In a square yard (square meter) representative area, the number of coarse aggregate particles missing is between 21 and 40. (2) Missing aggregate clusters is between 2 and 10 percent of the examined square yard (square meter) area. In medium severity raveling, there is some FOD potential.

High severity occurs if any one of these conditions exist: (1) In a square yard (square meter) representative area, the number of coarse aggregate particles missing is over 40. (2) Missing aggregate clusters is more than 10 percent of the examined square yard (square meter) area. In high severity raveling, there is significant FOD potential.

Note-this is a new distress since the 2007 survey



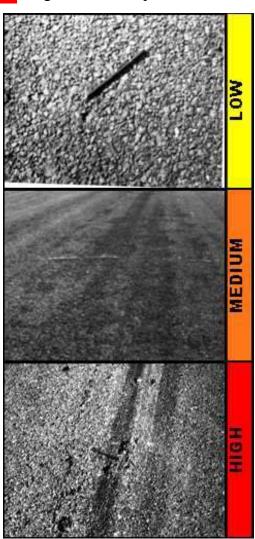
Slurry Seal/ Coal Tar Over Dense Mix Severity Levels



- (1) The scaled area is less than 1 percent. (2) In the case of coal tar where pattern cracking has developed, the surface cracks are less than 1/4 inch (6 mm) wide.
- (1) The scaled area is between 1 and 10 percent. (2) In the case of coal tar where pattern cracking has developed, the cracks are 1/4 inch (6 mm) wide or greater.
- (1) The scaled area is over 10 percent. (2) In the case of coal tar the surface is peeling off.

# Porous Friction Course Severity Levels

- In a 1 square foot (1/10 square meter) representative sample, the number of aggregate pieces missing is between 5 and 20 and/ or the number of missing aggregate clusters does not exceed 1.
- In a 1 square foot (1/10 square meter) representative sample, the number of aggregate pieces missing is between 21 and 40 and/ or the number of missing aggregate clusters is greater than 1 but does not exceed 25 percent of the area.
- In a 1 square foot (1/10 square meter) representative sample, the number of aggregate pieces missing is over 40 and/ or the number of missing aggregate clusters is greater than 25 percent of the area.



### 13. Rutting (AC)

A rut is a surface depression in the wheel path; however, in many instances ruts are noticeable only after a rainfall, when the wheel paths are filled with water. Pavement uplift may occur along the sides of the rut. Rutting stems from a permanent deformation in any of the pavement layers or sub-grade, usually caused by consolidation or lateral movement of the materials due to traffic loads. Significant rutting can lead to major structural failure of the pavement.

# Severities (based on rut depth):

- ◆ Low less than ½ inch in depth;
- ♦ Medium between ½ and 1 inch in depth;
- High exceeds 1 inch in depth.

- ♦ Low No action;
- Medium patch and/or overlay;
- ♦ High patch and/or overlay.

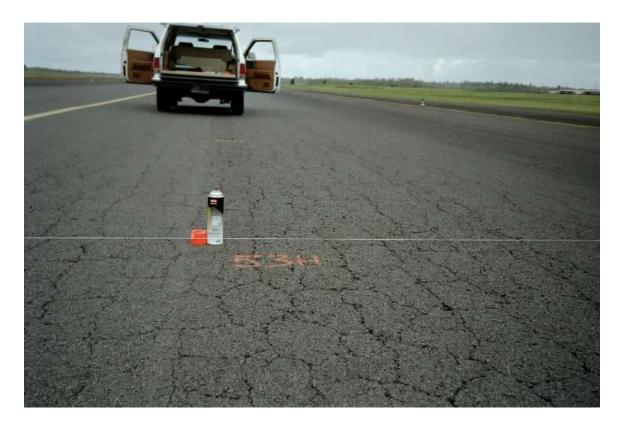


Figure C.9: AC Rutting.

# 14. Slippage Cracking (AC)

Slippage cracks are crescent- or half-moon shaped cracks having two ends pointed away from the direction of traffic. They are produced when braking or turning wheels cause the pavement surface to slide and deform. This usually occurs when there is a low-strength surface mix or poor bond between the surface and next layer of pavement structure.

<u>Severities</u>: No degrees of severity are defined. It is sufficient to indicate that a slippage crack exists.

- Do nothing;
- Partial or full depth patch.



Figure C.10: Slippage Cracking.

### 15. Swelling (AC)

### Description

A swell is characterized by an upward bulge in the pavement's surface. A swell may occur sharply over a small area or as a longer, gradual wave. Either type of swell can be accompanied by surface cracking. A swell is usually caused by frost action in the subgrade or by swelling soil, but a small swell can also occur on the surface of an asphalt overlay (over PCC) as a result of a blow- up in the PCC slab.

- Swell is barely visible and has a minor effect on the pavement's ride quality as determined at the normal aircraft speed for the pavement section under consideration. (Low-severity swells may not always be observable, but their existence can be confirmed by driving a vehicle over the section at the normal aircraft speed. An upward acceleration will occur if the swell is present).
- Swell can be observed without difficulty and has a significant effect on the pavement's ride quality as determined at the normal aircraft speed for the pavement section under consideration.
- Swell can be readily observed and severely affects the pavement's ride quality at the normal aircraft speed for the pavement section under consideration.

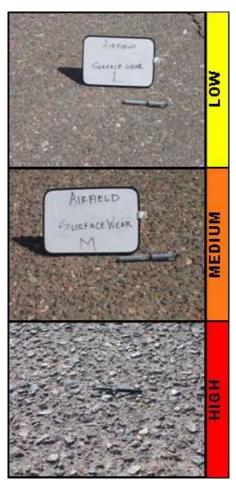


### 16. Weathering (AC)

### Description

The wearing away of the asphalt binder and fine aggregate matrix from the pavement surface.

- Asphalt surface beginning to show signs of aging which may be accelerated by climatic conditions. Loss is the fine aggregate matrix is noticeable and may be accompanied by fading of the asphalt color. Edges of the coarse aggregates are beginning to be exposed (less than 0.05 inches or 1 mm). Pavement may be relatively new (as new as 6 months old).
- Loss of fine aggregate matrix is noticeable and edges of coarse aggregate have been exposed up to 1/4 width (of the longest side) of the coarse aggregate due to the loss of fine aggregate matrix.
- Edges of coarse aggregate have been exposed greater than 1/4 width (of the longest side) of the coarse aggregate. There is considerable loss of fine aggregate matrix leading to potential or some loss of coarse aggregate.

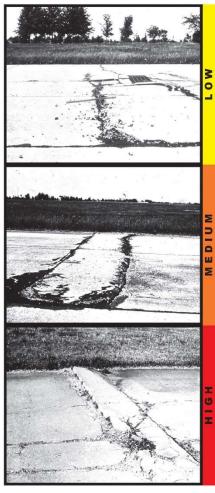


### 17. Blow-Up (PCC)

### Description

Blowups occur in hot weather, usually at a transverse crack or joint that is not wide enough to permit expansion by the concrete slabs. The insufficient width is usually caused by infiltration of incompressible materials into the joint space. When expansion cannot relieve enough pressure, a localized upward movement of the slab edges (buckling) or shattering will occur in the vicinity of the joint. Blowups can also occur at utility cuts and drainage inlets. This type of distress is almost always repaired immediately because of severe damage potential to aircraft. Blowups are included for reference when closed sections are being evaluated for reopening.

- Buckling or shattering has not rendered the pavement inoperative, and only a slight amount of roughness exists.
- **M** Buckling or shattering has not rendered the pavement inoperative, but a significant amount of roughness exists.
- **H** Buckling or shattering has rendered the pavement inoperative.



### 18. Corner Breaks (PCC)

A corner break is a crack that intersects the joints at a distance less than or equal to one-half the slab length on both sides, measured from the corner of the slab. For example, a slab with dimensions of 25 by 25 feet that has a crack intersecting the joint 5 feet from the corner on one side and 17 feet on the other side is not considered a corner break; it is a diagonal crack. However, a crack that intersects 7 feet on one side and 10 feet on the other is considered a corner break. A corner break differs from a corner spall in that the crack extends vertically through the entire slab thickness, while a corner spall intersects the joint at an angle. Load repetition combined with loss of support and curling stresses usually causes corner breaks.

#### Severities:

- ♦ Low Crack has either no spalling or minor spalling (no foreign object damage (FOD) potential). If non-filled, it has a mean width less than approximately 1/8 inch (3 millimeters); a filled crack can be of any width, but the filler material must be in satisfactory condition. The area between the corner break and the joints is not cracked;
- ♦ Medium One of the following conditions exists: (1) filled or non-filled crack is moderately spalled (some FOD potential); (2) a non-filled crack has a mean width between 1/8 inch (3 millimeters) and 1 inch (25 millimeters); (3) a filled crack is not spalled or only lightly spalled, but the filler is in unsatisfactory condition; (4) the area between the corner break and the joints is lightly cracked with loose or missing particles;
- ♦ High One of the following conditions exists: (1) filled or non-filled crack is severely spalled, causing definite FOD potential; (2) a non-filled crack has a mean width greater than approximately 1 inch (35 millimeters), creating a tire damage potential; or (3) the area between the corner break and the joints is severely cracked.

### Repair options:

- Low No action or seal cracks;
- Medium seal cracks;
- High seal cracks, apply a full or replace the slab.



depth patch

Figure C.11: PCC Corner Break.

### 19. Cracks: Longitudinal, Transverse and Diagonal (PCC)

These cracks divide the slab into two or three pieces, and are usually caused by a combination of load repetition, curling stresses, and shrinkage stresses. Low severity cracks are not considered major structural distresses. Medium or high severity cracks are usually working cracks and are considered major structural distresses.

### Severities:

- ◆ Low 1) unfilled cracks 1/4 inch to 1/2 inch wide with no faulting or spalling; 2) cracks less than 1/2 inch wide with low severity spalling; or 3) filled cracks of any width, with filter performing in a satisfactory manner and no faulting or spalling;
- Medium 1) un-filled cracks between 1/2 to 1 inch wide with no faulting or spalling or 2) filled cracks of any width faulting less than 1/8 inch or medium severity spalling;
- ♦ High 1) un-filled cracks with a width greater than 1 inch; 2) un-filled cracks of any width with faulting greater than 1/2 inch or medium severity faulting; or 3) filled cracks of any width faulting greater than 1/2 inch or high severity faulting.

- Low No action or seal cracks;
- Medium seal cracks;
- High seal cracks, apply a full depth patch or replace the slab.



Figure C.12: PCC Transverse Cracks.

## 20. Durability Cracks (PCC)

### Description

Durability cracking is caused by the inability of the concrete to withstand environmental factors such as freeze-thaw cycles. It usually appears as a pattern of cracks running parallel to a joint or linear crack. A dark coloring can usually be seen around the fine durability cracks. This type of cracking may eventually lead to disintegration of the concrete within 1 to 2 feet (300 to 600 millimeters) of the joint or crack.

- "D" cracking is defined by hairline cracks occurring in a limited area of the slab, such as one or two corners or along one joint. Little or no disintegration has occurred. No FOD potential.
- (1) "D" cracking has developed over a considerable amount of slab area with little or no disintegration or FOD potential; or (2) "D" cracking has occurred in a limited area of the slab, such as in one or two corners or along one joint, but pieces are missing and disintegration has occurred. Some FOD potential.
  - "D" cracking has developed over a considerable amount of slab area with disintegration of FOD potential.



### 21. Joint Seal Damage (PCC)

Joint seal damage is any condition, which enables soil or rocks to accumulate in the joints or allow significant infiltration of water. Accumulation of incompressible materials in the joint prevents the slab from expanding and may result in buckling, shattering, or spalling. Pliable joint filler bonded to the edges of the slabs protects joints from the accumulation of materials and also prevents water from seeping down and softening the foundation supporting the slab. Typical types of joint seal damage are: 1) stripping the joint sealant; 2) extrusion of joint sealant; 3) weed growth; 4) hardening of the filler; 5) loss of bond to the slab edges; and 6) lack or absence of sealant in the joint.

#### Severities:

- Low in generally good condition throughout the section. Sealant is performing well with only a minor amount of any of the above types of damage present;
- Medium in generally fair condition throughout the section, with one or more of any of the above types of damage present occurring to a moderate degree.
   Sealant needs immediate replacement within 2 years;
- High in generally poor condition throughout the section, with one or more of any of the above types of damages present, occurring to a severe degree. Sealant needs immediate replacement.

- Low No action;
- Medium seal joints;
- High seal joints.



Figure C.13: PCC Joint Seal Damage.

### 22. Small Patch (PCC)

A patch is an area where the original pavement has been removed and replaced by a filler material. For condition evaluation, patching is divided into two types: small (less than 5 square feet) and large (over 5 square feet). Large patches are described in the next section.

### Severities:

- Low Patch is functioning well, with little or no deterioration;
- Medium Patch has deteriorated, and/or moderate spalling can be seen around the edges. Patch material can be dislodged, with considerable effort (minor FOD potential);
- High Patch has deteriorated, either by spalling around the patch or cracking within the patch, to a state which warrants replacement.

- ◆ Low Do Nothing;
- Medium Replace patch or replace the slab:
- ♦ High Replace patch or replace the slab.



Figure C.14: PCC Small Patch.

### 23. Large Patch (PCC)

Patching is the same as defined for a small patch; however, the area of the patch is more than 5 square feet. A utility cut is a patch that has replaced the original pavement because of placement of underground utilities. The severity levels of a utility cut are the same as those for regular patching.

### Severities:

- Low Patch is functioning well, with little or no deterioration;
- Medium Patch has deteriorated, and/or moderate spalling can be seen around the edges. Patch material can be dislodged, with considerable effort (minor FOD potential);
- High Patch has deteriorated, either by spalling around the patch or cracking within the patch, to a state which warrants replacement.

- ◆ Low Do Nothing;
- Medium Replace patch or replace the slab;
- High Replace patch or replace the slab.



Figure C.15: PCC Large Patch.

## 24. Popouts (PCC)

A popout is a small piece of pavement that breaks loose from the surface due to freezethaw action in combination with expansive aggregates. Popouts usually range from approximately 1 inch to 4 inches in diameter and from 1/2 inch to 2 inches deep..

#### Severities:

No degrees of severity are defined for popouts. However, popouts must be extensive before they are counted as a distress; i.e., average popout density must exceed approximately three popouts per square yard over the entire slab area.



Figure C.16: Popouts.

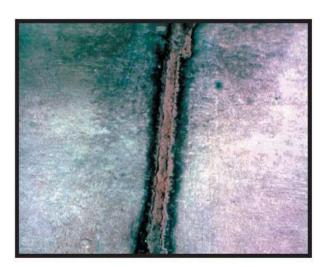
#### 25. Pumping (PCC)

#### **Description**

Pumping is the ejection of material by water through joints or cracks caused by deflection of the slab under passing loads. As the water is ejected, it carries particles of gravel, sand, clay, or silt and results in a progressive loss of pavement support. Surface staining and base or subgrade material on the pavement close to joints or cracks are evidence of pumping. Pumping near joints indicates poor joint sealer and loss of support which will lead to cracking under repeated loads.

#### Severity Levels

No degrees of severity are defined. It is sufficient to indicate that pumping exists.



#### 26. Scaling (PCC)

Map cracking or crazing refers to a network of shallow, fine, or hairline cracks that extend only through the upper surface of the concrete. The cracks tend to intersect at angles of 120 degrees. Map cracking or crazing is usually caused by over finishing the concrete and may lead to scaling of the surface, which is the breakdown of the slab surface to a depth of approximately 1/4 to 1/2 inch. Scaling may also be caused by improper construction and poor aggregate. Another recognized source of distress is the reaction between the alkalis (Na2O and K2O) in some cements and certain minerals in some aggregates. Products formed by the reaction between the alkalis and aggregate result in expansions that cause a breakdown in the concrete.

#### Severities:

- Low Crazing or map cracking exists over significant slab area. The surface is in good condition with no scaling. The crack pattern must be well defined and easily recognized;
- ♦ Medium Slab is scaled over approximately 5% or less of the surface with some FOD potential;
- High Slab is severely scaled causing a high FOD potential. Usually, more than 5% of the surface is affected.



## 27. Faulting (PCC)

Settlement or faulting is a difference of elevation at a joint or crack caused by upheaval or consolidation.

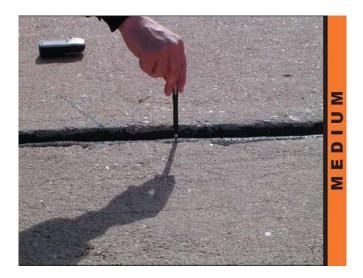
#### Severities:

Severity levels are defined by the difference in elevation across the fault and the associated decrease in ride quality and safety as severity increases.

	Runways/Taxiways	Aprons
L	< 1/4 inch	1/8 – 1/2 inch
M	1/4 - 1/2 inch	1/2 - 1 inch
Н	> 1/2 inch	> 1 inch

## Repair Options:

- ♦ Low No action;
- Medium Grinding along the joint;
- High Grinding or joint load transfer restoration.



#### 28. Shattered Slab (PCC)

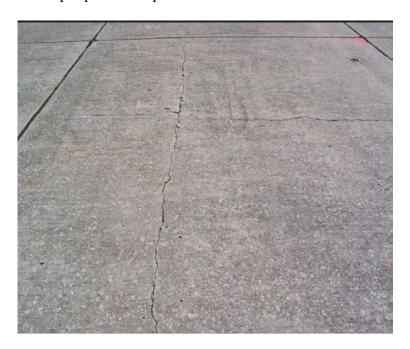
Intersecting cracks are cracks that break into four or more pieces because of overloading and/or inadequate support. The high-severity level of this distress type, as defined below, is referred to as a shattered slab. If all pieces or cracks are contained within a corner break, the distress is categorized as a severe corner break.

#### Severities:

- Low Slab is broken into four or five pieces with the vast majority of the cracks (over 85 percent) of low-severity;
- Medium (1) Slab is broken into four or five pieces with over 15 percent of the cracks of medium severity (no high-severity cracks); or (2) slab is broken into six or more pieces with over 85 percent of the cracks of low-;
- ♦ High At this level of severity, the slab is called shattered: (1) slab is broken into four or five pieces with some or all of the cracks of high severity; (2) slab is broken into six or more pieces with over 15 percent of the cracks of medium- or high-severity.

#### Repair options:

- ♦ Low Seal Cracks;
- Medium Full depth patch or replace the slab;
- High Full depth patch or replace the slab.



## 29. Shrinkage Crack (PCC)

Shrinkage cracks are hairline cracks that are usually only a few feet long and do not extend across the entire slab. They are formed during the setting and curing of the concrete and usually do not extend through the depth of the slab.

#### Severities:

No degrees of severity are defined. It is sufficient to indicate that shrinkage cracks exist.

## Repair options:

Do Nothing



#### 30. Joint Spalls (PCC)

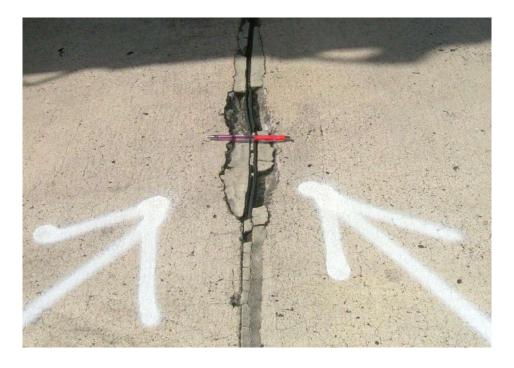
Joint spalling is the disintegration of the slab edges within 2 feet of the side of the joint. A joint spall usually does not extend vertically through the slab, but intersects the joint at an angle. Spalling results from excessive stresses at the joint crack caused by infiltration of incompressible materials or traffic loads. Weak concrete at the joint (caused by overworking) combined with traffic loads is another cause of spalling.

#### Severities:

- Low over 2 feet long and is broken into no more than three pieces defined by low or medium severity cracks, with little or no FOD potential, or is 2) less than 2 feet long and is broken into more than three pieces, with little FOD or tire damage potential;
- Medium over 2 feet long and is broken into more than 3 pieces defined by light or medium cracks or some FOD potential existing, or is 2) less than 2 feet long and is broken into pieces or fragmented, with some of the pieces loose or absent, causing considerable FOD or tire damage potential;
- High over 2 feet long and is broken into more than three pieces defined by one or more high severity cracks with high FOD potential.

#### Repair Options:

- Low No action;
- Medium perform a partial depth patch;
- High perform a partial depth patch.



#### 31. Corner Spalls (PCC)

Corner spalling is the raveling or breakdown of the slab within approximately 2 feet of the corner. A corner spall differs from a corner break in that the spall angles downward to intersect the joint while the break extends vertically through the slab.

#### Severities:

- Low either 1) the spall is broken into one or two pieces defined by low severity cracks with little or no FOD potential; or 2) the spall is defined by one medium severity crack with little or no FOD potential;
- ♦ Medium 1) the spall is broken into two or more pieces defined by medium severity cracks, and a few small fragments may be absent or loose; 2) the spall is defined by one severe, fragmented crack that may be accompanied by a few hairline cracks or 3) the spall has deteriorated to the point where loose material is causing FOD potential;
- ♦ High 1) the spall has broken into two or more pieces defined by high severity fragmented cracks, with loose or absent fragments; 2) pieces of the spall have been displaced to the extent that tire damage hazard exists, or 3) the spall has deteriorated to the point where loose material is causing high FOD potential.

#### Repair Options:

- ♦ Low No action;
- Medium partial depth patch;
- High partial depth patch.



#### 32. ASR (PCC)

ASR is caused by chemical reaction between alkalis and certain reactive silica minerals which form a gel. The gel absorbs water, causing expansion which may damage the concrete and adjacent structures. Alkalis are most often introduced by the portland cement within the pavement. ASR cracking may be accelerated by chemical pavement deicers.

Visual indicators that ASR may be present include:

- 1. Cracking of the concrete pavement (often in a map pattern)
- 2. White, brown, gray or other colored gel or staining may be present at the crack surface
- 3. Aggregate popouts
- 4. Increase in concrete volume (expansion) that may result in distortion of adjacent or integral structures or physical elements. Examples of expansion include shoving of asphalt pavements, light can tilting, slab faulting, joint misalignment, and extrusion of joint seals or expansion joint fillers.

Because ASR is material-dependent, ASR is generally present throughout the pavement section. Coring and concrete petrographic analysis is the only definitive method to confirm the presence of ASR. The following should be kept in mind when identifying the presence of ASR through visual inspection:

- 1. Generally ASR distresses are not observed in the first few years after construction. In contrast, plastic shrinkage cracking can occur the day of construction and is apparent within the first year.
- 2. ASR is differentiated from D-Cracking by the presence of cracking perpendicular to the joint face. D-Cracking predominantly develops as a series of parallel cracks to joint faces and linear cracking within the slab.
- 3. ASR is differentiated from Map Cracking/ Scaling by the presence of visual signs of expansion.

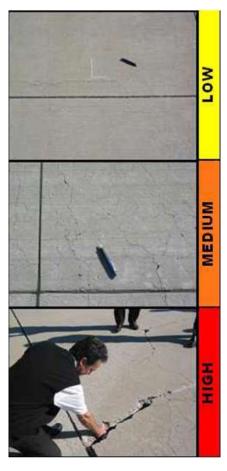
#### Severity Levels

Minimal to no Foreign Object Damage (FOD) potential from cracks, joints or ASR related popouts; cracks at the surface are tight (predominantly 1 mm or less). Little to no evidence of movement in pavement or surrounding structures or elements.

Some FOD potential; increased sweeping or other FOD removal methods may be required. May be evidence of slab movement and/ or some damage to adjacent structures or elements.

Medium ASR distress is differentiated from low by having one or more of the following: increased FOD potential, increased cracking of the slab, some fragments along cracks or at crack intersections present, surface popouts of concrete may occur, pattern of wider cracks (predominantly 1 mm or wider) that may be subdivided by tighter cracks.

One or both of the following exist: 1) Loose or missing concrete fragments which pose high FOD potential, 2) Slab surface integrity and function significantly degraded and pavement requires immediate repair; may also require repairs to adjacent structures or elements.





#### Re Inspection Report

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43	BLOCK CRAC	CKING		M	(	5988.00	SqFt							
52	DAVELING			M		2400 00	CaEt							

6495.00 SqFt

**PCI:** 27

52

48

52

57

RAVELING

RAVELING

WEATHERING

Sample Number: 09

**Sample Comments** L & T CR

WEATHERING

M

Н

M

M

Η

R

Туре

3499.00 SqFt

3499.00 SqFt

600.00 Ft

3247.00 SqFt

3247.00 SqFt

Area

Brar	rch	A01			N	ne	Apror	01 Oz	ark	Use	APRON	1	A	rea	131,310 SqFt	
Secti	<b>ion</b> 0	1		of 2	2	]	Fran	Edge o	of Pavement		То	Secti	on 02		LastConst	1/1/2005
Surf	ace A	C	Family	A A	LDO	T_Apro	ons	Zo	ne		Cat	egory:			Rank S	
Area	E .	,	78,123 SqFt		I	ength		392	Ft	Width		291 F	t			
Slab	8		Slabl	Length	1		Ft		SlabWidth		Ft			Joint Length	<b>ı</b> 1	Ft
Sha	lder:		Street	t <b>Type</b>	•				Grade 0					Lanes 0		
Secti	anCan	ments														
Worl	kDate	1/1/2005		Work	Тур	e New	Constructi	on - Ini	tial	(	Code NU	-IN		IsMajor	MRR True	
Last	Insp D	ate: 11/4	/2019			Totals	amples	15		Survey	ed 5					
Can	itions	PCI:	66													
Insp	ection(	Connents														
San	pleNum	mber: 01	7	Гуре		R		Area	524	4.00 SqFt		PCI:	65			
		ments														
48	L&7	CR			M		350.00	Ft								
57		THERING			L		5244.00									
Sanj	pleNum	<b>ber:</b> 03	ר	Гуре		R	1	Area	677	6.00 SqFt		PCI:	70			
Sang	pleCon	nerts														
48	L & 7				L		420.00									
48	L&T				M		210.00									
57		THERING			L	D	6776.00		500	0.00 G E		TV-T	67			
	_	mber: 07	1	Гуре		R	4	Area	5000	0.00 SqFt		PCI:	6/			
Sanj	pleCan	ments														
48	L & 7				L		200.00									
48 57	L&T WFA	CR THERING			M L		200.00 5000.00									
		riber: 11		Farm		R		Area	500	0.00 SqFt		PCI:	62			
		ments	1	Гуре		1	1	-1CH	5000	o.oo sqrt		ru:	02			
	-						100.00	Е.								
48 48	L&7 L&7				L M		100.00 200.00									
<del>4</del> 0 57		THERING			L		3750.00									
57		THERING			M		1250.00	-								
		<b>rber:</b> 13		Гуре		R		Area	500	0.00 SqFt		PCI:	66			
		ments								•						
48	L & 7	CR			L		100.00	Ft								
48	L & 1				M		150.00									
57		THERING			L		3750.00									
57	WEA	THERING			M		1250.00	SqFt								

Ozark Airport - Blackwell Field

Nane

71J

Network

Brar	<b>xh</b> A02			Name	Apron	02 Oza	rk	Use	APRON	Area	2	48,373 SqFt	
Secti	<b>can</b> 01		of 1		From	Taxiwa	y C		<b>To</b> Edge	e of Pavement		Last Const:	1/1/1956
Surf	ace AC	Fanily:	ΑI	DOT_Ap	orons	Zon	æ		Category:			Rank S	
Area	8	48,373 SqFt		Lengt	h	324 F	it .	Width	158 F	`t			
Slab	8	SlabL	ength	l	Ft		SlabWidth		Ft	Je	intLength	F	t
Show	kder:	Street	Туре				Grade 0			L			
Secti	anCaments												
Worl	<b>kDate</b> 1/1/195	56 <b>T</b>	Work	Type No	ew Construction	on - Initi	ial	C	Code NU-IN		IsMajorN	<b>RR</b> True	
Last	Insp Date 11	1/4/2019		Tota	alSamples	9		Survey	edt 4				
Can	iltions PCI:	62											
Insp	ectionCommen	<b>ts</b>											
Sanj	pleNumber: (	)1 <b>T</b>	ype	R		Area	4747	7.00 SqFt	PCI:	60			
Sanj	pleComents												
48	L & T CR			L	122.00	Ft							
48	L & T CR			M	320.00	Ft							
57	WEATHERIN	NG		L	4747.00	SqFt							
Sanj	pleNumber: (	)3 <b>T</b>	ype	R	ı	Area	5000	0.00 SqFt	PCI:	72			
Sanj	pleConnents												
48	L & T CR			L	250.00	Ft							
48	L & T CR			M	135.00	Ft							
57	WEATHERIN	NG		L	5000.00	SqFt							
Sanj	pleNumber: (	)5 <b>T</b>	ype	R	1	Area	5000	0.00 SqFt	PCI:	55			
Sanj	pleComments												
48	L & T CR			L	241.00	Ft							
48	L & T CR			M	450.00	Ft							
57	WEATHERIN	NG		L	5000.00	SqFt							
Sanj	pleNumber: (	)9 <b>T</b>	ype	R	1	Area	6162	2.00 SqFt	PCI:	63			
Sanj	pleComents												
48	L & T CR			L	400.00	Ft							
48	L & T CR			M	350.00	Ft							
57	WEATHERIN	NG		L	6162.00	SqFt							

Nane

Ozark Airport - Blackwell Field

71J

Network

Netw	<b>orks</b> 71J					Na	ne O:	zark Airport	- Black	well Fi	eld				
Bran	<b>da</b> A03			Nane	Aŗ	oron 03 Oz	ark	Us	e A	PRON	A	Area	80,00	00 SqFt	
Sectio	<b>11</b> 01		of 1		From	Taxiw	ay B			То	Edge of Pav	ement	L	est Const:	1/2/2016
Surfa	ce AC	Famil	y. Al	LDOT_A	prons	Zo	ne			Categ	pry:		R	ande S	
Area		80,000 SqFt		Leng	th	400	Ft	Width		2	00 Ft				
Slabs	:	Slab	Length	1		Ft	SlabWidth	1		Ft		Joint I	ægh	Ft	
Shoul	der:	Stree	tType	3			Grade	0				Lanes	0		
Sectio	nComents														
Work	Date 1/1/2016		Work	Туре В	ase Course	- Aggrega	ite		Code	BA-A	<b>.</b> G	Is	Majar M&I	<b>&amp;</b> False	
Work	<b>Date:</b> 1/2/2016	· •	Work	Type N	ew Constru	uction - In	tial		Code	NU-I	N	Is	Majar M&I	<b>t</b> True	
Lastl	Insp Date 11/	4/2019		Tat	alSample	16		Surv	eyed	5					
Cand	itions PCI:	94													
Inspe	ctionConnent	s													
Samp	leNumber: 01		Туре	R		Area	50	00.00 SqFt		1	<b>PCI:</b> 94				
Samp	leConnents														
57	WEATHERING	G		L	5000	.00 SqFt									
Samp	leNumber: 07	7	Туре	R		Area	50	00.00 SqFt		1	<b>PCI:</b> 94				
Samp	leComents														
57	WEATHERIN	G		L	5000	.00 SqFt									
Samp	leNumber: 09	)	Туре	R		Area	50	00.00 SqFt		1	<b>PCI:</b> 94				
Samp	leComents:														
57	WEATHERING	G		L	5000	.00 SqFt									
Samp	leNunber: 10	)	Type	R		Area	50	00.00 SqFt		1	<b>PCI:</b> 94				
Samp	leComents														
57	WEATHERING	G		L	5000	.00 SqFt									
Samp	leNumber: 16	j	Туре	R		Area	50	00.00 SqFt		1	<b>PCI:</b> 94				
Samp	leComents														
57	WEATHERIN			L	5000	.00 SqFt									

Netv	vark 71J				Name	Ozar	k Airport -	Black	well Field				
Brar	rch A04		Nane	Apron	04 Ozark		Use	AI	PRON	Area	122	2,760 SqFt	
Secti	<b>cn</b> 01	of 1		From	Гахіway А	1			To Edge	of Pavemer	nt	Last Const:	10/2/2008
Surf	acce AAC	Family: A	LDOT_Ap	orons	Zore				Category:			Rank S	
Area	122,76	0 SqFt	Lengt	h	660 Ft		Width		186 Ft	:			
Slab	8	SlabLength	1	Ft	S	labWidth			Ft		JointLength	F	t
Sha	lder:	Street Type	1		G	trade 0				]	Lanes 0		
Secti	anCaments												
Worl	<b>kDate</b> 1/1/1956	Work	<b>Type</b> Ne	ew Constructio	n - Initial			Code	NU-IN		Is <b>Maj</b> arM	RR True	
Worl	<b>kDate</b> 10/1/2008	Work	<b>Type</b> Su	rface Treatmen	nt - Double	e Bitum.		Code	LC-DB		Is <b>Maj</b> arM	<b>RR</b> False	
War	<b>kDate</b> 10/2/2008	Work	<b>Type</b> Ov	erlay - AC Th	in			Code	OL-AT		Is <b>Maj</b> arM	RR True	
Last	<b>Insp Date</b> 11/4/2019	1	Tota	Samples	28		Surve	yed:	5				
	itions PCI: 68			_			•						
Insp	ectionComments												
Sam	pleNumber: 01	Туре	R	A	rea	5000	.00 SqFt		PCI:	68			
	pleComents	-31											
48	L & T CR		L	200.00	Ft								
48	L & T CR		M	189.00	Ft								
57	WEATHERING		L	5000.00	SqFt								
Sanj	<b>pleNumber:</b> 03	Туре	R	A	rea	5000	.00 SqFt		PCI:	74			
Sanj	pleComments												
48	L & T CR		L	200.00	Ft								
48	L & T CR		M	100.00									
57	WEATHERING		L	5000.00	SqFt								
-	<b>pleNumber:</b> 10	Type	R	A	nes.	5000	.00 SqFt		PCI:	67			
Sanj	pleComents												
48	L & T CR		L	156.00	Ft								
48	L & T CR		M	200.00	Ft								
57	WEATHERING		L	5000.00	SqFt								
Sanj	<b>pleNumber:</b> 19	Туре	R	A	rea	5000	.00 SqFt		PCI:	66			
Sanj	pleComents												
48	L & T CR		L	125.00	Ft								
48	L & T CR		M	225.00	Ft								
57	WEATHERING		L	5000.00	SqFt								
Sang	<b>pleNumber:</b> 21	Туре	R	A	nea	5000	.00 SqFt		PCI:	67			
Sanj	pleComments												
48	L & T CR		L	230.00	Ft								
48	L & T CR		M	200.00	Ft								
57	WEATHERING		L	5000.00	SqFt								

Network 71J			Nane	Ozark Airport - Bl	ackwell Field	
Branch R1331		Name	Runway 13-31 Ozark	Use	RUNWAY Ar	412,160 SqFt
Section 01	of 1	]	From Runway 13 End	l	To Runway 31 Er	nd <b>Last Const.:</b> 1/1/2005
Surface AC Fa	nily: ALI	DOT_RWs	Zœ		Category:	Rank P
<b>Area</b> 412,160 Sq	Ft	Length	5,152 Ft	Width	80 Ft	
Slabs Sl	abLength		Ft <b>Slab V</b>	<b>Vicith</b>	Ft	<b>Joint Length</b> Ft
Strukke: St	reet Type		Grade	0		Lates 0
SectionComments:						
<b>WorkDate</b> 1/1/2005	Work	Type New	Construction - Initial	Co	de NU-IN	IsMejorMRR: True
Last Irop Date 11/4/2019		TotalS	amples 89	Surveyed	11	
Carditions PCI: 73						
InspectionConnents						
SampleNumber: 03	Туре	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
SampleComments						
48 L & T CR	]	L	375.00 Ft			
57 WEATHERING		M	6000.00 SqFt			
Sample Number: 10	Туре	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
Sample Comments						
48 L & T CR		L	375.00 Ft			
57 WEATHERING		M	6000.00 SqFt			
Sample Number: 17	Type	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
SampleComments						
48 L&TCR		L	375.00 Ft			
57 WEATHERING		M 	6000.00 SqFt	6000 00 S-E4	TOCK: 75	
SampleNumber: 24 SampleComments:	Type	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
48 L&TCR	1	L	375.00 Ft			
57 WEATHERING		M	6000.00 SqFt			
SampleNumber: 31	Туре	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
SampleComments						
48 L & T CR	]	L	375.00 Ft			
57 WEATHERING		M	6000.00 SqFt			
<b>SampleNumber:</b> 38	Type	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
SampleConnents						
48 L & T CR		L	375.00 Ft			
57 WEATHERING		M 	6000.00 SqFt	(000 00 G F:	TVT - 21	
SampleNumber: 45	Type	R	Area	6000.00 SqFt	<b>PCI:</b> 71	
SampleConnents						
48 L & T CR 48 L & T CR		L M	450.00 Ft 5.00 Ft			
57 WEATHERING		M	6000.00 SqFt			
SampleNumber: 52	Туре	R	Area	6000.00 SqFt	<b>PCI:</b> 75	
SampleComments						
48 L & T CR	]	L	375.00 Ft			
57 WEATHERING		М	6000.00 SqFt			
<b>SampleNumber:</b> 53	Type	A	Area	6000.00 SqFt	<b>PCI:</b> 70	
SampleConnents						
48 L & T CR	]	L	336.00 Ft			
50 PATCHING 57 WEATHERING		L M	689.00 SqFt			
57 WEATHERING	Туре	R R	5311.00 SqFt <b>Area</b>	6000.00 SqFt	<b>PCI:</b> 70	
<b>Sample Number:</b> 59						

48	L & T CR	M	30.00 Ft			
57	WEATHERING	M	6000.00 SqFt			
San	pleNumber: 66 Type	R	Area	6000.00 SqFt	<b>PCI:</b> 70	
San	pleConnents					
48	LONGITUDINAL/TRANSVERSE CRACKING	E L	300.00 Ft			
48	L & T CR	M	30.00 Ft			

L 450.00 Ft

M 6000.00 SqFt

48

57

L & T CR

WEATHERING

twork 71J		Nane	Ozark Airport - Bl	ackwell Field	
and TA	Name	Taxiway A Ozark	Use	TAXIWAY Area	188,480 SqFt
otion 01	of 1 <b>F</b>	Runway 13 End	!	<b>To</b> Runway 31 End	<b>Last Const.:</b> 1/1/2005
nface AC	Family: ALDOT_AC Ta	axiways <b>Zone</b>		Category:	Rank P
<b>188</b> ,	,480 SqFt <b>Length</b>	5,421 Ft	Width	35 Ft	
bs	SlabLength	Ft <b>Slab V</b>	Vidth	Ft	<b>Joint Leigh</b> Ft
culder:	Street Type	Grade	• 0		Lanes 0
dianCaments					
<b>Cork Date</b> 1/1/2005	WorkType New C	Construction - Initial	Co	de NU-IN	IsMejorMRR: True
<b>st Insp Date</b> 11/4/20	019 TotalSa	mples 36	Surveyed	1 7	
nditions PCI: 78	8				
spectionComments					
<b>npleNumber:</b> 01	<b>Type</b> R	Area	5250.00 SqFt	<b>PCI:</b> 75	
mpleConnents					
L & T CR	L	124.00 Ft			
WEATHERING	M	5250.00 SqFt			
<b>npleNumber:</b> 05	<b>Type</b> R	Area	5250.00 SqFt	<b>PCI:</b> 79	
mpleComments					
L & T CR	L	150.00 Ft			
L & T CR	M	50.00 Ft			
WEATHERING	L	5250.00 SqFt			
<b>npleNumber:</b> 12	<b>Type</b> R	Area	5250.00 SqFt	<b>PCI:</b> 85	
mpleComments:					
L & T CR	L	150.00 Ft			
WEATHERING	L	5250.00 SqFt			
<b>npleNumber:</b> 19	<b>Type</b> R	Area	5250.00 SqFt	<b>PCI:</b> 81	
mpleComments					
L & T CR	L	250.00 Ft			
WEATHERING	L	5250.00 SqFt			
<b>npleNumber:</b> 28	<b>Type</b> R	Area	5250.00 SqFt	<b>PCI:</b> 82	
mpleConnents					
L & T CR	L	125.00 Ft			
L & T CR	M	12.00 Ft			
WEATHERING	L	5250.00 SqFt	5250 00 G E:		
mpleNumber: 33	<b>Type</b> R	Area	5250.00 SqFt	<b>PCI:</b> 71	
mpleComments					
L & T CR	L	100.00 Ft			
		-	5250 00 SaEt	<b>DCT.</b> 71	
_	iype r	ALE	5250.00 SqFt	<b>FG:</b> /1	
_					
	L				
		_			
L & T CR WEATHERING  IIII 36  IIIII 36  IIIII Connects	M L Type R	100.00 Ft 150.00 Ft 5250.00 SqFt  ATEX  277.00 Ft 3.00 Ft 3.410.00 SqFt 1840.00 SqFt	5250.00 SqFt	<b>PCI:</b> 71	

Network 71J			Nane	Ozark Airport - l	Blackwell Field		
Branch TA1		Nane	Taxiway A1 Oza	ark <b>Use</b>	TAXIWAY	Area	11,139 SqFt
Section 01	of	1	From Runway 1	3-31	To Taxiway A		<b>Last Const.</b> : 1/1/2005
Surface AC	Family:	ALDOT_AC	Taxiways <b>Zone</b>		Category:		Rank S
Area	11,139 SqFt	Length	184 Ft	Width	42 Ft		
Slabs	SlabLen	gth	Ft S	labWidth	Ft	Joint Length	Ft
Shoulder:	Street Ty	ype	C	<b>Grade</b> 0		Lates 0	
SectionComments:							
<b>WorkDate</b> 1/1/2005	5 <b>V</b> V	trkType New	Construction - Initial		Code NU-IN	Is <b>Maj</b> ar I	<b>MRR</b> True
Last Insp Date 11	/4/2019	Total	Samples 2	Survey	erk 2		
Conditions PCI:			<b>-</b>				
Inspection Connen							
SampleNumber: 0		ne R	Area	4989.00 SqFt	<b>PCI:</b> 70		
Sample Comments	<b>J1</b>			1			
- 18 L&TCR		L	180.00 Ft				
18 L&TCR		M	31.00 Ft				
WEATHERIN	IG	M	4989.00 SqFt				
<b>Sample Number:</b> 02	2 <b>Ty</b> r	ne R	Area	6150.00 SqFt	<b>PCI:</b> 84		
SampleComents							
48 L & T CR		L	37.00 Ft				
48 L & T CR		M	20.00 Ft				
57 WEATHERIN	IG .	L	6150.00 SqFt				

Netw	<b>rk</b> 71J			Nane	Ozark Airport - 1	Blackwell Field	
Bran	h TA2		Nane	Taxiway A2 Ozark	Use	TAXIWAY A	10,910 SqFt
Sectic	<b>11</b> 01	of	1	From Runway 13-3	[	<b>To</b> Taxiway A	<b>Last Const:</b> 1/1/2005
Surfa	oe AC	Family:	ALDOT_AC	Taxiways <b>Zone</b>		Category:	Rark S
Area	1	0,910 SqFt	Length	183 Ft	Width	37 Ft	
Slabs		SlabLeng	<b>#</b> h	Ft <b>Slab</b>	Width	Ft	Joint Length Ft
Shod	der:	Street Ty	pe	Grad	<b>le</b> 0		Lanes 0
Sectic	nComents						
Wark	<b>Date:</b> 1/1/2005	Wo	rkType Nev	w Construction - Initial	(	Code NU-IN	IsMajorM&R: True
Last	<b>nsp Date</b> 11/4/2	2019	Total	Samples 2	Survey	ed 2	
Cand	tions PCI:	79		_	_		
Inspe	ctionComments						
Samp	leNumber: 01	Тург	a R	Area	5063.00 SqFt	<b>PCI:</b> 66	
Samp	leComents						
48	L & T CR		L	160.00 Ft			
48	L & T CR		M	33.00 Ft			
52	RAVELING		Н	6.00 SqFt			
57	WEATHERING		L	2520.00 SqFt			
	WEATHERING		M	2332.00 SqFt			
57	WEATHERING						
	leNumber: 02	Тург		Area	5847.00 SqFt	<b>PCI:</b> 90	
Samp		Тург		Area	5847.00 SqFt	<b>PCE</b> 90	
_	leNumber: 02	Тург		<b>Arex</b> 38.00 Ft	5847.00 SqFt	<b>PCE</b> : 90	

Netw	vorke 71.	Г					Nan	<b>12</b> Oza	rk Airport -	Blacky	well Fi	eld				
Bran	<b>xh</b> TB			N	ane	Taxiwa	ay B Oz	zark	Use	TA	XIWA	Υ	Area		41,232 SqFt	
Secti	<b>on</b> 01		of	1		From 1	Runway	y 13-31			То	Apron 03			LastConst	: 1/2/2016
Surf	ace AC		Family:	ALDC	T_AC	Taxiways	Zon	æ			Cate	gry:			Rank P	
Area	<b>t</b>	41,2	32 SqFt	1	Length	i .	1,100 F	<sup>7</sup> t	Width			35 Ft				
Slab	8		SlabLeng	<b>#</b> h		Ft		SlabWidth			Ft		J	intLength	]	Ft
Shou	lder:		Street Ty	pe				Grade 0					L			
Secti	anCanner	ts														
Worl	kDate 1/1/	2016	Wo	rkTy	e Bas	e Course - Ag	ggregat	e	(	Code	BA-A	AG		<b>IsMøja</b> rl	<b>MRR:</b> False	
Worl	kDate 1/2/	2016	Wo	rkTy	ne Nev	v Constructio	n - Initi	ial	(	Code	NU-I	N		IsMajor	VRR True	
Last	Insp Date	11/4/201	9		Total	Samples 8	8		Surve	yedt 5	5					
Cax	iltions P	<b>CI:</b> 92														
Insp	ectionCom	nents														
Sanj	pleNumber:	01	Тур	3	R	A	nea	5250	0.00 SqFt		]	PCI: 87				
Sanj	pleConner	ts														
48	L & T CR			L		107.00	Ft									
57	WEATHE			L		5250.00	SqFt									
Sanj	pleNumber:	03	Тур	3	R	A	Les	5250	0.00 SqFt		]	<b>PCE:</b> 94				
Sanj	pleConner	ts:														
57	WEATHE	RING		L		5250.00	SqFt									
Sanj	pleNumber:	05	Тур	3	R	A	nes	5250	0.00 SqFt		]	<b>PCI:</b> 94				
Sanj	pleConner	ts														
57	WEATHE	RING		L		5250.00	SqFt									
Sanj	pleNumber:	07	Тур	3	R	A	nea	5250	0.00 SqFt		]	<b>PCI:</b> 94				
Sanj	pleConner	<b>ts</b>														
57	WEATHE	RING		L		5250.00	SqFt									
Sanj	pleNumber:	08	Тур	9	A	A	nes	2284	4.00 SqFt		]	PCI: 83				
Sanj	pleConner	ts														
48	L & T CR			M		25.00	Ft									
57	WEATHE	RING		L		2284.00	SqFt									

Network 71J				Nane	Ozark Airport - I	Blackwell Field		
Branch TC		Nar	re Taxiv	vay C Ozark	Use	TAXIWAY	Area	7,359 SqFt
Section 01	o	of 1	From	Runway 13-31		To Apron 02		<b>Last Const:</b> 1/1/1956
Surface AC	Fanily:	ALDOT	_AC Taxiways	Zore		Category:		Rank S
Area	7,359 SqFt	Le	ngth	160 Ft	Width	35 Ft		
Slabs	SlabLe	ngth	Ft	SkabV	<b>Vicith</b>	Ft	JointLength	Ft
Shoulder:	Street T	ype		Grad	<b>3</b> 0		Lanes 0	
SectionComments								
Work Date: 1/1/19	56 <b>V</b>	Vork Type	New Construct	ion - Initial	C	Code NU-IN	IsM <b>aj</b> arN	True
Last Insp Date 1	1/4/2019	7	TotalSamples	2	Survey	ed 2		
Conditions PCI			•					
InspectionConne	nts							
		<b>pe</b> R	₹ .	Area	4940.00 SqFt	<b>PCI:</b> 66		
SampleNumber:	01 <b>Ty</b>	<b>pe</b> F	₹ .	Area	4940.00 SqFt	<b>PCI:</b> 66		
SanpleNumber: SampleComents	01 <b>Ty</b>	<b>pe</b> F	353.00		4940.00 SqFt	<b>PCI:</b> 66		
Sample Number: Sample Comments  48 L&TCR	01 <b>Ty</b>	•		) Ft	4940.00 SqFt	<b>PCI:</b> 66		
Sample Number: Sample Connents  48 L&TCR 48 L&TCR	01 <b>Ty</b>	L	353.00	) Ft ) Ft	4940.00 SqFt	<b>PCI:</b> 66		
Sample Number: Sample Comments  48 L&TCR  48 L&TCR  57 WEATHERII	01 <b>Ty</b>	L M	353.00 15.00	) Ft ) Ft ) SqFt	4940.00 SqFt	PCI: 66		
Sample Number: Sample Connerts:  48    L & T CR  48    L & T CR  57    WEATHERII  57    WEATHERII	01 <b>Ty</b> NG NG	L M L M	353.00 15.00 2900.00 2040.00	) Ft ) Ft ) SqFt	4940.00 SqFt 2420.00 SqFt	PCI: 66		
Sample Number: Sample Connents:  48    L & T CR  48    L & T CR  57    WEATHERD  57    WEATHERD  Sample Number:	01 <b>Ty</b> NG NG 02 <b>Ty</b>	L M L M	353.00 15.00 2900.00 2040.00	) Ft ) Ft ) SqFt ) SqFt				
Sample Number: Sample Connents:  48	01 <b>Ty</b> NG NG 02 <b>Ty</b>	L M L M	353.00 15.00 2900.00 2040.00	Ft Ft SqFt SqFt Area				
48 L&TCR 57 WEATHERII 57 WEATHERII SampleNumber: SampleComments	01 <b>Ty</b> NG NG 02 <b>Ty</b>	L M L M	353.00 15.00 2900.00 2040.00	Ft Ft SqFt SqFt Area				

Network	71J				Nane	Ozark	: Airport - E	Blackwell I	ield			
Branch	TC01		Nane	Taxiv	vay Connector 0	1 Ozark	Use	TAXIW	AY	Area	2,903 S	qFt
Section	01	0	f 1	From	Taxiway A			То	Edge of P	avement	LastC	<b>Const:</b> 1/1/195
Surface	AC	Family:	ALDOT_AC	C Taxiways	Zore			Cata	gny		Rark	S
Area		2,903 SqFt	Lengt	h	61 Ft	,	Width		30 Ft			
Slabs		SlabLe	ngth	Ft	Slab	Width		Ft		JointLe	ngth	Ft
Shoulder:		Street T	ype		Grad	<b>e</b> 0				Lanes	0	
SectionCo	ments											
WorkDate	e 1/1/1956	V	<b>VarkType</b> No	ew Construct	on - Initial		C	Corde NU	-IN	IsM	<b>kjarM&amp;R:</b> T	rue
LastInsp	Date 11/4	4/2019	Tota	alSamples	1		Surveye	edt 1				
Carditian	R PCI:	79										
Inspection	Comerts	8										
SampleN	umber: 01	Ty	pe R		Area	2903.0	00 SqFt		<b>PCI:</b> 79			
SampleCo	ments											
48 L&	T CR		L	13.00	Ft							
48 L&	T CR		M	50.00	Ft							
57 WE	ATHERING	7	L	2903.00	SaFt							

Network 71J			Nane	Ozark A	irport - E	Blackwell F	ield			
Branch TC02	]	<b>Name</b> Tax	iway Connector (	)2 Ozark	Use	TAXIW	AY	Area	6,215 SqFt	
Section 01	of 1	From	Taxiway A			To	Apron 01		LastConst	1/1/1997
Surface AC	Family: ALD	OT_AC Taxiways	Zone			Cate	gay:		Rank S	
Area	6,215 SqFt	Length	110 Ft	W	idth		35 Ft			
Slabs	SlabLength	]	Ft <b>Slab</b>	Width		Ft		Joint Len	<b>gth</b> 1	Ft
Shoulder:	<b>Street Type</b>		Grad	<b>le</b> 0				Lanes	0	
SectionConnents:										
<b>WorkDate</b> 1/1/1900	WorkTy	ype New Constru	ction - Initial		C	ode NU-	IN	IsMe	jorM&R: True	
<b>WorkDate</b> 1/1/1997	WorkTy	ype New Construc	ction - Initial		C	ode NU-	IN	IsMe	jarM&R: True	
Last Insp Date 11/4	/2019	TotalSamples	1		Surveye	<b>xt</b> 1				
Carditions PCI:	38									
InspectionConnents										
Sample Number: 01	Туре	R	Area	6215.00	SqFt		<b>PCI:</b> 38			
SampleComents										
43 BLOCK CR	M	1 125.0	00 SqFt							
48 L & T CR	L	74.0	00 Ft							
48 L & T CR	M	1 335.0	00 Ft							
50 PATCHING	N	1 90.0	00 SqFt							
57 WEATHERING	N	1 3062.0	00 SqFt							

Netv	work 71J			N	<b>Ozark</b>	Airport - Bla	ickwell Field			
Bra	nch TC03		Nane	Taxiway Co	onnector 03 Ozark	Use	TAXIWAY	Area	6,819 SqFt	
Secti	<b>ion</b> 01		of 1	From Taxi	way A		<b>To</b> Apron 01		Last Const:	10/19/1990
Surf	àce AC	Family:	ALDOT_A	C Taxiways <b>Z</b>	ane		Category:		Rank S	
Area	æ	6,819 SqFt	Lengt	<b>h</b> 100	) Ft	Width	40 Ft			
Slab	8	SlabL	ength	Ft	SlabWidth		Ft	Joint Length	Ft	
Sha	dder:	Street'	Туре		Grade 0			Lanes 0		
Secti	ianCaments									
War	<b>kDate</b> 1/1/1900	0 1	WorkType N	ew Construction - I	nitial	Ca	le NU-IN	Is <b>Maj</b> arN	True	
War	<b>kDate</b> 10/19/19	990	WorkType N	ew Construction - I	nitial	Co	e NU-IN	Is <b>Maj</b> arN	True	
Last	:Insp Date 11/	/4/2019	Tot	alSamples 1		Surveyed	1			
	ditions PCI:			-		·				
Insp										
	ectionConnent	<b>S</b>								
	ectionConnent pleNunber: 0:		ype R	Area	6819.	00 SqFt	<b>PCI:</b> 16			
Sam			<b>ype</b> R	Area	6819.	00 SqFt	<b>PCI:</b> 16			
San	<b>pleNumber:</b> 0	1 <b>T</b> ,	<b>ype</b> R	<b>Area</b> 130.00 SqF		00 SqFt	<b>PCI:</b> 16			
San	pleNumber: 0	1 <b>T</b> ,	<b>V1</b>		t	00 SqFt	<b>PCI:</b> 16			
Sam Sam 41	pleNumber: 03 pleComments  ALLIGATOR	1 <b>T</b> CR CKING	Н	130.00 SqF	t t	00 SqFt	<b>PCI:</b> 16			
<b>Sam</b> <b>Sam</b> 41 43 45	pleNumber: 03 pleConnerts  ALLIGATOR BLOCK CRAC	1 <b>T</b> CR CKING	H M	130.00 SqF 4747.00 SqF	t t	00 SqFt	<b>PCI:</b> 16			
San 9 41 43 45 48 50	pleNumber: 01 pleComments  ALLIGATOR BLOCK CRAC DEPRESSION L & T CR PATCHING	1 <b>T</b> CR CKING	H M M L L	130.00 SqF 4747.00 SqF 48.00 SqF 110.00 Ft 1248.00 SqF	t t t	00 SqFt	<b>PCI:</b> 16			
<b>San</b> y 41 43 45 48	pleNumber: 01 pleConnerts  ALLIGATOR BLOCK CRAC DEPRESSION L & T CR PATCHING PATCHING	1 <b>T</b> CR CKING	H M M L	130.00 SqF 4747.00 SqF 48.00 SqF 110.00 Ft	t t t	00 SqFt	<b>PCI:</b> 16			
41 43 45 48 50 50 52	pleNumber: 01 pleComments  ALLIGATOR BLOCK CRAC DEPRESSION L & T CR PATCHING PATCHING RAVELING	1 <b>T</b> CR CKING	H M M L L M M	130.00 SqF 4747.00 SqF 48.00 SqF 110.00 Ft 1248.00 SqF 694.00 SqF 2413.00 SqF	t t t t t	00 SqFt	<b>PCI:</b> 16			
Sam 41 43 45 48 50 50	pleNumber: 01 pleConnerts  ALLIGATOR BLOCK CRAC DEPRESSION L & T CR PATCHING PATCHING	1 <b>T</b> CR CKING	H M M L L L	130.00 SqF 4747.00 SqF 48.00 SqF 110.00 Ft 1248.00 SqF 694.00 SqF	t t t t t t	00 SqFt	<b>PCI:</b> 16			

Network	71J				N	<b>ene</b> Oza	ırk Airport - I	Blackwell Fi	eld			
Branch	THAN	IG01		Nane	Taxiway Ha	angar 01 Ozark	Use	TAXIWA	AY A	inea	18,282 SqFt	
Section	02		of 2	]	From Apro	n 04		To	T-Hangars		Last Const.:	1/1/1956
Surface	AC	Fan	ily: Al	LDOT_AC T	Γaxilanes <b>Z</b>	ane		Categ	gray.		Rank T	
Area		9,184 SqF	t	Length	206	5 Ft	Width		44 Ft			
Slabs		Sla	bLength	ı	Ft	SlabWidth		Ft		Joint Length	F	t
Shoulder:	:	Str	eet Type			Grade 0				Lanes 0		
SectionC	coments											
WorkDa	te 1/1/1950	6	Work	Type New	Construction - Ir	nitial	C	Coodle NU-I	N	<b>IsMaj</b> or	M&R: True	
Last Inso	Date 11	/4/2019		Totals	Samples 2		Survey	edt 2				
_	Date 11			Totals	Samples 2		Survey	ede 2				
Carditia	ns PCI:	26		TotalS	Samples 2		Survey	ed 2				
Canditia Inspectia	ns PCI: nConnen	26 <b>Les</b>	Tuna			459'			<b>PCT</b> 25			
Carditia Inspectia Sample N	ns PCI: nConnen Junber: 0	26 <b>Les</b>	Туре	<b>Totals</b>	Area	459:	Strvey		POE: 25			
Canditia Inspectia Sample N Sample C	ns PCI: nCannen inber: 0	26 <b>Les</b>	Туре	R		4592			PCI: 25			
Condition Inspection Sample N Sample C 43 BL	ne PCI: nCament lunber: 0 Caments LOCK CR	26 <b>ts</b>	Туре	R M	<b>Area</b> 4592.00 SqF	t			PCI: 25			
Condition Inspection Sample N Sample C 43 BL	ns PCI: nCannen inber: 0	26 <b>ts</b>	Туре	R	Area	t			POE: 25			
Condition Inspection Sample N Sample C 43 BL 57 W	ne PCI: nCament lunber: 0 Caments LOCK CR	26 <b>ts</b>	Туре	R M	<b>Area</b> 4592.00 SqF	t t		1	PCI: 25			
Condition Inspection Sample N Sample C 43 BL 57 WI Sample N	ne PCI: nConnent Amber: 0 Connents LOCK CR EATHERIN	26 <b>ts</b>		R M H	4592.00 SqFi 4592.00 SqFi	t t	2.00 SqFt	1				
Condition Inspection Sample N Sample C 43 BL 57 WI Sample N Sample C	ne PCI: nConnerts Lock CR EATHERIN	26 <b>ts</b>		R M H	4592.00 SqFi 4592.00 SqFi	t t 459	2.00 SqFt	1				
Condition Inspection Sample N Sample C 43 BI 57 WI Sample N Sample C 43 BI	ne PCI: nConnerts LOCK CR EATHERIN Linber: 0	26 <b>ts</b>		R M H	4592.00 SqFi 4592.00 SqFi 4592.00 SqFi	t t 459:	2.00 SqFt	1				

Nets	work 71J			Na	Ozark Airport -	· Blackwell Field		
Bra	n <b>ch</b> THANG	G01	Nane	Taxiway Han	ngar 01 Ozark <b>Use</b>	TAXIWAY	Area	18,282 SqFt
Sect	<b>ion</b> 01		of 2	From Apron	04	To T-Hang	gars	<b>Last Const.</b> 1/1/1956
Surf	Pace AC	Family:	ALDOT_AC	Taxilanes Zo	ne	Category:		Rank T
Are	æ	9,098 SqFt	Length	182	Ft <b>Width</b>	50 Ft		
Slab	B	SlabL	ength	Ft	SlabWidth	Ft	JaintL	<b>ength</b> Ft
Sho	ulder:	Street'	Гуре		Grade 0		Lanes	0
Sect	ionComments							
Wa	<b>kDate</b> 1/1/1956	; <b>T</b>	<b>WorkType</b> Ne	w Construction - Init	tial	Cade NU-IN	Isl	Major M&R: True
Last	Insp Date 11/	4/2019	Tota	Samples 2	Surve	seed 2		
Can	ditions PCI:	68						
Insp	ectionCorment	8						
San	<b>pleNumber:</b> 01	T	ype R	Area	5000.00 SqFt	PCI:	62	
	pleConnents	•	<b>,                                    </b>		•			
48	L & T CR		L	22.00 Ft				
48	L & T CR		M	151.00 Ft				
52	RAVELING		L	48.00 SqFt				
52	RAVELING		Н	4.00 SqFt				
57	WEATHERING	G	L	2474.00 SqFt				
57	WEATHERING	G	M	2474.00 SqFt				
San	<b>pleNumber:</b> 02	· <b>T</b>	ype R	Area	4098.00 SqFt	PCI:	75	
San	pleConnents							
	L & T CR		L	20.00 Ft				
48	Læick							
	L&TCR L&TCR		M	50.00 Ft				
48		G	M L	50.00 Ft 2049.00 SqFt				

Network	71J				Nane	Ozark Airport - l	Blackwell Field		
Branch	THANG02		Nane	Taxiway	Hangar 02 O	zark <b>Use</b>	TAXIWAY	Area	31,073 SqFt
Section	01	oi	f 1 <b>F</b>	rom Ta	xiway A		<b>To</b> T-Ha	angars	<b>Last Const:</b> 6/1/2018
Surface	AC	Family:	ALDOT_AC Ta	ixilanes	Zone		Category:		Rank T
Area	31,07	73 SqFt	Length		556 Ft	Width	49 F	`t	
Slabs		SlabLer	ggth	Ft	SlabV	<b>Nicht</b>	Ft	Joint Le	<b>ngh</b> Ft
Shoulder:		Street T	ype		Grade	<b>9</b> 0		Lares	0
SectionCo	nneris								
WorkDate	e 6/1/2018	V	tarkType New (	Construction	- Initial	(	Code NU-IN	IsM	<b>GOTMER</b> True
 Last Insp l	<b>Date</b> 11/4/2019	9	TotalSa	mples 5		Survey	edi 1		
Carditians	<b>PCE</b> 100								
Inspection	Coments								
SampleNu	mber: 01	Ту	pee R	Ar	eat	5000.00 SqFt	PCI:	100	

**Sample Comments** 

<sup>&</sup>lt;No Distress>



## Appendix E

## **Distress Summary Report**

Ozark Airport - Blackwell Field (71J)

Branch ID	Section ID	Surface <sup>1</sup>	Area (sf)	Distress Number	Description	Distress Mechanism	Severity	Quantity	Quantity Units	Distress Density
A01	01	AC	78,123	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	2,371	Ft	3.0%
A01	01	AC	78,123	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	3,209	Ft	4.1%
A01	01	AC	78,123	57	WEATHERING	Climate/Durability	Low	70,895	SqFt	90.7%
A01	01	AC	78,123	57	WEATHERING	Climate/Durability	Medium	7,228	SqFt	9.3%
A01	02	AC	53,187	43	BLOCK CRACKING	Climate/Durability	Medium	38,425	SqFt	72.2%
A01	02	AC	53,187	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	1,364	Ft	2.6%
A01	02	AC	53,187	52	RAVELING	Climate/Durability	Medium	20,667	SqFt	38.9%
A01	02	AC	53,187	57	WEATHERING	Climate/Durability	High	32,540	SqFt	61.2%
A02	01	AC	48,373	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	2,344	Ft	4.8%
A02	01	AC	48,373	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	2,903	Ft	6.0%
A02	01	AC	48,373	57	WEATHERING	Climate/Durability	Low	48,373	SqFt	100.0%
A03	01	AC	80,000	57	WEATHERING	Climate/Durability	Low	80,000	SqFt	100.0%
A04	01	AAC	122,760	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	4,473	Ft	3.6%
A04	01	AAC	122,760	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	4,488	Ft	3.7%
A04	01	AAC	122,760	57	WEATHERING	Climate/Durability	Low	122,760	SqFt	100.0%
R1331	01	AC	412,160	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	26,229	Ft	6.4%
R1331	01	AC	412,160	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	440	Ft	0.1%
R1331	01	AC	412,160	50	PATCHING	Climate/Durability	Low	689	SqFt	0.2%
R1331	01	AC	412,160	57	WEATHERING	Climate/Durability	Medium	411,471	SqFt	99.8%
TA	01	AC	188,480	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	6,031	Ft	3.2%
ТА	01	AC	188,480	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	1,103	Ft	0.6%

## Appendix E

## **Distress Summary Report**

Ozark Airport - Blackwell Field (71J)

Branch ID	Section ID	Surface <sup>1</sup>	Area (sf)	Distress Number	Description	Distress Mechanism	Severity	Quantity	Quantity Units	Distress Density
TA	01	AC	188,480	57	WEATHERING	Climate/Durability	Low	152,117	SqFt	80.7%
TA	01	AC	188,480	57	WEATHERING	Climate/Durability	Medium	36,363	SqFt	19.3%
TA1	01	AC	11,139	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	217	Ft	1.9%
TA1	01	AC	11,139	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	51	Ft	0.5%
TA1	01	AC	11,139	57	WEATHERING	Climate/Durability	Low	6,150	SqFt	55.2%
TA1	01	AC	11,139	57	WEATHERING	Climate/Durability	Medium	4,989	SqFt	44.8%
TA2	01	AC	10,910	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	198	Ft	1.8%
TA2	01	AC	10,910	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	33	Ft	0.3%
TA2	01	AC	10,910	52	RAVELING	Climate/Durability	High	6	SqFt	0.1%
TA2	01	AC	10,910	57	WEATHERING	Climate/Durability	Low	8,367	SqFt	76.7%
TA2	01	AC	10,910	57	WEATHERING	Climate/Durability	Medium	2,332	SqFt	21.4%
ТВ	01	AC	41,232	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	198	Ft	0.5%
ТВ	01	AC	41,232	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	25	Ft	0.1%
ТВ	01	AC	41,232	57	WEATHERING	Climate/Durability	Low	41,232	SqFt	100.0%
тс	01	AC	7,359	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	426		5.7%
тс	01	AC	7,359	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	23	Ft	0.3%
TC	01	AC	7,359	57	WEATHERING	Climate/Durability	Low	5,384	SqFt	72.3%
TC	01	AC	7,359	57	WEATHERING	Climate/Durability	Medium	2,064	SqFt	27.7%
TC01	01	AC	2,903	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	13	Ft	0.4%
TC01	01	AC	2,903	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	50	Ft	1.7%
TC01	01	AC	2,903	57	WEATHERING	Climate/Durability	Low	2,903	SqFt	100.0%
TC02	01	AC	6,215	43	BLOCK CRACKING	Climate/Durability	Medium	125	SqFt	2.0%

## Appendix E

#### **Distress Summary Report**

Ozark Airport - Blackwell Field (71J)

Branch ID	Section ID	Surface <sup>1</sup>	Area (sf)	Distress Number	Description	Distress Mechanism	Severity	Quantity	Quantity Units	Distress Density
TC02	01	AC	6,215	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	74	Ft	1.2%
TC02	01	AC	6,215	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	335	Ft	5.4%
TC02	01	AC	6,215	50	PATCHING	Climate/Durability	Medium	90	SqFt	1.4%
TC02	01	AC	6,215	57	WEATHERING	Climate/Durability	High	3,062	SqFt	49.3%
TC02	01	AC	6,215	57	WEATHERING	Climate/Durability	Medium	3,062	SqFt	49.3%
TC03	01	AC	6,819	41	ALLIGATOR CRACKING	Load	High	130	SqFt	1.9%
TC03	01	AC	6,819	43	BLOCK CRACKING	Climate/Durability	Medium	4,747	SqFt	69.6%
TC03	01	AC	6,819	45	DEPRESSION	Other	Medium	48	SqFt	0.7%
TC03	01	AC	6,819	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	110	Ft	1.6%
TC03	01	AC	6,819	50	PATCHING	Climate/Durability	Low	1,248	SqFt	18.3%
TC03	01	AC	6,819	50	PATCHING	Climate/Durability	Medium	694	SqFt	10.2%
TC03	01	AC	6,819	52	RAVELING	Climate/Durability	High	50	SqFt	0.7%
TC03	01	AC	6,819	52	RAVELING	Climate/Durability	Medium	2,413	SqFt	35.4%
TC03	01	AC	6,819	57	WEATHERING	Climate/Durability	High	2,413	SqFt	35.4%
THANG01	01	AC	9,098	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Low	42	Ft	0.5%
THANG01	01	AC	9,098	48	LONGITUDINAL/TRANSVERSE CRACKING	Climate/Durability	Medium	201	Ft	2.2%
THANG01	01	AC	9,098	52	RAVELING	Climate/Durability	High	4	SqFt	0.0%
THANG01	01	AC	9,098	52	RAVELING	Climate/Durability	Low	48	SqFt	0.5%
THANG01	01	AC	9,098	57	WEATHERING	Climate/Durability	Low	4,523	SqFt	49.7%
THANG01	01	AC	9,098	57	WEATHERING	Climate/Durability	Medium	4,523	SqFt	49.7%
THANG01	02	AC	9,184	43	BLOCK CRACKING	Climate/Durability	Medium	8,234	SqFt	89.7%
THANG01	02	AC	9,184	50	PATCHING	Climate/Durability	Low	950	SqFt	10.3%
THANG01	02	AC	9,184	57	WEATHERING	Climate/Durability	High	8,234	SqFt	89.7%
THANG02	01	AC	31,073					0		0.0%

<sup>&</sup>lt;sup>1</sup> AC = Asphalt Cement Concrete, AAC = Aphalt Overlay AC, PCC = Portland Cement Concrete, APC = Asphalt Overlay PCC

<sup>&</sup>lt;sup>2</sup> LCD = Last construction date. The date of the last major pavement rehabilitation (e.g. AC overlay)

## **APPENDIX F**

## **INVENTORY**

F1: Section Forecasted Pavement Condition Rating

F2: Branch PCI Rating F3: Branch FOD Rating

# Appendix F1 Forecasted Section PCI

Ozark Airpot - Blackwell Field (71J)

Branch ID	Section ID	Forecasted PCI								
DIGITOR	Section in	2021	2022	2023	2024	2025	2026	2027		
A01	01	63	61	59	57	55	52	50		
A01	02	19	17	15	13	11	8	6		
A02	01	59	57	55	53	51	48	46		
A03	01	91	89	87	85	83	80	78		
A04	01	65	63	61	59	57	54	52		
R1331	01	71	70	70	69	66	57	52		
TA	01	76	74	71	68	64	60	55		
TA1	01	76	74	71	68	64	60	55		
TA2	01	77	75	72	70	66	62	57		
ТВ	01	89	86	84	81	79	77	75		
TC	01	67	63	59	54	49	46	44		
TC01	01	77	75	72	70	66	62	57		
TC02	01	34	30	27	23	20	16	13		
TC03	01	12	8	5	1	0	0	0		
THANG01	01	64	59	54	49	46	44	40		
THANG01	02	22	18	15	11	8	4	1		
THANG02	01	99	98	96	94	92	89	86		

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## **Branch Condition Report**

Page 1 of 2

Pavement Database: ALDOT\_Combined\_201201

Branch ID	Number of Sections	Sum Section Length (Ft)	Avg Section Width (Ft)	True Area (SqFt)	Use	Average PCI	Standard Deviation PCI	Weighted Average PCI
A01	2	1,297.00	195.50	131,310.00	APRON	44.00	22.00	48.18
A02	1	324.00	158.00	48,373.00	APRON	62.00	0.00	62.00
A03	1	400.00	200.00	80,000.00	APRON	94.00	0.00	94.00
A04	1	660.00	186.00	122,760.00	APRON	68.00	0.00	68.00
R1331	1	5,152.00	80.00	412,160.00	RUNWAY	73.00	0.00	73.00
TA	1	5,421.00	35.00	188,480.00	TAXIWAY	78.00	0.00	78.00
TA1	1	184.00	42.00	11,139.00	TAXIWAY	78.00	0.00	78.00
TA2	1	183.00	37.00	10,910.00	TAXIWAY	79.00	0.00	79.00
TB	1	1,100.00	35.00	41,232.00	TAXIWAY	92.00	0.00	92.00
TC	1	160.00	35.00	7,359.00	TAXIWAY	71.00	0.00	71.00
TC01	1	61.00	30.00	2,903.00	TAXIWAY	79.00	0.00	79.00
TC02	1	110.00	35.00	6,215.00	TAXIWAY	38.00	0.00	38.00
TC03	1	100.00	40.00	6,819.00	TAXIWAY	16.00	0.00	16.00
THANG01	2	388.00	47.00	18,282.00	TAXIWAY	47.00	21.00	46.90
THANG02	1	556.00	49.00	31,073.00	TAXIWAY	100.00	0.00	100.00

Pavement Management System PAVER 7.0 TM

2/1/2021 Branch Condition Report	
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Pavement Database: ALDOT\_Combined\_201201

Use Category	Number of Sections	Total Area (SqFt)	Arithmetic Average PCI	Average STD PCI	Weighted Average PCI
APRON	5	382,443.00	62.40	23.13	65.87
RUNWAY	1	412,160.00	73.00	0.00	73.00
TAXIWAY	11	324,412.00	65.91	25.89	77.95
ALL	17	1,119,015.00	65.29	24.44	72.00

Page 2 of 2

Pavement Management System PAVER 7.0 TM

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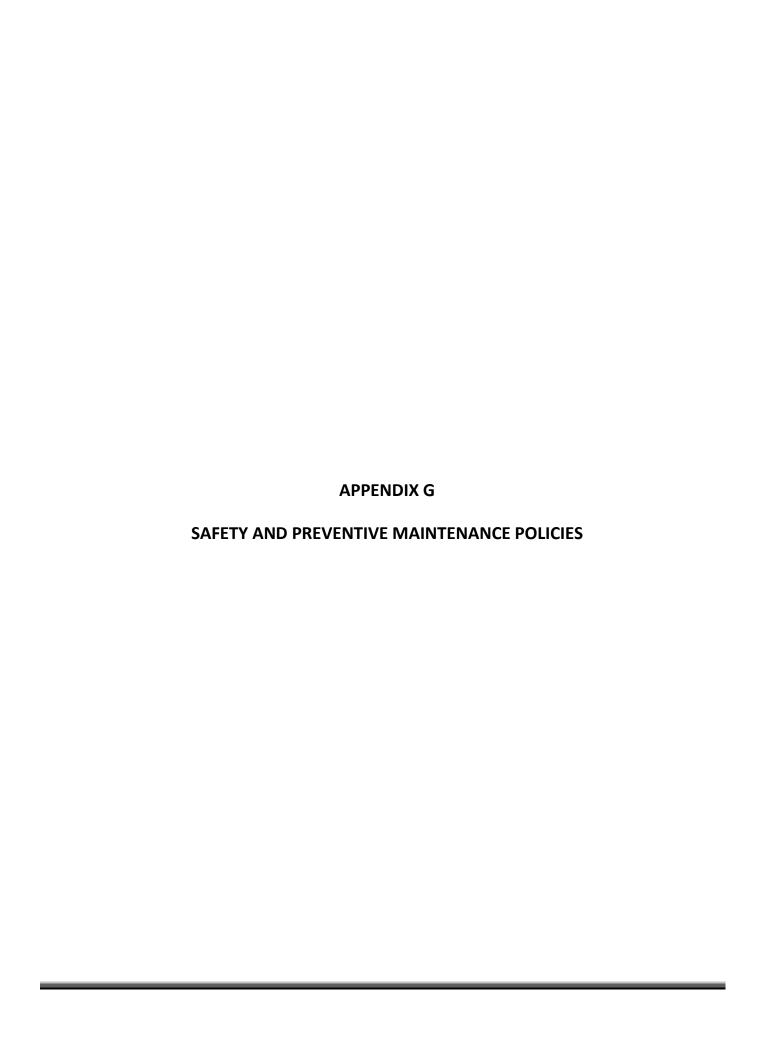
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# Appendix G1 Localized Safety (Stopgap) Repair Policy

Distress	Distress Severity	Description	Code	Work Type	Work Unit
41	High	ALLIGATOR CR	PA-FD	Patching - AC Full-Depth	SqFt
43	High	BLOCK CR	CS-AC	Crack Sealing - AC	Ft
45	High	DEPRESSION	PA-FD	Patching - AC Full-Depth	SqFt
47	High	JT REF. CR	CS-AC	Crack Sealing - AC	Ft
48	High	L & T CR	CS-AC	Crack Sealing - AC	Ft
50	High	PATCHING	PA-FD	Patching - AC Full-Depth	SqFt
53	High	RUTTING	PA-FD	Patching - AC Full-Depth	SqFt
54	High	SHOVING	PA-PD	Patching - AC Partial-Depth	SqFt
55	NA	SLIPPAGE CR	PA-PD	Patching - AC Partial-Depth	SqFt
56	High	SWELLING	PA-FD	Patching - AC Full-Depth	SqFt
61	High	BLOW-UP	SL-PC	Slab Replacement - PCC	SqFt
61	Medium	BLOW-UP	PA-PF	Patching - PCC Full Depth	SqFt
62	High	CORNER BREAK	PA-PF	Patching - PCC Full Depth	SqFt
63	High	LINEAR CR	PA-PF	Patching - PCC Full Depth	SqFt
63	Medium	LINEAR CR	CS-PC	Crack Sealing - PCC	Ft
64	High	DURABIL. CR	SL-PC	Slab Replacement - PCC	SqFt
64	Medium	DURABIL. CR	PA-PF	Patching - PCC Full Depth	SqFt
66	High	SMALL PATCH	PA-PP	Patching - PCC Partial Depth	SqFt
67	High	LARGE PATCH	PA-PF	Patching - PCC Full Depth	SqFt
70	High	SCALING	SL-PC	Slab Replacement - PCC	SqFt
71	High	FAULTING	GR-PP	Grinding (Localized)	Ft
72	High	SHAT. SLAB	SL-PC	Slab Replacement - PCC	SqFt
74	High	JOINT SPALL	PA-PP	Patching - PCC Partial Depth	SqFt
75	High	CORNER SPALL	PA-PP	Patching - PCC Partial Depth	SqFt
76	High	ASR	SL-PC	Slab Replacement - PCC	SqFt

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## **APPENDIX H**

# **M&R UNIT COSTS**

H1: M&R Unit Costs

H2: Component Costs for Repair

H3: Airport Category

#### Maintenance and Repair (M&R) Unit Costs

The M&R costs developed for the ALDOT PMP include costs for maintenance, preservation, and repair activities and are described below.

#### Unit Costs Source Data

The source for the M&R costs data is RSMeans, which has data for 14 locations throughout Alabama, as identified by the yellow highlighted boxes in Figure 1. The cost data is presented in terms of individual line items like asphalt wearing course, aggregate base etc., which were consolidated to develop the activity costs described below.

The cost data show a distinct difference in costs between locations north and south of Birmingham, especially for the higher value items like the asphalt layers. Therefore, the unit costs were developed accordingly for the airports north and south of Birmingham, as identified in Figure 1. Appendix H2 presents the component costs used in developing the M&R costs.

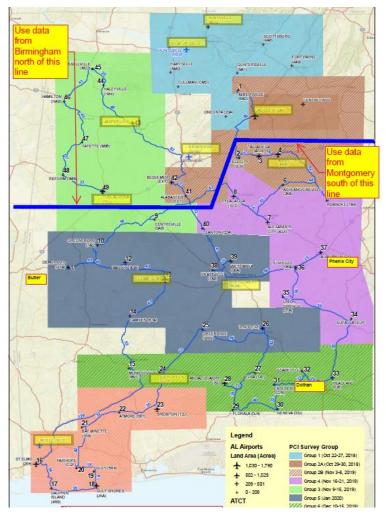


Figure 1: RSMeans Unit Costs Locations.

#### Maintenance & Repair (M&R) Activities

Maintenance activities are localized activities which are typically assigned in the first year of the M&R plan based on the observed distresses.

Repair activities are further subdivided into preservation, rehabilitation, and reconstruction. Repair activities are conducted for larger areas, typically at the section level and are assigned based on the # h # @ #h u #h in # # importance within the overall network and typically ranges from 55 to 70. The CP was set at 70 for the ALDOT runway pavements and 65 for the other pavements.

Activity Type	PCI	Activity
Preservation	> CP	Runway Surface Treatment
Preservation	> CP	Taxiway and Apron Surface Treatment
	> CP	2" AC OL <sup>1</sup>
Rehabilitation	55 - CP	Mill 2" & 2" AC OL
	45 - 55	Mill 2" & 3" AC OL
Reconstruction	0 - 45	Reconstruct with AC

Table 1: Repair Activities.

The depths for the milling and overlay (AC OL) in Table 1 were established by creating a balance between removal of surficial distress and providing additional pavement structural capacity. All overlay options include full-depth patching to repair localized distresses.

From the FAA 5010 records, the Alabama airport network includes a wide range of allowable aircraft loads. The airports were divided into three categories of allowable aircraft loads based on requirements for minimum pavement thickness and the use of a P-401 surface layer. The categories are based on the aircraft maximum gross takeoff weight (MGTOW) and include: less than 12,500 lbs, 12,500 to 30,000 lbs, and 30,000 to 100,000 lbs. Appendix H3 presents the category for each airport.

For any sections requiring reconstruction, the pavement sections were established primarily in accordance with the requirements in a section with the requirements in the section with the section with the requirements in the section with the requirements in the section with the section

2,500 lbs
 12,500 30,000 lbs
 30,000 10s
 4 h-403 (State HMA Mix) + 6 P-209 Base
 h-403 (State HMA Mix) + 8 h-209 Base
 h-401 + 10 h-209 Base

It is important to note that while the FAA requires a stabilized base for those pavements that support aircraft operations with MGTOWs that are greater than 100,000 lbs, the number of such operations is minimal for those airports shown in Appendix H3. As a result, the cost of a stabilized base is excluded in  $^{\circ}$  O \ u  $^{\circ}$  hU hU h  $^{\circ}$  =

design and aircraft fleet mix development, project-level construction work could include the use of a stabilized base at that time.

<sup>&</sup>lt;sup>1</sup>For Sections with Structural Distress and PCI greater than Critical PCI

#### M&R Unit Costs

Paving projects typically include additional project costs like mobilization, design, construction administration and inspections, and drainage improvements. A summary of non-direct pavement construction line items has been included in the unit costs in Tables 5 and 6 as described below. These non-direct items are expressed as a percentage of the total component costs for each activity.

These non-

APMP project cost estimation. These percentages may vary for Alabama airport construction projects; however, since the direct pavement scope of work is estimated in a network-level evaluation, these conservative estimates serve as a good starting point for the development of realistic total project costs and annual APMP budgets for ALDOT. For repair activities such as Mill & Overlay, which typically do not include significant drainage work, the corresponding multiplier was reduced by 50 percent. The non-direct cost factors are presented in Table 2.

Factor Function of		Estimate			
racioi	Function of	Preservation	Rehabilitation	Reconstruction	
Mobilization	All costs, less design	10%	10%	10%	
Drainage Improvements	Paving costs	-	4%	8%	
Contingency	All costs, less mobilization and design	10%	20%	20%	
Design & CM	All costs, less mobilization and design	15%	20%	20%	

Table 2: Cost Factors.

The M&R unit costs for maintenance, preservation, and repair activities were developed from the RSMeans cost data and are presented in the following section.

#### Maintenance

The maintenance activities include crack seal, and full and partial-depth patching. The unit costs are presented in Table 3.

Activity	Unit Cost	Unit
Seal Cracks - AC	\$3.95	lf
AC Full-Depth Patching	\$25.05	sf
AC Partial-Dept Patching	\$16.28	sf
Seal Cracks PCC	\$6.00	lf
PCC Full-Depth Patching	\$35.00	sf
PCC Partial-Depth Patching	\$175.00	sf
Jt. Seal	\$8.00	lf
Slab Replacement	\$20.00	sf

Table 3: Unit Costs for Maintenance.

#### Preservation

The unit costs for the surface treatments are presented in Table 4. They include sealing of cracks and application of pavement markings.

Table 4: Unit Costs for Preservation Activities.

Activity	Unit Cost	Unit
Runway Surface Treatment	\$0.57	sf
Taxiway and Apron Surface Treatment	\$0.88	sf

#### Rehabilitation and Reconstruction

As discussed previously, repair activities are also divided into rehabilitation and reconstruction. The unit costs for airport repair for the Northern Region (Birmingham Area) and Southern Region (Montgomery Area) are shown in Tables 5 and 6, respectively.

Table 5: Unit Costs for Repair Activities, Northern Region.

Activity Type	Activity	MGTOW, thousand lbs		
Activity Type	Activity	· 2.5	12.5-30	30-100
	2" AC OL	\$3.	78	\$4.19
Rehabilitation	Mill 2" & 2" AC OL	\$4.	15	\$4.56
	Mill 2" & 3" AC OL	\$5.18		\$5.79
Reconstruction	AC Reconstruction	\$8.40	\$9.10	\$10.91

Table 6: Unit Costs for Repair Activities, Southern Region.

Activity Type	Activity	MGTOW, thousand lbs					
Activity Type	Activity	· 2.5	12.5-30	30-100			
	2" AC OL	\$3.	\$3.91				
Rehabilitation	Mill 2" & 2" AC OL	\$3.	\$4.27				
	Mill 2" & 3" AC OL	\$4.	\$5.37				
Reconstruction	Reconstruction AC Reconstruction		\$8.25	\$9.87			

# Appendix H2 Component Costs for Repair

Activity Type	Unit	Birmingham (Northern)	Montgomery (Southern)	Comments
Milling 1" to 3"	SY	\$2.08	\$2.01	
Pavement Demolition	SY	\$6.34	\$6.12	
Haulage - For Demolition & AC	CY	\$6.08	\$5.87	
Haulage for 12" Thick Demolition	SY	\$2.03	\$1.96	
Haulage for 2" Thick AC Paving	SY	\$0.34	\$0.33	
Haulage for 3" Thick AC Paving	SY	\$0.51	\$0.49	
Haulage for 4" Thick AC Paving	SY	\$0.68	\$0.65	
AC Wearing Course	Ton	\$97.42	\$86.90	
AC Binder Course	Ton	\$87.80	\$78.17	
P401 - For airports with >60 kip aircraft	Ton	\$116.90	\$104.28	Assumed P401 cost to be 20% greater than AC Wearing Course
6" Aggregate Base (P208)	SY	\$10.17	\$9.12	
8" Aggregate Base (P208)	SY	\$13.29	\$11.89	
6" P209 Aggregate Base	SY	\$12.20	\$10.94	Assumed P209 cost to be 20% greater than P208
8" P209 Aggregate Base	SY	\$15.95	\$14.27	Assumed P209 cost to be 20% greater than P208
10" P209 Aggregate Base	SY	\$19.94	\$17.84	Direct multiplier for 10" from 8"
4" P154 Aggregate Base	SY	\$5.42	\$4.86	Assumed P154 cost to be 20% lower than P208
6" P154 Aggregate Base	SY	\$8.14	\$7.30	Assumed P154 cost to be 20% lower than P208
Pavement Markings	sf	\$1.48	\$1.39	

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## **APPENDIX I**

## **PAVEMENT CAPITAL IMPROVEMENT PROGRAM**

I1: PCIP Summary

I2: Year 1 Maintenance Plan

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# Appendix I2 Localized Maintenance Plan

## OzakAirport-Badwell Field (71.)

D-J-ID	Section	D.F.	Distress	Desiries	G4_	Distress	Distress	Percent	W.LD.	Wark	Wark	Urit	
BandlD		Rollicy	Code	Description	Severity	Qb	Unit	Distress	WarkDescription	Qb	Urit	Cost	WorkCost
AOE	<b>O1</b>	Safety	4	L&TG	Mediun	<b>290</b> £	R	$\epsilon$	No Localized M&F	C		<b>\$00</b> 0	<b>\$</b> C
AOE	O1	Safety	4	L&TG	Low	234	R	48	No Localized M&F	•		<b>\$000</b>	<b>\$</b> C
AOE	O1	Safety	57	WEATHERNG	Iou	4837	SqR	100	No Localized M&F	C		<b>\$00</b> 0	80
AOE	O1	Revertive	57	WEATHERNG	Low	80,000	SqR	100	No Localized M&F	C		<b>\$00</b> 0	<b>\$</b> C
AO4	O1	Resentise	4	L&TG	Mediun	449	R	366	GadsSealing- AC	4/19	R	<b>839</b> E	\$17,72
AO4	O1	Resentise	57	WEATHERNG	Low	122,780	SqFl	100	No Localized M&F	•		<b>\$000</b>	<b>\$</b> C
<b>AO</b> 4	O1	Resentive	4	L&TG	Iov	447	R	364	No Localized M&F	C		<b>\$000</b>	<b>\$</b> (
R1331	O1	Revertive	<b>5</b> X	PAICHNG	Iou	680	SqFI	017	No Localized M&F	C		<b>\$00</b> 0	<b>\$</b> C
R1331	O1	Revertive	4	L&TG	Mediun	410	R	011	GadeSealing- AC	440	R	<b>839</b> E	<b>\$1,73</b>
R1331	Oı	Revertive	57	WEATHERN	Mediun	411,471	SqFI	998	No Localized M&F	C		<b>\$00</b> 0	<b>\$</b> C
R1331	Oı	Revertive	4	L&TG	Iov	26,22	R	636	No Localized M&F	C		<b>\$00</b> 0	\$0
TA	O1	Revertive	57	WEXIHERYC	Mediun	3636	SqF	192	No Localized M&F	C		<b>\$00</b> 0	80
TA	O1	Revertive	4	L&TG	Mediun	1,100	R	050	GadeSealing- AC	1,102	R	<b>839</b> E	<b>84,35</b> £
TA	O1	Revertive	57	WEXIHERYC	Iov	152,117	SqF	8071	No Localized M&F	C		<b>\$00</b> 0	80
TA	01	Revertive	4	L&TG	Low	6081	R	32	No Localized M&F	•		<b>\$000</b>	80
TA1	O1	Revertive	4	L&TG	Mediun	51			GadeSealing- AC	51	R	<b>839</b> E	\$201
TA1	01	Revertive	57	WEXIHENC	Mediun	4,980	_	447	No Localized M&F	•		<b>\$000</b>	<b>\$</b> C
TA1	O1	Revertive	4	L&TG	Low	217	R	<b>1.9</b> 5	No Localized M&F	•		<b>\$000</b>	<b>\$</b> C
TA1	O1	Revertive	57	WEXIHERYC	Low	6150	ScpFl		No Localized M&F	C		<b>\$000</b>	80
TAL	OI	Revertive	57	WEXIHERNO	Low	836	_		No Localized M&F	C		<b>\$000</b>	80
TAE	O1	Revertive	52	RAVHING	Hg		SqFI		Patching- ACPartial Dep	6	SqR	\$162	<b>\$9</b> £
TA2	O1	Resentive	4€	L&TG	Low	195			NoIocalizedM&F	C		<b>\$000</b>	<b>\$</b> C
TAE	O1	Resentive	57	WEXIHERNO	Mediun	2,332	_	21.3	NoIocalizedM&F	C		<b>\$000</b>	<b>\$</b> C
TAE	O1	Revertive	4€	L&TG	Mediun	33			GadsSealing- AC	33	R	<b>839</b>	<b>\$13</b> 0
Ŧ	OI	Revertive	4	L&TG	Iov	198			No Localized M&F	C		<b>\$000</b>	80
Ŧ	OI	Revertive	57	WEATHERNO	Iov	41,23	_		No Localized M&F	C		<b>\$000</b>	80
TE .	O1	Resentive	4	L&TG	Median	25			GackSealing- AC		R	<b>839</b>	<b>89</b> 0
T	O1	Resentive	4	L&TG	Mediun	22	_		GackSealing- AC	23	R	<b>839</b>	<b>\$9</b> £
T	O	Revertive	57	WEATHERNG	Medin	2,064	SqR	27.72	NolocalizedM&F	•		<b>\$00</b> 0	80

# Appendix I2 Localized Maintenance Plan

# OzakAirport-Badwell Field (71.)

BarchID	Section II	Ricy	Distress Code	Description	Severity	Q <sub>3</sub>	Uri	Percent Distress	WakDesaiption	Work Work Q5 U		WarkCost
K	<b>O1</b>	Revertive	57	WEXIHERYC	Iow	5384	SqFI	722	No Localized M&F	C	<b>\$00</b> 0	\$0
K	O1	Reventive	4	L&TG	Iov	426	R	572	No Localized M&F	C	<b>\$00</b> 0	<b>\$</b> C
TCOI	O1	Reventive	57	WEXIHERING	Iov	2,900	SqFl	100	No Localized M&F	C	<b>\$00</b> 0	<b>\$</b> C
TCOI	O1	Revertive	4	L&TG	Iow	13	R	045	No Localized M&F	C	<b>\$00</b> 0	80
TCOI	O1	Reventive	4	L&TG	Mediun	50	R	1.72	GadeSealing- AC	50 R	<b>\$39</b> E	\$196
THANGO	O1	Reventive	57	WEXIHERING	Iov	4,523	SqFl	4971	No Localized M&F	C	<b>\$00</b> 0	80
THANGO	O1	Reventive	4	L&TCF	Mediun	201	R	221	GadkSealing- AC	201 R	\$39£	\$794
THANGO	O1	Revertive	4	L&TG	Iow	42	R	046	No Localized M&F	C	<b>\$00</b> 0	80
THANGO	O1	Reventive	52	RAVHING	Iov	4	SqFl	053	No Localized M&F	C	<b>\$00</b> 0	<b>\$</b> C
THANGO	O1	Reventive	57	WEXIHERING	Mediun	4,523	SqFl	4971	No Localized M&F	C	<b>\$00</b> 0	80
THANGO	O1	Reventive	52	RAVHING	Hg	4	SqFl	004	Patching- ACPartial Dep	4 <b>Sq</b>	<b>\$1625</b>	<b>\$6</b> 5
THANGO	Œ	Safety	<b>5</b> 0	PAICHNG	Iov	950	SqFI	1034	No Localized M&F	C	8000	80
THANGO	Œ	Safety	57	WEATHERN	Hg	8234	SqFl	8963	No Localized M&F	C	<b>\$00</b> 0	80
THANGO	Œ	Safety	46	HOKG	Mediun	8234	SqF	8968	NolocalizedM&F	C	<b>\$00</b> 0	80