REMOVING BRIDGE DECKS: THE EXISTING GIRDERS SHALL NOT BE DAMAGED IN ANY WAY DUE TO THE REMOVAL OF THE BRIDGE DECK. THE CONTRACTOR SHALL NOT USE ANY REMOVAL EQUIPMENT LARGER THAN A 70 POUND HAND HELD PAVEMENT BREAKER. THE HAMMER SIZE SHALL BE LIMITED TO 35 POUNDS WHEN WORKING WITHIN 6" OF THE BREAKLINE OR EDGE OF GIRDER. CARE SHALL BE TAKEN NOT TO DAMAGE THE CONCRETE AND STEEL REINFORCEMENT TO BE RETAINED. WHERE THE DECK IS BEING REMOVED BETWEEN GIRDERS FOR ARMOR JOINT REPLACEMENT, THE CONSTRUCTION JOINT SHALL BE TAPERED TOWARD THE BREAKOUT AREA TO PREVENT SPALLS ON THE UNDERSIDE OF THE SLAB. THE CONTRACTOR SHALL REPAIR CONCRETE SPALLS AND DAMAGED STEEL REINFORCEMENT, AT NO ADDITIONAL COST TO THE PROJECT, BY A METHOD APPROVED BY THE ENGINEER.

SHOP DRAWINGS: STRUCTURAL STEEL DETAILS SHOWN ON THE PLANS ARE FOR ESTIMATING PURPOSES ONLY, AND ARE NOT GUARANTEED BY THE STATE TO BE ENTIRELY COMPLETE AND CORRECT. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CORRECT SHOP DRAWINGS AND DETAIL DIMENSIONS TO FIT THE STRUCTURE. ATTENTION IS CALLED TO SECTION 105, PARTICULARLY ARTICLE 105.02, AND ARTICLE 836.14 OF THE SPECIFICATIONS.

COLUMN REINFORCING BARS: WHEN THE CAP IS NOT MORE THAN TWO (2) INCHES WIDER ON EACH SIDE OF THE COLUMN, THE FOLLOWING SHALL APPLY: COLUMN BARS SHALL BE TIED TO THE TOP FEW COLUMN HOOPS IN A MANNER THAT WILL ALLOW COLUMN BARS TO CLEAR CAP REINFORCING AND BE LOCATED INSIDE OF CAP REINFORCING.

MANDATORY NOTCH TOUGHNESS REQUIREMENTS FOR STRUCTURE STEEL: MEMBERS AS SPECIFIED IN SECTION 836.01 (b) 1 OF CURRENT ALABAMA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS SHALL MEET THE LONGITUDINAL CHARPY V-NOTCH TEST. A MEMBER SHALL BE ORDERED WITH ADDITIONAL LENGTH SO THE CONTRACTOR CAN FURNISH THE BUREAU OF MATERIALS AND TESTS AN EIGHTEEN (18) INCH SAMPLE FOR CHECK TESTING.

PRESTRESSED CONCRETE GIRDERS WITH POURED-IN-PLACE DECK:

(A) SHOP DRAWINGS: SHOP DRAWINGS SHALL BE SUBMITTED AND SHALL SHOW A COMPLETE DETENSIONING SCHEDULE THAT WILL MINIMIZE TENSION IN THE CONCRETE DURING RELEASE OF THE STRANDS. DETAILED CONCRETE STRESSES DURING EACH OPERATION OF DETENSIONING SHALL BE SUBMITTED WITH THE DRAWINGS. THE SHOP DRAWINGS SHALL SHOW COMPLETE GIRDER DETAILS INCLUDING SHIELDING AND ALL REINFORCING AND STRUCTURAL STEEL.

(B) FORMS: ALL GIRDERS SHALL BE CAST ON CONCRETE FLOORED PALLETS AND IN METAL FORMS.

(C) FINISH: THE ENTIRE TOP OF THE GIRDERS SHALL BE SCRUBBED TRANSVERSELY TO A FULL MAGNITUDE OF APPROXIMATELY 1/4" AT THE TIME OF INITIAL SET TO REMOVE ALL LAITANCE AND TO PROVIDE A ROUGHENED SURFACE. THE USE OF ALL MEMBRANE CURING COMPOUND SHALL NOT BE PERMITTED ON THE TOP OF THE TOP FLANGE OF PRESTRESSED GIRDERS.

(D) SCARIFYING: GIRDER SIDES AT EDGE BEAMS/END WALLS SHALL BE SCARIFIED FOR BONDING AFTER GIRDERS HAVE BEEN ERECTED AND FORM LINES HAVE BEEN ESTABLISHED.

INSERTS: INSERTS ENCASED IN TOP OF EXTERIOR GIERDERS FOR USE IN FORMING OVERHANG WILL BE CONSIDERED ON GIRDER DETAILS SUBMITTED FOR APPROVAL.

MISC. HARDWARE: ACCESSORIES ENCASE IN GIRDERS FOR USE IN ATTACHING ANY TEMPORARY BRACING WILL BE CONSIDERED ON GIRDER DETAILS SUBMITTED FOR APPROVAL. AFTER TEMPORARY BRACING IS REMOVED, ANY HOLES THAT EXIST SHALL BE GROUTED AND SURFACE RUBBED TO A NEAT FINISH.

CONCRETE PEDESTALS: CONCRETE PEDESTALS MAY BE POURED CONCURRENTLY WITH THE CAP OR POURED SEPARATELY IF A TYPE II EPOXY ADHESIVE IS APPLIED TO THE CONSTRUCTION JOINT JUST PRIOR TO POURING THE PEDESTALS.

23. ANCHOR BOLT INSTALLATION: WELLS ARE REQUIRED FOR THE PLACEMENT OF ANCHOR BOLTS. SEE SECTION 508.03 (d) 2e OF THE STANDARD SPECIFICATIONS FOR FURTHER REQUIREMENTS.

24. BRIDGE DECK FINISH: THE FINAL BRIDGE DECK FINISH BEHIND THE SCREED SHALL BE OBTAINED BY BURLAP DRAG TO MATCH THE EXISTING DECK FINISH.

(25. YEAR OF COMPLETION AND REFERENCE MARK: THE YEAR OF COMPLETION OF THIS STRUCTURE, AND THE PERMANENT REFERENCE MARK, AS SHOWN ON BRIDGE SPECIAL PROJECT DRAWING BBR-1 OR BBR-2 AND SBD-1 ARE REQUIRED FOR THIS STRUCTURE.

DRILLED SHAFTS: EXTERIOR SURFACES OF PERMANENT CASINGS FOR DRILLED SHAFTS SHALL BE COATED (PRIMER COAT ONLY) FROM THE TOP OF THE CASING DOWN TO ______.

METAL STAY-IN-PLACE FORMS: THIS STRUCTURE HAS BEEN DESIGNED TO ALLOW THE USE OF METAL STAY-IN-PLACE FORMS AT THE CONTRACTOR'S OPTION. SEE SUB-ARTICLE 501.03 (I) OF THE SPECIFICATIONS FOR NECESSARY DETAILS AND REQUIREMENTS. NO FIELD WELDING WILL BE PERMITTED ON STRUCTURAL STEEL MEMBERS UNLESS OTHERWISE NOTED ON THE BRIDGE DRAWINGS. THE CONTRACTOR SHALL EXERCISE CARE WHEN INSTALLING STAY-IN-PLACE FORMS TO INSURE THAT NO FIELD WELDS OR ARC STRIKES OCCUR ON THE STRUCTURAL STEEL MEMBERS.

SUBSURFACE INVESTIGATION: GENERAL SOIL STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON AN ENGINEERING INTERPRETATION OF ALL AVAILABLE SUBSURFACE INFORMATION BY THE GEOTECHNICAL SECTION OF THE BUREAU OF MATERIALS AND TESTS OR FOUNDATION CONSULTANTS AND MAY NOT NECESSARILY REFLECT THE ACTUAL VARIATION IN SUBSURFACE CONDITIONS BETWEEN BORINGS AND SAMPLES. DETAILED DATA AND FIELD INTERPRETATION OF CONDITIONS ENCOUNTERED IN INDIVIDUAL BORINGS ARE SHOWN ON THE BORING LOGS.

REFERENCE PROJECT NUMBER

YEAR NUMBER

THE OBSERVED WATER LEVELS AND CONDITIONS INDICATED ON THE SOIL PROFILE AND BORING LOGS ARE AS RECORDED AT THE TIME OF EXPLORATION. THESE WATER LEVELS AND CONDITIONS MAY VARY CONSIDERABLY, WITH TIME, ACCORDING TO THE PREVAILING CLIMATE, RAINFALL OR OTHER FACTORS AND ARE OTHERWISE DEPENDENT ON THE DURATION OF AND THE METHODS USED IN THE EXPLORATION PROGRAM.

SOUND ENGINEERING JUDGEMENT WAS EXERCISED IN PREPARING THE SUBSURFACE INFORMATION PRESENTED HEREIN. THIS INFORMATION WAS PREPARED AND IS INTENDED FOR STATE DESIGN AND ESTIMATE PURPOSES. ITS PRESENTATION ON THE PLANS OR ELSEWHERE IS FOR THE PURPOSE OF PROVIDING INTENDED USERS WITH ACCESS TO THE SAME INFORMATION AVAILABLE TO THE STATE. THIS SUBSURFACE INFORMATION IS PRESENTED IN GOOD FAITH AND IS NOT INTENDED AS A SUBSTITUTE FOR PERSONAL INVESTIGATION, INDEPENDENT INTERPRETATIONS OR JUDGMENT OF THE CONTRACTOR.

FOUNDATION REPORT: ACCESS TO A FOUNDATION REPORT AND CORE BORINGS FOR THIS PROJECT CAN BE ARRANGED BY CONTACTING THE GEOTECHNICAL SECTION OF THE ALABAMA DEPARTMENT OF TRANSPORTATION.

GIRDER ERECTION: THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE STABILITY AND POSITIONAL CORRECTNESS (PLUMBNESS, ALIGNMENT, ETC.) OF THE GIRDERS DURING ALL PHASES OF CONSTRUCTION. ANY TEMPORARY BRACING AND/OR SUPPORT DEEMED NECESSARY BY THE CONTRACTOR TO ENSURE THE ABOVE UNTIL CONSTRUCTION IS COMPLETE SHALL BE PROVIDED BY THE CONTRACTOR AT NO ADDITIONAL EXPENSE TO THE STATE. WORKING DRAWINGS FOR BRACING SHALL BE SUBMITTED IN ACCORDANCE WITH ARTICLE 501.03 (I) OF THE STANDARD SPECIFICATIONS.

FIELD WELDING: NO FIELD WELDING WILL BE PERMITTED ON STEEL SUPER-STRUCTURE ELEMENTS UNLESS OTHERWISE NOTED ON THE BRIDGE PLANS OR APPROVED IN WRITING BY THE BRIDGE ENGINEER.

OLD BRIDGE REMOVAL: IN ADDITION TO THE SPECIFICATION REQUIREMENTS FOR REMOVING THE OLD BRIDGE, ANY OF THE EXISTING BRIDGE SUBSTRUCTURE THAT INTERFERES WITH THE CONSTRUCTION OF THE REQUIRED BRIDGE SHALL BE REMOVED AS DIRECTED BY THE ENGINEER. ALL COSTS FOR THIS WORK SHALL BE INCLUDED IN PAY ITEM 206A, REMOVAL OF OLD BRIDGE.

NOTE: REFER TO BRIDGE PLANS FOR NOTE NUMBERS APPLICABLE TO THE PROJECT AND FOR SPECIAL NOTES.

ASSISTANT BRIDGE ENGINEER	BRIDGE ENGINEER
(1) 1. Com 11/20/2020	Wer tokano a/rope
DATE	DATE

ALABAMA DEPARTMENT **OF TRANSPORTATION**

1. REVISE NOTE 10 KCM 8/13/19

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: PROJECT NO. COUNTY(S)

MENT OF TRANSPORTATION AND ARE NOT TO BE COPIED, REPRODUCED, ALTERED OR USED BY ANYONE, OR ANY ORGANIZATION, WITHOUT THE EXPRESSED WRITTE CONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE OF HESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

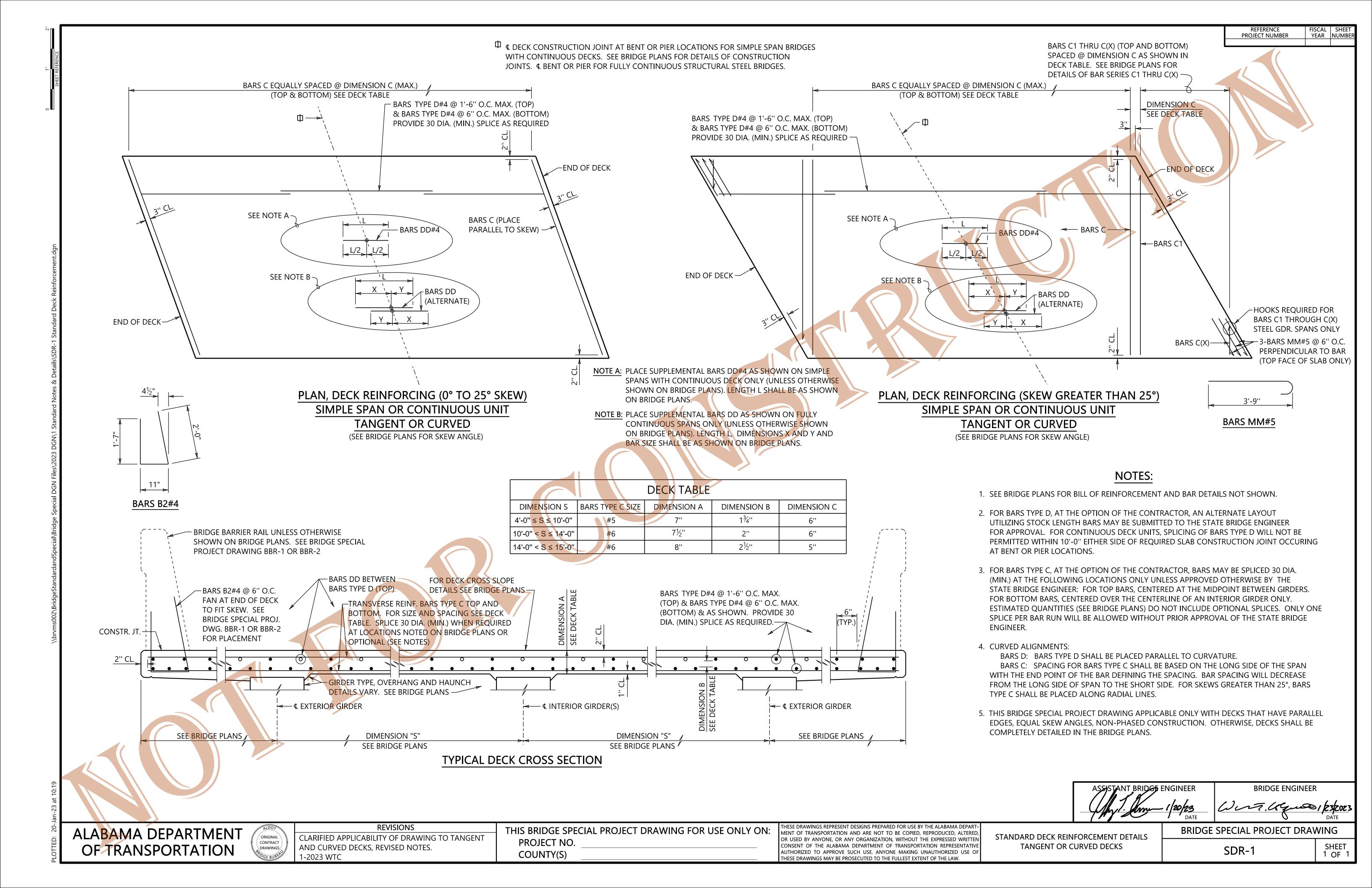
STANDARD BRIDGE NOTES

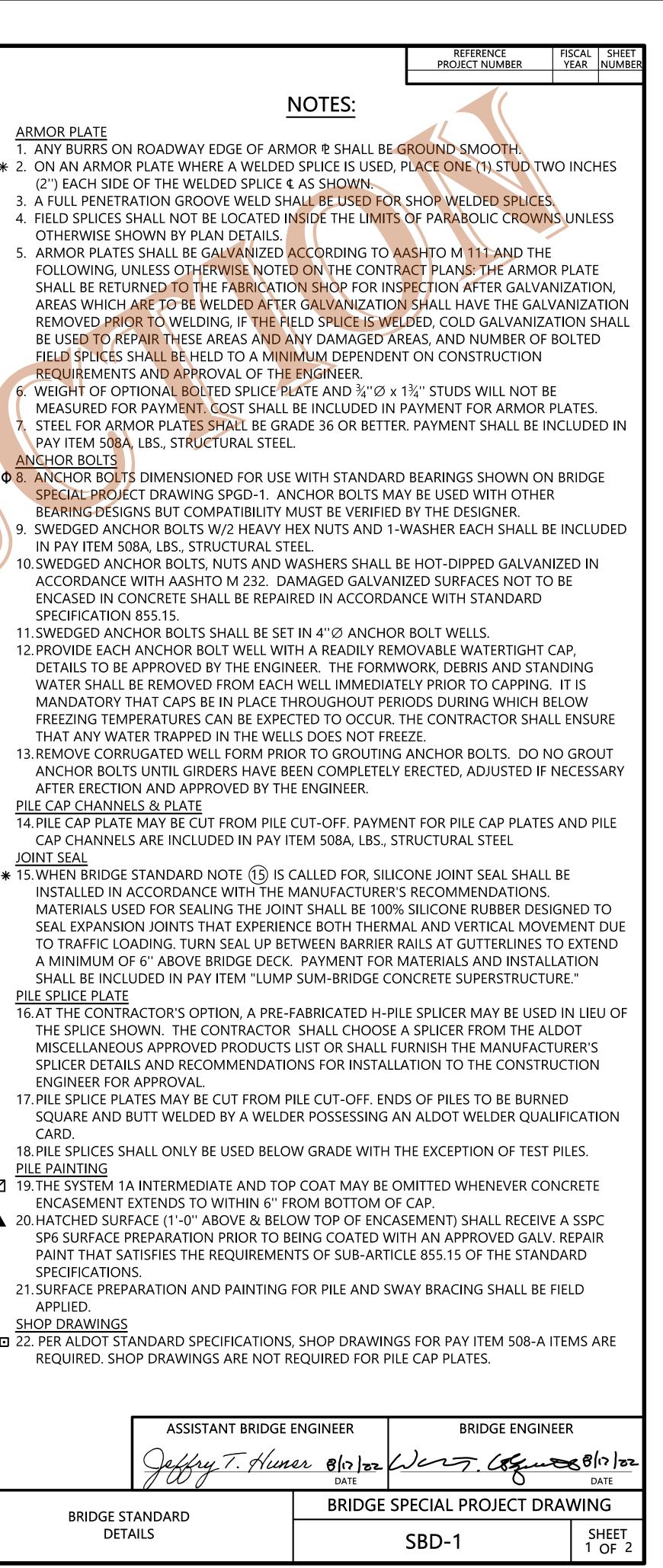
BRIDGE SPECIAL PROJECT DRAWING

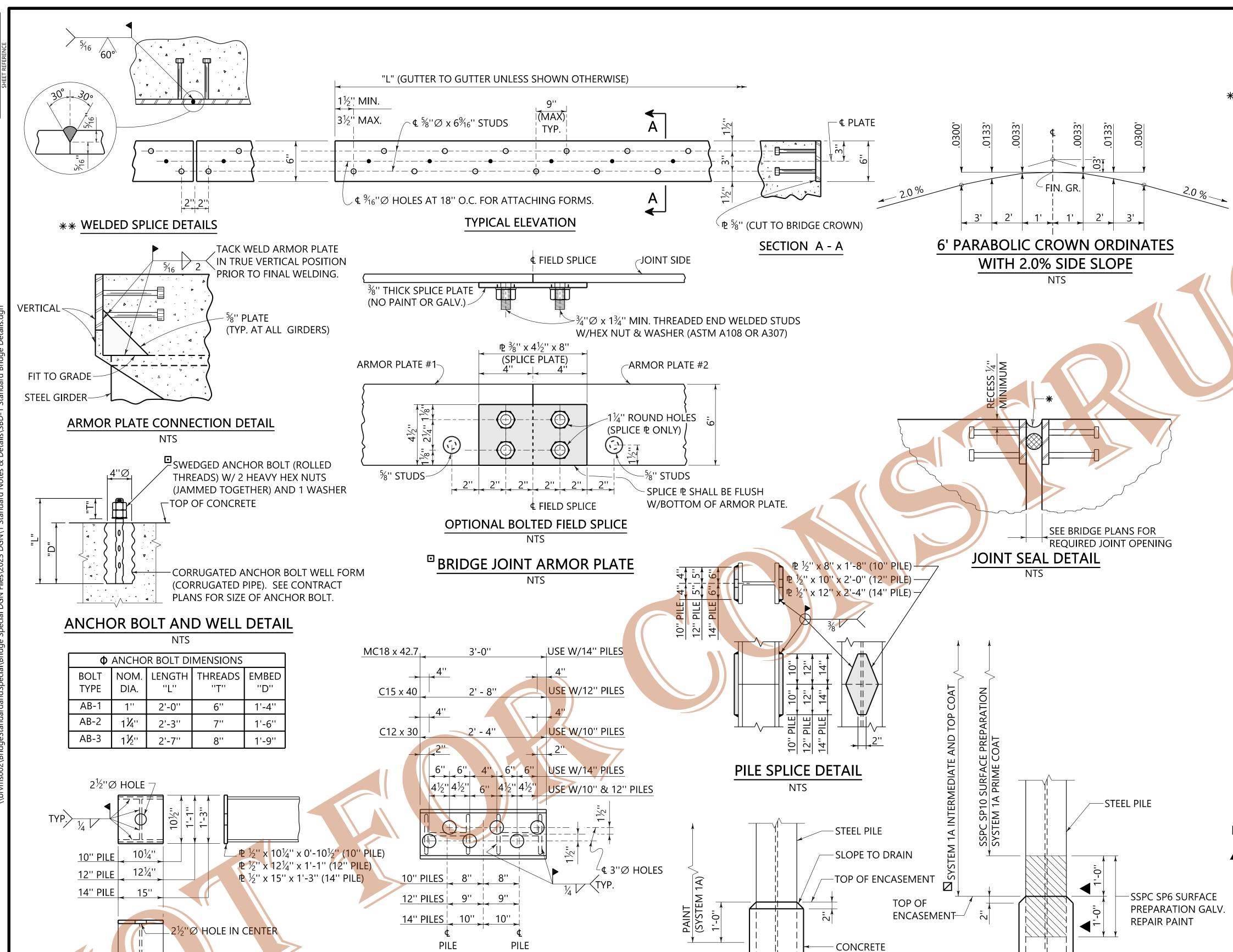
SBN-1

SHEET 1 OF 1

REVISIONS







ARMOR PLATE 1. ANY BURRS ON ROADWAY EDGE OF ARMOR ® SHALL BE GROUND SMOOTH

** 2. ON AN ARMOR PLATE WHERE A WELDED SPLICE IS USED, PLACE ONE (1) STUD TWO INCHES (2") EACH SIDE OF THE WELDED SPLICE € AS SHOWN.

3. A FULL PENETRATION GROOVE WELD SHALL BE USED FOR SHOP WELDED SPLICES.

- 4. FIELD SPLICES SHALL NOT BE LOCATED INSIDE THE LIMITS OF PARABOLIC CROWNS UNLESS OTHERWISE SHOWN BY PLAN DETAILS.
- 5. ARMOR PLATES SHALL BE GALVANIZED ACCORDING TO AASHTO M 111 AND THE FOLLOWING, UNLESS OTHERWISE NOTED ON THE CONTRACT PLANS: THE ARMOR PLATE SHALL BE RETURNED TO THE FABRICATION SHOP FOR INSPECTION AFTER GALVANIZATION, AREAS WHICH ARE TO BE WELDED AFTER GALVANIZATION SHALL HAVE THE GALVANIZATION REMOVED PRIOR TO WELDING, IF THE FIELD SPLICE IS WELDED, COLD GALVANIZATION SHALL BE USED TO REPAIR THESE AREAS AND ANY DAMAGED AREAS, AND NUMBER OF BOLTED FIELD SPLICES SHALL BE HELD TO A MINIMUM DEPENDENT ON CONSTRUCTION REQUIREMENTS AND APPROVAL OF THE ENGINEER
- 6. WEIGHT OF OPTIONAL BOLTED SPLICE PLATE AND $\frac{3}{4}$ " $\propto 1\frac{3}{4}$ " STUDS WILL NOT BE MEASURED FOR PAYMENT. COST SHALL BE INCLUDED IN PAYMENT FOR ARMOR PLATES.
- 7. STEEL FOR ARMOR PLAT<mark>ES SHA</mark>LL BE GRADE 36 OR BETTER. PAYMENT SHALL BE INCLUDED IN PAY ITEM 508A, LBS., STRUCTURAL STEEL. ANCHOR BOLTS
- SPECIAL PROJECT DRAWING SPGD-1. ANCHOR BOLTS MAY BE USED WITH OTHER BEARING DESIGNS BUT COMPATIBILITY MUST BE VERIFIED BY THE DESIGNER
- 10. SWEDGED ANCHOR BOLTS, NUTS AND WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH AASHTO M 232. DAMAGED GALVANIZED SURFACES NOT TO BE ENCASED IN CONCRETE SHALL BE REPAIRED IN ACCORDANCE WITH STANDARD SPECIFICATION 855.15.
- 11.SWEDGED ANCHOR BOLTS SHALL BE SET IN 4"Ø ANCHOR BOLT WELLS.
- 12. PROVIDE EACH ANCHOR BOLT WELL WITH A READILY REMOVABLE WATERTIGHT CAP, DETAILS TO BE APPROVED BY THE ENGINEER. THE FORMWORK, DEBRIS AND STANDING WATER SHALL BE REMOVED FROM EACH WELL IMMEDIATELY PRIOR TO CAPPING. IT IS MANDATORY THAT CAPS BE IN PLACE THROUGHOUT PERIODS DURING WHICH BELOW FREEZING TEMPERATURES CAN BE EXPECTED TO OCCUR. THE CONTRACTOR SHALL ENSURE THAT ANY WATER TRAPPED IN THE WELLS DOES NOT FREEZE.
- 13. REMOVE CORRUGATED WELL FORM PRIOR TO GROUTING ANCHOR BOLTS. DO NO GROUT ANCHOR BOLTS UNTIL GIRDERS HAVE BEEN COMPLETELY ERECTED, ADJUSTED IF NECESSARY AFTER ERECTION AND APPROVED BY THE ENGINEER.

PILE CAP CHANNELS & PLATE

- 14. PILE CAP PLATE MAY BE CUT FROM PILE CUT-OFF. PAYMENT FOR PILE CAP PLATES AND PILE CAP CHANNELS ARE INCLUDED IN PAY ITEM 508A, LBS., STRUCTURAL STEEL JOINT SEAL
- * 15. WHEN BRIDGE STANDARD NOTE (15) IS CALLED FOR, SILICONE JOINT SEAL SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. MATERIALS USED FOR SEALING THE JOINT SHALL BE 100% SILICONE RUBBER DESIGNED TO SEAL EXPANSION JOINTS THAT EXPERIENCE BOTH THERMAL AND VERTICAL MOVEMENT DUE TO TRAFFIC LOADING. TURN SEAL UP BETWEEN BARRIER RAILS AT GUTTERLINES TO EXTEND A MINIMUM OF 6" ABOVE BRIDGE DECK. PAYMENT FOR MATERIALS AND INSTALLATION SHALL BE INCLUDED IN PAY ITEM "LUMP SUM-BRIDGE CONCRETE SUPERSTRUCTURE." PILE SPLICE PLATE
- 16.AT THE CONTRACTOR'S OPTION, A PRE-FABRICATED H-PILE SPLICER MAY BE USED IN LIEU OF THE SPLICE SHOWN. THE CONTRACTOR SHALL CHOOSE A SPLICER FROM THE ALDOT MISCELLANEOUS APPROVED PRODUCTS LIST OR SHALL FURNISH THE MANUFACTURER'S SPLICER DETAILS AND RECOMMENDATIONS FOR INSTALLATION TO THE CONSTRUCTION ENGINEER FOR APPROVAL
- 17. PILE SPLICE PLATES MAY BE CUT FROM PILE CUT-OFF. ENDS OF PILES TO BE BURNED SQUARE AND BUTT WELDED BY A WELDER POSSESSING AN ALDOT WELDER QUALIFICATION
- 18. PILE SPLICES SHALL ONLY BE USED BELOW GRADE WITH THE EXCEPTION OF TEST PILES. PILE PAINTING
- ☐ 19. THE SYSTEM 1A INTERMEDIATE AND TOP COAT MAY BE OMITTED WHENEVER CONCRETE ENCASEMENT EXTENDS TO WITHIN 6" FROM BOTTOM OF CAP.
- ▲ 20. HATCHED SURFACE (1'-0" ABOVE & BELOW TOP OF ENCASEMENT) SHALL RECEIVE A SSPC SP6 SURFACE PREPARATION PRIOR TO BEING COATED WITH AN APPROVED GALV. REPAIR PAINT THAT SATISFIES THE REQUIREMENTS OF SUB-ARTICLE 855.15 OF THE STANDARD SPECIFICATIONS.
 - 21. SURFACE PREPARATION AND PAINTING FOR PILE AND SWAY BRACING SHALL BE FIELD APPLIED. SHOP DRAWINGS
- 22. PER ALDOT STANDARD SPECIFICATIONS, SHOP DRAWINGS FOR PAY ITEM 508-A ITEMS ARE REQUIRED. SHOP DRAWINGS ARE NOT REQUIRED FOR PILE CAP PLATES.

ASSISTANT BRIDGE ENGINEER fry T. Huner 8/17/02 West Comes 8/17/02

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: PROJECT NO.

PILE PAINTING DETAIL

(OPTION "A")

OR USED BY ANYONE, OR ANY ORGANIZATION, WITHOUT THE EXPRESSED WRITTE CONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE O HESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

PILE PAINTING DETAIL

(OPTION "B")

-CONCRETE

ENCASEMENT

BRIDGE STANDARD **DETAILS**

ALABAMA DEPARTMENT **OF TRANSPORTATION**

PILE CAP PLATE

CONTRACT

REVISED ANCHOR BOLT AND WELL DETAIL WTC 8-17-2022

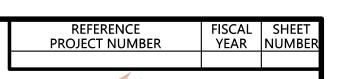
REVISIONS

[□] PILE CAP CHANNEL

COUNTY(S)

ENCASEMENT

PILE PAINTING DETAIL NOTES



NOTES: MISCELLANEOUS SLAB DETAILS

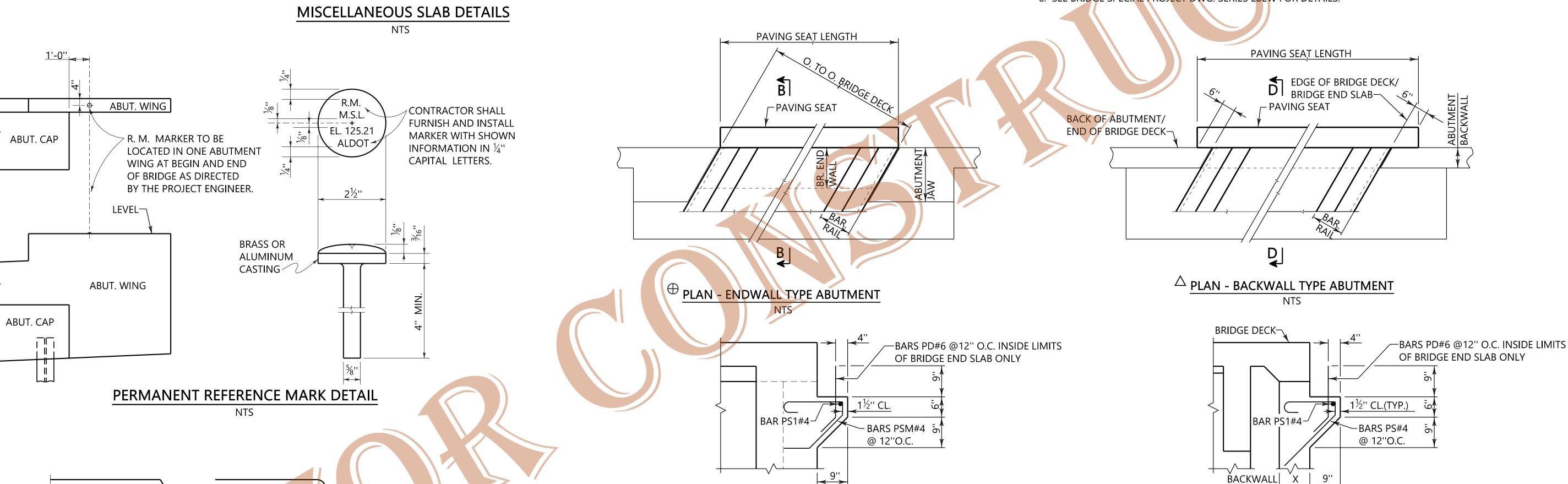
1. DRAIN SHALL BE WHITE OR GRAY PVC (SANITARY) SEWER PIPE (ARTICLE 854.11). LOCATE AND ATTACH TWO (2) 1" x 1" (MIN.) STAYS 180° APART, AS SHOWN. STAYS SHALL BE MADE FROM PVC PIPE AND BONDED WITH PVC SOLVENT CEMENT. OMIT DRAINS OVER TRAFFIC LANES, UNPROTECTED SLOPE FILLS, HIGH SIDE IF SUPERELEVATED AND WITHIN 10'-0" OF BRIDGE ENDS AND 5'-0" OF INTERIOR BRIDGE JOINTS. SPACE @ 5'-0" O.C. UNLESS SHOWN OTHERWISE ON THE CONTRACT PLANS.

PERMANENT REFERENCE MARK

- 2. BRASS MARKER TO BE CAST IN ONE PIECE, OUT OF LEADED-SEMI-RED BRASS ASTM B584 ALLOY 848 OR APPROVED EQUAL. ALUMINUM DISC SHALL BE ALUMINUM ALLOY CS 208 IN ACCORDANCE WITH ASTM B26 OR APPROVED EQUAL.
- 3. INFORMATION AND FIELD STENCILING (ILLUSTRATED IN LIGHT WT. LETTERS) SHALL BE THE RESPONSIBILITY OF THE STATE (COUNTY ON COUNTY PROJECTS).

PAVEMENT SEAT

- 4. PAVING SEATS NOT USED WHEN BRIDGE JOINT IS ON INSIDE OF ABUTMENT BACKWALL. SEE BRIDGE SPECIAL PROJECT DWG. TYPE BES-450(IJ)
- ⊕ 5. APPLIES TO BRIDGE JOINT ON OUTSIDE OF ABUTMENT BACKWALL ONLY. SEE BRIDGE SPECIAL PROJECT DWG. TYPE BES-450(OJ)
- 6. SEE BRIDGE SPECIAL PROJECT DWG. SERIES EBEW FOR DETAILS.



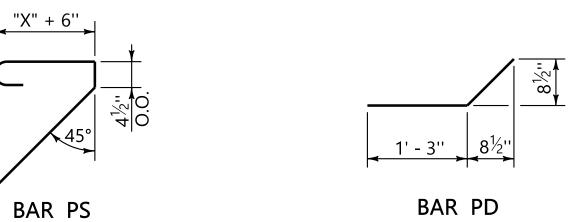
1" MIN.

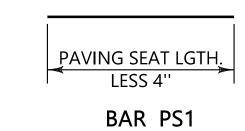
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VIEW C-C

-4''Ø DRAIN (OPTION TWO)

PAVING SEAT DETAILS





TYP. END OF DECK REINFORCEMENT DETAIL WHEN BRIDGE IS SKEWED GREATER THAN 25° NTS

ALABAMA DEPARTMENT

OF TRANSPORTATION

SLAB MAY BE POURED

PROVIDING 2" RISER AT

CONTRACTOR'S OPTION

CONTINUOUS

DRIP BEAD

CONTRACT

REVISED SKEW DEGREE IN DECK REINFORCEMENT KCM 12/14/21

REVISIONS

BARS MM

HOOKS REQUIRED FOR TRANSVERSE REINF. ON

STEEL GDR. SPANS ONLY

B-BARS MM#5 @ 6" O.C. PERPENDICULAR TO BAR

(TOP FACE OF SLAB ONLY)

-1"x1" (MIN.) STAYS

-4"Ø DRAIN (OPTION ONE)

COUNTY(S)

SECTION B-B

NTS

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BRIDGE STANDARD

BRIDGE SPECIAL PROJECT DRAWING SHEET 2 OF 2

LONGITUDINAL

TRANSVERSE

REINFORCEMENT

REINFORCEMENT-

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: PROJECT NO.

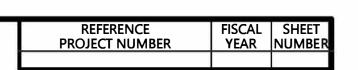
BAR PSM#4

DETAILS

SECTION D-D

NTS

SBD-1



BARRIER REINFORCEMENT

1. FAN BARS B1 AND B2 AT SKEWED JOINTS AS REQ'D. TO FIT SKEW. LENGTHS OF BARS BL#4 SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR MAY, AT HIS OPTION, PLACE BARS BL CONTINOUSLY AND CUT AT JOINT LOCATIONS. TWO INCH (2") END COVER SHALL BE PROVIDED AT ALL JOINT LOCATIONS. SPLICE BARS BL#4 30 DIA. (MIN.). MINIMUM LENGTH OF BARS BL 10'-0".

YEAR OF COMPLETION

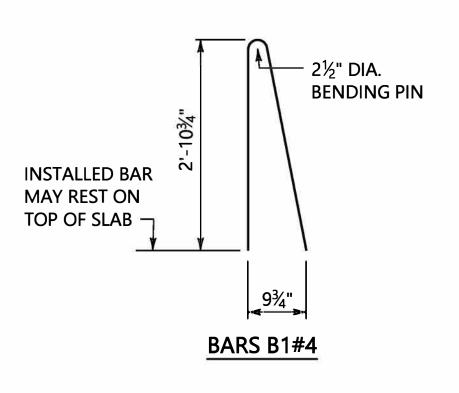
- 2. THE YEAR OF COMPLETION OF BRIDGE SHALL BE CONSTRUCTED ON THE INSIDE FACE OF BARRIER RAIL AT BEGIN AND END, AS SHOWN. NUMERALS SHALL BE OF A STANDARD ARIAL FONT, SIX INCH (6") TALL WITH AN APPROXIMATE 3/4" TO 1" THICKNESS.
- 3. NUMERALS MAY BE CONSTRUCTED OF A SUITABLE MATERIAL AND ATTACHED TO FORMS IN ORDER TO CAST THREE-EIGHTHS INCH (%") DEEP INDENTATIONS IN CONCRETE. EDGES OF NUMERALS SHOULD HAVE AN INWARD BEVEL TO FACILITATE REMOVAL OF FORMS.
- 4. UPON APPROVAL OF THE ENGINEER, THE CONTRACTOR MAY USE PREFORMED, BLACK THREE-EIGHTHS INCH (%") MINIMUM DEPTH NUMERALS THAT ARE PERMANENTLY EMBEDDED WITHIN THE BARRIER RAIL SO THE FACE OF THE NUMERALS ARE FLUSH WITH THE CONCRETE FACE. EDGES OF NUMERALS TO HAVE AN OUTWARD BEVEL TO ENSURE PERMANENT EMBEDMENT.
- 5. COST OF NUMERALS SHALL BE INCLUDED IN THE PAY ITEM 510J, BRIDGE BARRIER RAIL, TYPE 36" SINGLE SLOPE.

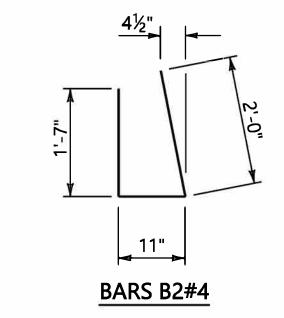
OPEN JOINTS IN BARRIER

- 6. IN ADDITION TO THE OPEN JOINTS SHOWN ON THE BRIDGE DRAWINGS AT THE BEGINNING AND END OF SPANS, OPEN JOINTS IN BARRIER RAILS SHALL BE LOCATED AS FOLLOWS: FOR SPANS UP TO 60 FEET IN LENGTH, PROVIDE ONE OPEN JOINT AT MID-POINT OF THE SPAN. FOR SPANS EXCEEDING 60 FEET IN LENGTH, PROVIDE ONE OPEN JOINT AT MID-POINT OF THE SPAN AND ADDITIONAL JOINTS AT EQUAL SPACES NOT TO EXCEED 30 FEET BETWEEN
- 7. FOR CONTINUOUS SPAN UNITS, OPEN JOINTS IN BARRIER RAILS SHALL ALSO BE PROVIDED AT INTERIOR BENT LOCATIONS.
- 8. JOINT OPENINGS SHALL BE THREE-QUARTERS INCH ($rac{3}{4}$ ") IN WIDTH WHENEVER BARRIER RAILS ARE CONSTRUCTED BY MEANS OTHER THAN A SLIP FORM EXTRUSION MACHINE. **WATERSTOPS**
- 9. OPEN JOINTS IN BARRIER RAIL TO BE SEALED WITH A 6" x 3" x 3/8" WATERSTOP, EXCEPT AS NOTED BELOW. WATERSTOP MATERIAL SHALL CONFORM TO REQUIREMENTS OF SUB-ARTICLE 832.05(b) OF THE STANDARD SPECIFICATIONS. WATERSTOP SHALL BE BONDED TO BRIDGE DECK WITH AN APPROVED ADHESIVE MEETING REQUIREMENTS OF SUB-ARTICLE 832.03(a)2b OF THE STANDARD SPECIFICATIONS.
- 10. WATERSTOP NOT REQUIRED ON HIGH SIDE OF SLOPED DECKS, WHEN CLASS 3 SURFACE FINISH IS REQUIRED, OR WHEN JOINT OPENING IS SAW CUT PER SUB-ARTICLE 510.03(c)6j OF THE STANDARD SPECIFICATIONS.
- 11. COST OF WATERSTOPS SHALL BE INCLUDED IN PAY ITEM 510J, BRIDGE BARRIER RAIL TYPE 36" SINGLE SLOPE.

DRAIN BLOCKOUTS

12. DRAIN BLOCKOUTS ARE TO BE USED ONLY WHEN SPECIFIED ON THE CONTRACT PLANS. OMIT DRAINS OVER TRAFFIC LANES, UNPROTECTED SLOPE FILLS, HIGH SIDE IF SUPER-ELEVATED AND WITHIN 10'-0" OF BRIDGE ENDS. NO BLOCKOUT SHALL BE PLACED WITHIN 5'-0" OF AN OPEN JOINT IN THE BARRIER RAIL. SPACE BLOCKOUTS @ 5'-0" O.C. (MAX.) UNLESS SHOWN OTHERWISE ON THE CONTRACT PLANS.



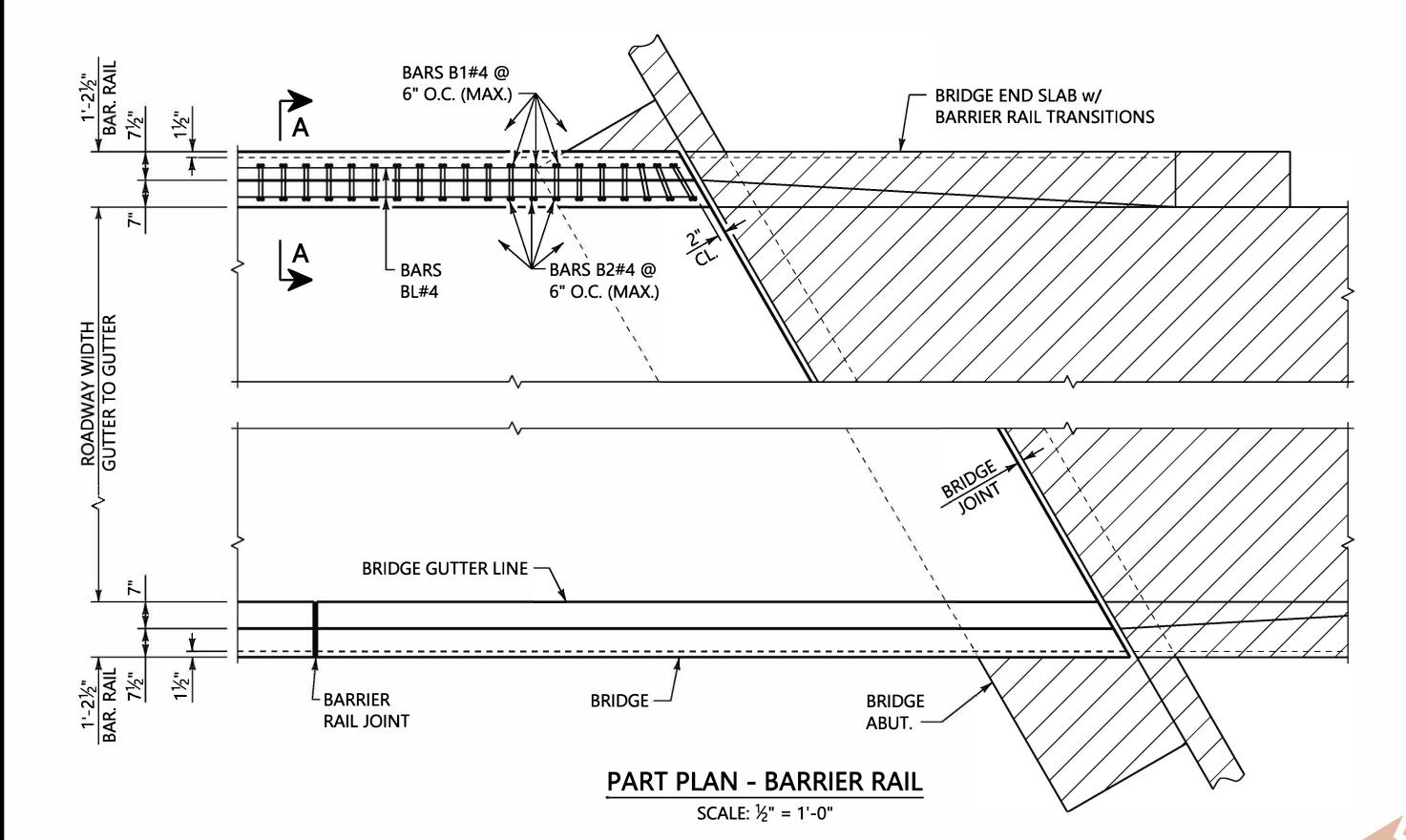


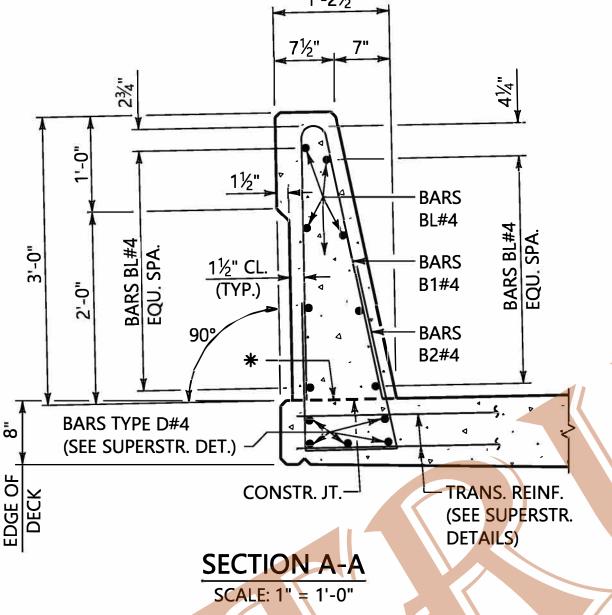
ASSISTANT BRIDGE ENGINEER **BRIDGE ENGINEER**

BRIDGE BARRIER RAIL

BRIDGE SPECIAL PROJECT DRAWING

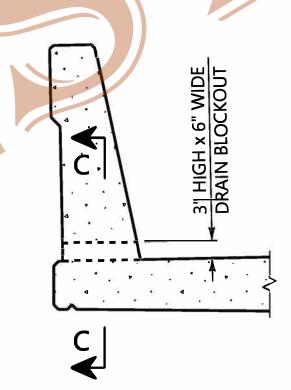
SHEET 1 OF 1





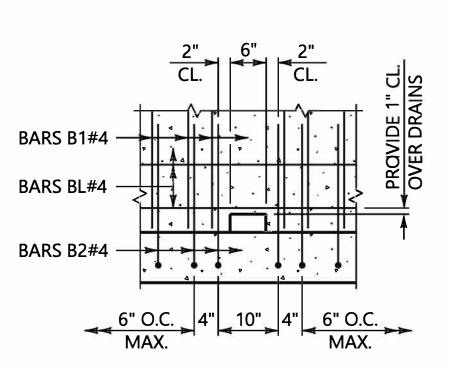
* MATCH SLOPE SHOWN ON TYPICAL BRIDGE

CROSS SECTION OF CONTRACT PLANS.



DRAIN BLOCKOUT

SCALE: $\frac{3}{4}$ " = 1'-0"



SECTION C-C

SCALE: 3/4" = 1'-0"

PART ELEVATION - BARRIER RAIL SCALE: 3/4" = 1'-0"

1" SPACING

ORIGINAL CONTRACT

- 6" x 3" x ¾" WATERSTOP

SECTION B-B

(WATERSTOP)

SCALE: 3/4" = 1'-0"

OPEN JOINT

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON:

END OF RAIL

BRIDGE DECK

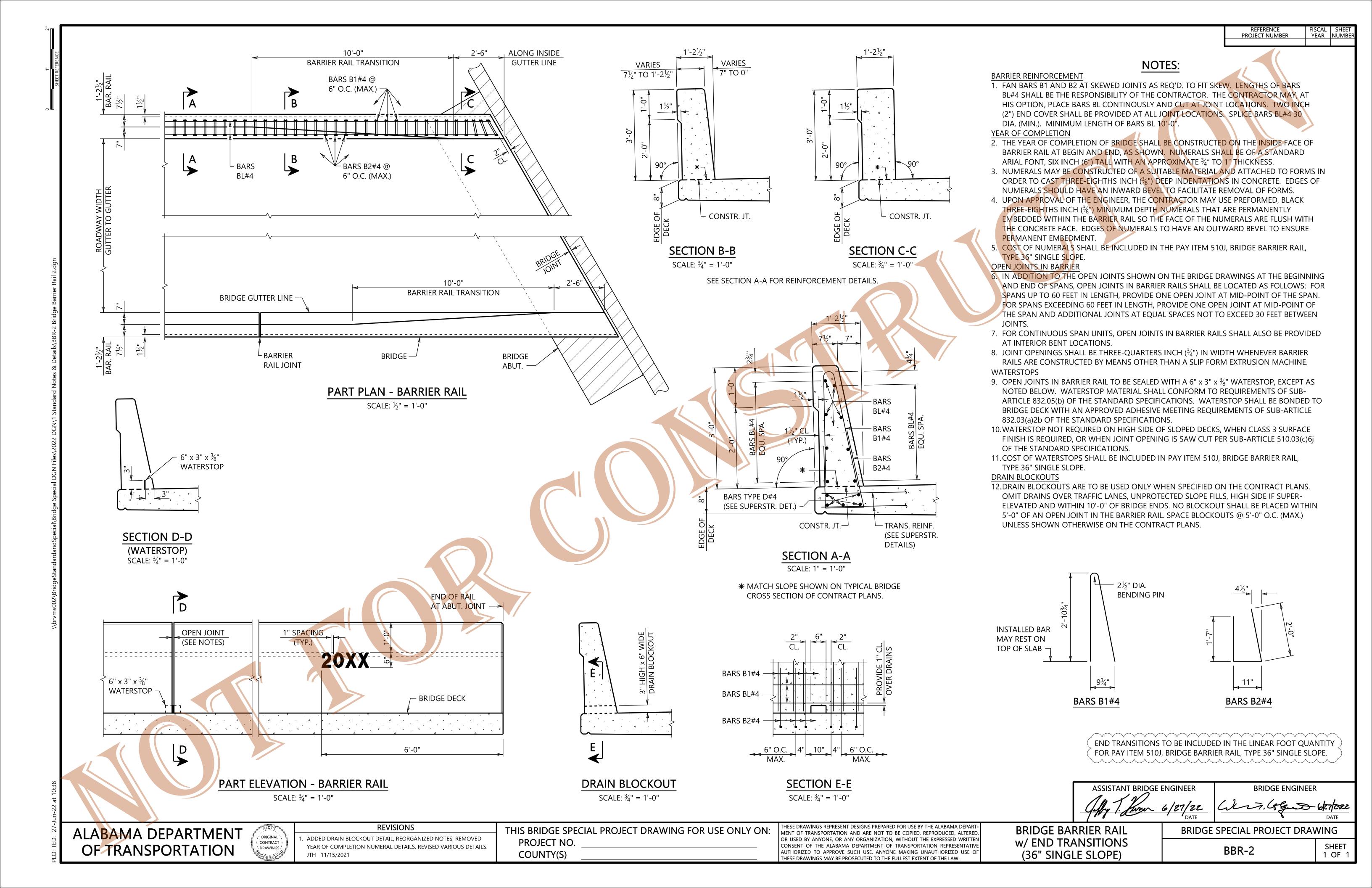
6'-0"

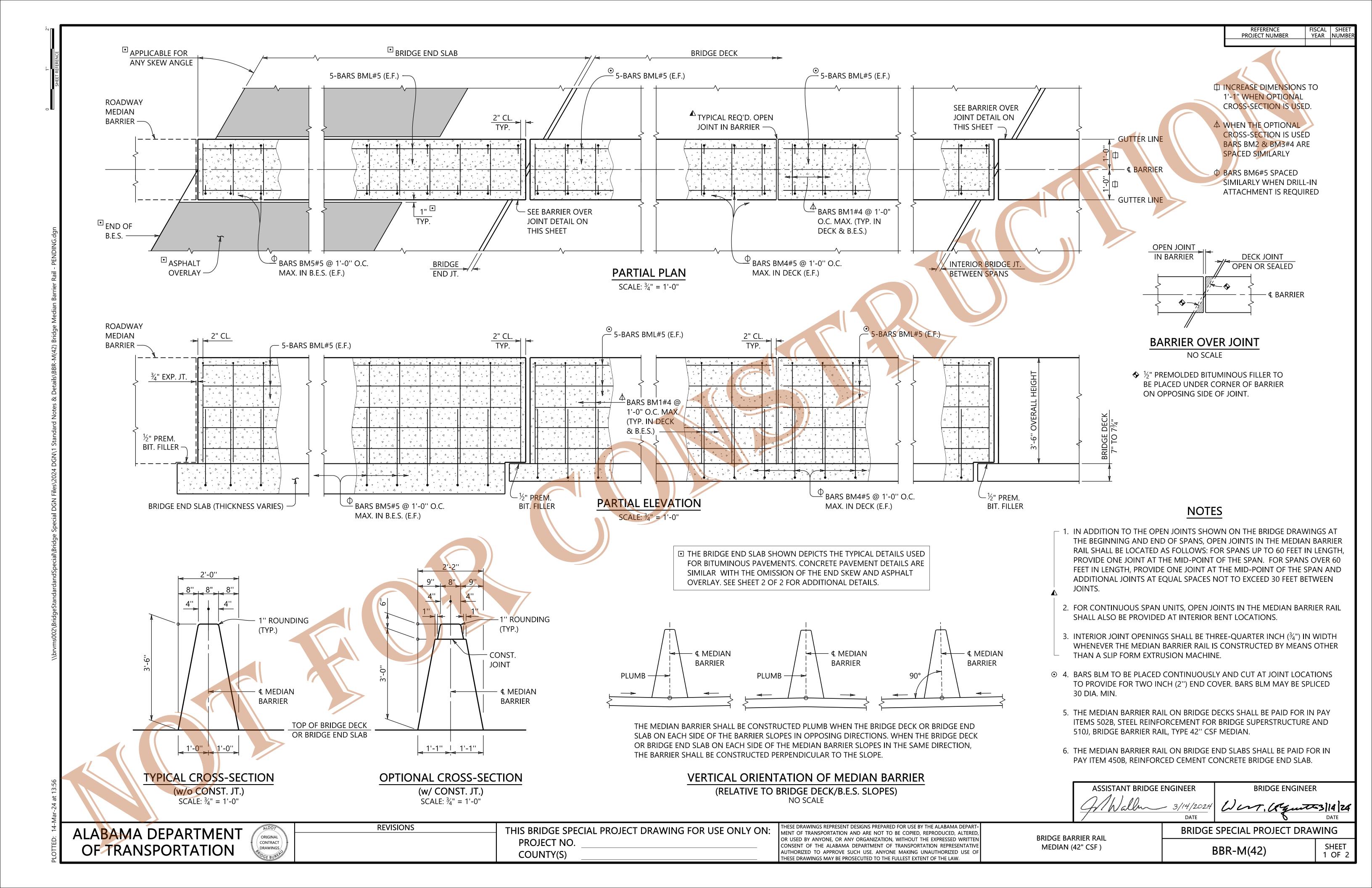
AT ABUT. JOINT →

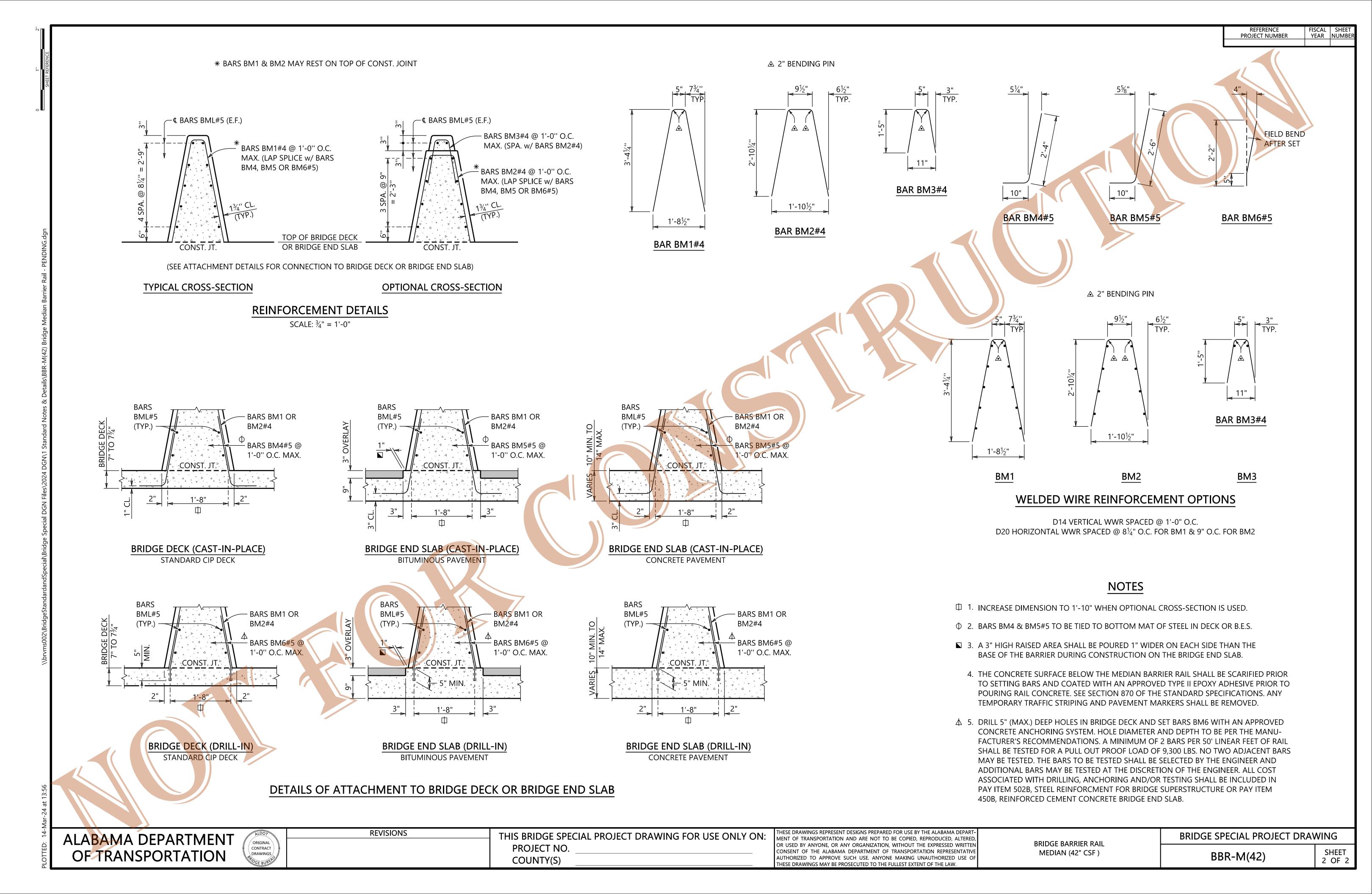
(36" SINGLE SLOPE)

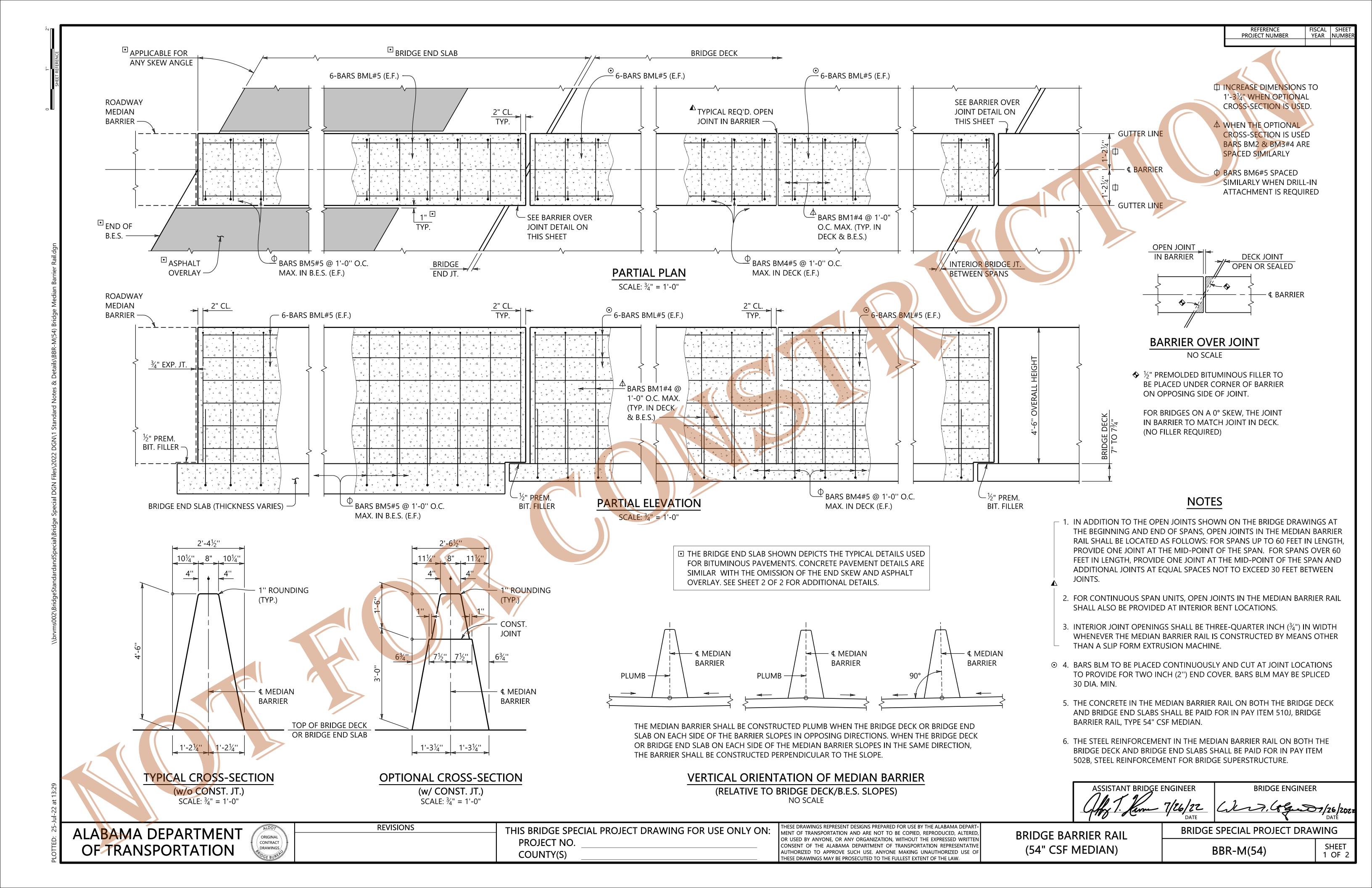
BBR-1

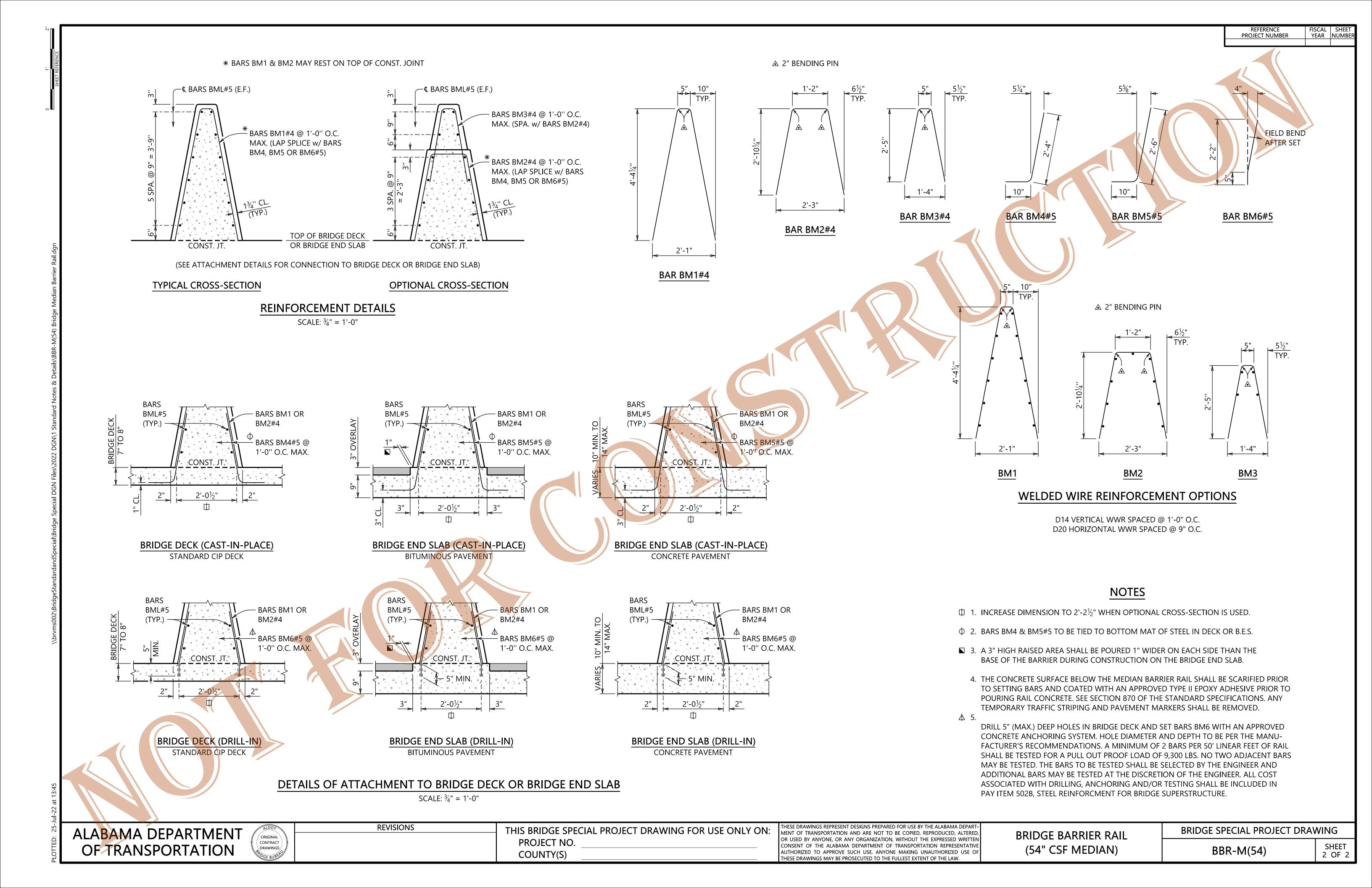
REVISIONS ALABAMA DEPARTMENT ADDED DRAIN BLOCKOUT DETAIL, REORGANIZED NOTES, REMOVED OR USED BY ANYONE, OR ANY ORGANIZATION, WITHOUT THE EXPRESSED WRITTE PROJECT NO. **OF TRANSPORTATION** CONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE YEAR OF COMPLETION NUMERAL DETAILS, REVISED VARIOUS DETAILS. AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE OI COUNTY(S) HESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

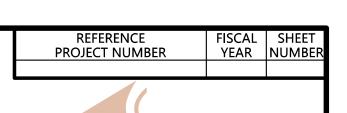




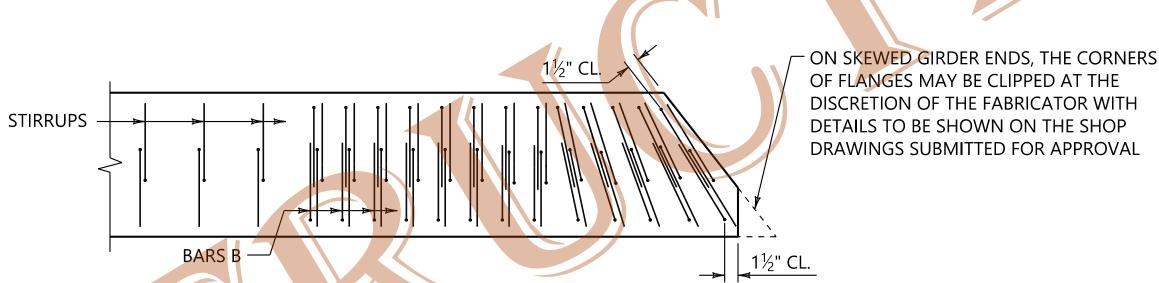








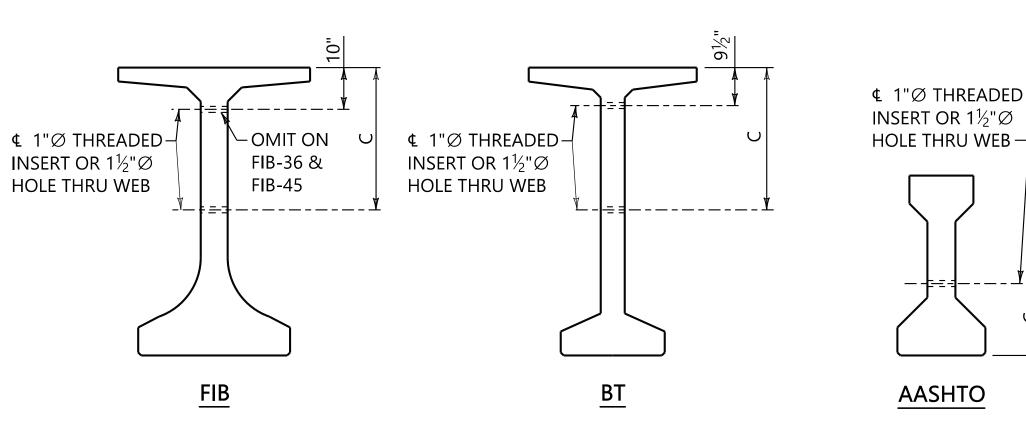
- 1. THREADED INSERTS AND %" \varnothing x 3" CAP SCREWS WITH 1 WASHER EACH SHALL BE INCLUDED IN PAY ITEM 513B - PRETENSIONED-PRESTRESSED CONCRETE GIRDERS.
- 2. CONNECTION ANGLES SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH AASHTO M 111. 7/8" CAP SCREWS AND WASHERS SHALL BE HOT DIP GALVANIZED IN ACCORDANCE WITH AASHTO M 232. DAMAGED GALVANIZED SURFACES NOT TO BE ENCASED IN CONCRETE SHALL BE REPAIRED IN ACCORDANCE WITH STANDARD SPECIFICATION 855.15.
- 3. FOR SEMI-INTEGRAL TYPE ABUTMENTS ONLY, CAST BARS NB 2'-0" INTO END OF GIRDER. BARS MAY BE BENT AFTER REMOVAL OF FORMS.
- 1. FOR SKEWED GIRDER ENDS, PLACE FIRST SET OF STIRRUPS AND BARS B PARALLEL TO GIRDER END AND INCREMENTALLY ADJUST OVER THE FIRST FEW SETS UNTIL 90° TO GIRDER IS OBTAINED.



PLACEMENT OF STIRRUPS AND BARS B AT SKEWED GIRDER ENDS

CONNECTION ANGLE								
HOLE & SLOT SIZE								
BOLT	SLOT							
AB-1	1¼"	1¼" x 7"						
AB-2	1½"	1½" x 7"						
AB-3	1¾"	1¾" x 7"						

GIRDER WEB
END OF GIRDER
ALIGN WITH SKEW
© 1"Ø THREADED INSERT OR 1½"Ø HOLE THRU WEB
TYPICAL PLAN VIEW



DIMENSION	TAB	LE	
GIRDER SHAPE	А	В	С
AASHTO TYPE I AASHTO TYPE I MOD.	4"	2"	1'-4"
AASHTO TYPE I MOD. (III)	4"	4"	1'-4"
AASHTO TYPE I MOD. (BT) AASHTO TYPE I MOD. (BT+)	3½"	6"	1'-3½"
AASHTO TYPE II	4"	6"	1'-6"
AASHTO TYPE II MOD.	4"	8"	1'-6"
AASHTO TYPE III	4"	8"	1'-7½"
AASHTO TYPE III MOD. BT-54 / BT-54 MOD. BT-56 / BT-56 MOD.	3½''	10''	1'-7½"
BT-63 / BT-63 MOD. BT-65 / BT-65 MOD.	3½"	10''	2'-0½"
BT-72 / BT-72 MOD. BT-74 / BT-74 MOD.	3½"	10''	2'-5½"
FIB-36	4"	11''	10''
FIB-45	4"	11''	1'-3''
FIB-54	4"	11''	1'-8''
FIB-63	4"	11''	2'-1''
FIB-72	4"	11''	2'-6''

PLACEMENT	OF THE	READED
INSERTS AND	HOLES	IN WEBS

ASSISTANT BRIDGE ENGINEER	BRIDGE ENGINEER
	114 Com 3114
/ DATE	O DATE

ALABAMA DEPARTMENT **OF TRANSPORTATION**

⊈ GDR.-

REVISIONS REVISED NOTES, TABULATED DIMENSIONS, ADDED FIB DETAILS, AND REMOVED HOLD DOWN DETAIL.

PROJECT NO. COUNTY(S)

ONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE OI HESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

BRIDGE SPECIAL PROJECT DRAWING

SPGD-1

SHEET 1 OF 2

NO GAP AT FIXED END (SEE PLAN DETAILS AND SEE BRIDGE PLANS FOR CONNECTION ANGLES BR. SP. PROJ. DWG. SBD-1) — REQUIRED BEARING TYPE TYPICAL END VIEW CONNECTION ANGLE DETAILS

- **℄** THREADED INSERTS

OR EXP.

- & BEARING &

& ANCHOR BOLTS

BOTTOM FLANGE -

— ⊈ THREADED INSERTS

TYPICAL PLAN VIEW

-BOTTOM FLANGE

- CONNECTION

ANGLES, FIXED

& ANCHOR BOLT —

& ANCHOR

BOLT -

THREADED INSERTS-

BOTTOM OF GIRDER AND

OMIT ON FIB-36

OMIT ON FIB-36 —

& FIB-45

ANCHOR BOLTS

BOTTOM OF CONN. ANGLE

BARS NB#5 **⊈** BARS NB#5— **℄** BARS € GDR.-⊈ GDR. NB#5 -OMIT ON — TYPE I **AASHTO**

⁷⁄₈"∅ GALVANIZED CAP

WITH WASHER

- PROVIDE ½" GAP AT EXPANSION END TO

ALLOW FOR MOVEMENT;

SCREW (3" UNDER HEAD)

PLACEMENT OF BARS NB IN GIRDER END AT ABUTMENTS (SEMI-INTEGRAL TYPE ONLY)

CONTRACT

⊈ BARS

NB#5---

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON:

L 6" x 6" x ½"

PLAN (FIXED)

1'-0"

L 6" x 6" x ½"

PLAN (EXP.)

1'-0" `L 6" x 6" x ½"´

ELEVATION

♦ HOLE AND ANCHOR BOLT

♦ SLOT AND ANCHOR BOLT

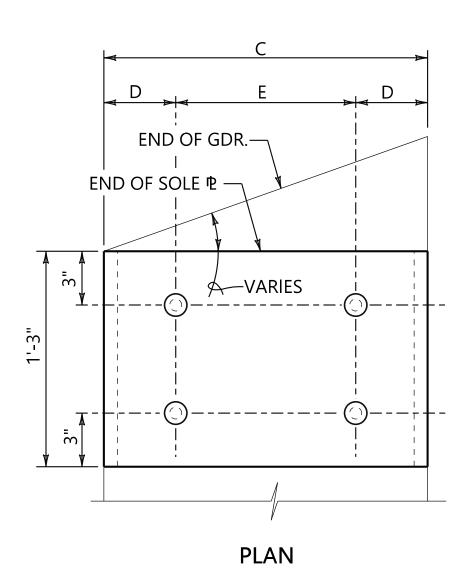
¢ 1¼" x 4"

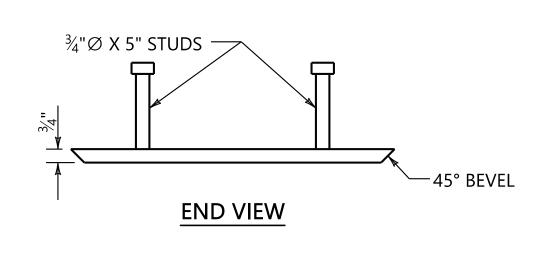
SLOTTED HOLES

(SEE CHART FOR SIZE)

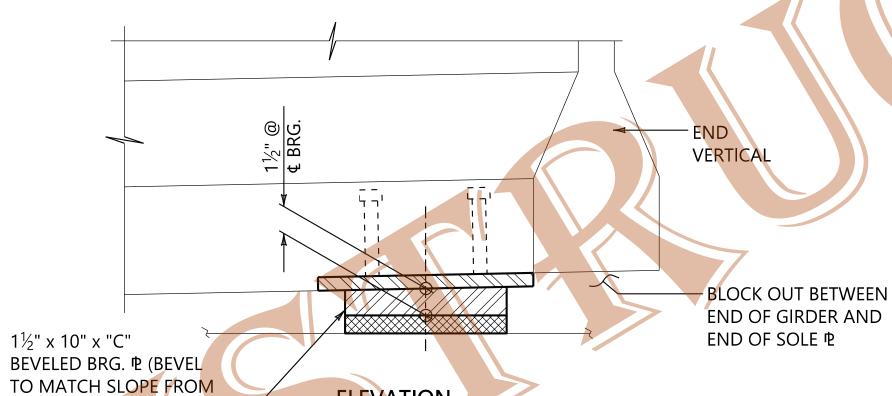
(SEE CHART FOR SIZE) -

STANDARD PRESTRESSED GIRDER DETAILS





-SOLE № (REQUIRED FOR BEARINGS TYPE 4. **OPTIONAL FOR BEARINGS** TYPE 2). 1 I 1 I 1 I 1 I - ELASTOMERIC BEARING PAD **END VIEW**



ELEVATION

BEARING DETAIL

BEARING TO BEARING) -

ELASTOMERIC BEARING PAD DETAIL

ELEVATION

12 GA. STEEL SHIM ⅊ —

 $\frac{1}{8}$ " (TYP. AT EDGES)

SOLE PLATE DETA	ЛL

	ELASTOMERIC BEARINGS TYPE 2											
	ELASTOMERIC BEARING PAD DATA											
ELASTOMERIC	MAXIMUM	MAXIMUM	BEARIN	G PAD DIME	NSIONS		INDIVIDUA	AL LAYERS		REQ	UIRED 12 GU	AGE
BEARING	SPAN	LOAD	THICKNESS	LENGTH	WIDTH	EXTE	RIOR	INTE	ERIOR	STE	EL SHIM PLA	ΓES
MARK	LENGTH	DL + LL	"A"	"B"	WIDIH	NUMBER	THICKNESS	NUMBER	THICKNESS	NUMBER	LENGTH	WIDTH
B1	150 FT.	129 KIPS	1½"	1'-2½"	10"	2	1/4"	2	1/"	3	1'-21/4"	9¾"
B2	200 FT.	154 KIPS	2"	1'-4½"	10"	2	1/4"	3	1/2"	4	1'-41/4"	9¾"
В3	200 FT.	207 KIPS	2"	1'-8½"	10"	2	1/4"	3	1/2"	4	1'-81/4"	9¾"
B4	250 FT.	261 KIPS	2½"	2'-0½"	10"	2	1/4"	4	1/2"	5	2'-01/4"	9¾"
B5	300 FT.	289 KIPS	3"	2'-21/2"	10"	2	1/4"	5	1/2"	6	2'-21/4"	9¾"
В6	300 FT.	430 KIPS	3"	3'-01/2"	10"	2	1/4"	5	1/2"	6	3'-01/4"	93/4"

BEARING USAGE							
MARK	GIRDER SHAPE						
B1 OR VB1	AASHTO TYPE I						
B2 OR VB2	AASHTO TYPE II						
DZ ON VDZ	AASHTO TYPE I MOD.						
	AASHTO TYPE III						
B3 OR VB3	AASHTO TYPE II MOD.						
	AASHTO TYPE I MOD. (III)						
	BULB-TEE						
B4 OR VB4	AASHTO TYPE III MOD.						
	AASHTO TYPE I MOD. (BT & BT+)						
B5 OR VB5	BULB-TEE MOD.						
B6 OR VB6	FLORIDA FIB						

1. SOLE PLATES SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH

AASHTO M 111. BEVELED EDGES OF THE SOLE PLATE TO RECEIVE FIELD

WELDING SHALL BE GROUND TO BARE METAL BEFORE BEING CAST IN

MANUFACTURER OF THE BEARINGS WHENEVER TYPE 4 ELASTOMERIC

THE BRIDGE ENGINEER FOR APPROVAL AND SHALL INCLUDE ALL BEARINGS OF ALL TYPES FOR EACH STRUCTURE. THE LAYOUT SHALL

LOCATE EACH BEARING WITH RESPECT TO UNIQUE IDENTIFICATION

BEARINGS ARE SPECIFIED IN THE BRIDGE PLANS. THE LAYOUT SHALL BE

INCLUDED IN THE BEARING PAD FABRICATION DRAWINGS SUBMITTED TO

NUMBERS AND SHALL INDICATE CORRECT PLACEMENT OF BEARING WITH

THE CONTRACTOR SHALL REMOVE ANY RUST THAT APPEARS IN THE FIELD WELD AREAS OF THE BEARING PLATE AND SOLE PLATE BY WIRE BRUSHING JUST PRIOR TO FIELD WELDING THESE PLATES. ALL DECK POURS SHALL BE

SECTIONS 511 AND 837 OF THE STANDARD SPECIFICATIONS FOR BEARING

PRESTRESSED GIRDER SHAPES AS INDICATED IN TABLE. BEARINGS MAY BE

COMPLETED PRIOR TO WELDING BEARING PLATE TO SOLE PLATE. SEE

ADAPTED FOR USE WITH OTHER MODIFIED SHAPES AS APPROPRIATE.

5. MAXIMUM SPAN LENGTH BASED ON ALLOWABLE SHEAR DEFORMATION OF ELASTOMER. SPAN LENGTH MEASURED FROM & FIXED BEARING TO

6. EXTERIOR LAYER THICKNESS MEASURED FROM OUTSIDE SURFACE OF PAD

TO € SHIM PLATE. INTERIOR LAYER THICKNESS MEASURED FROM € SHIM

4. BEARING MARK INDICATES TYPICAL USAGE WITH ALDOT STANDARD

GIRDER. SOLE PLATES SHALL BE INCLUDED IN PAY ITEM 513B. 2. A BEARING LAYOUT (ERECTION PLAN) SHALL BE PROVIDED BY THE

ELASTOMERIC BEARINGS TYPE 4															
ELASTOMERIC BEARING PAD DATA										SOLE ®	& BEARING	P DATA			
ELASTOMERIC	MAXIMUM MAXIMUM		BEARIN	EARING PAD DIMENSIONS INDIVIDUAL LAYERS REQUIRED 12 GUAGE			IMENSIONS INDIVIDUAL LAYERS REQUIRED				IAGE	LENGTH	CONNI	ECTION	
BEARING	SPAN	LOAD	THICKNESS	LENGTH	WIDTH	EXTE	ERIOR	INTI	ERIOR	STE	EL SHIM PLA	TES	LENGTH "C"	STUD S	PACING
MARK	LENGTH	DL + LL	"A"	"B"		NUMBER	THICKNESS	NUMBER	THICKNESS	NUMBER	LENGTH	WIDTH		"D"	"E"
VB1	150 FT.	129 KIPS	1½"	1'-2½"	10"	2	1/4"	2	1/2"	3	1'-2¼"	9¾"	1'-4"	4"	8"
VB2	200 FT.	154 KIPS	2"	1'-4½"	10"	2	1/4"	3	1/2"	4	1'-4¼"	9¾"	1'-6"	5"	8"
VB3	200 FT.	207 KIPS	2"	1'-8½"	10"	2	1/4"	3	1/2"	4	1'-8¼"	9¾"	1'-10"	5"	1'-0"
VB4	250 FT.	261 KIPS	2½"	2'-0½"	10"	2	1/4"	4	1/2"	5	2'-01/4"	9¾"	2'-2"	5"	1'-4"
VB5	300 FT.	289 KIPS	3"	2'-2½"	10"	2	1/4"	5	1/2"	6	2'-2¼"	9¾"	2'-4"	6"	1'-4"
VB6	300 FT.	430 KIPS	3"	3'-0½"	10"	2	1/4"	5	1/2"	6	3'-01/4"	9¾"	3'-2"	6"	2'-2"

JTH 1/2024

JNW 1/2024

ALABAMA DEPARTMENT OF TRANSPORTATION

REVISIONS REVISED NOTES, BEARING AND BEARING USAGE. REVISED ELEVATION DETAILS AND TABLES.

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: PROJECT NO. COUNTY(S)

THESE DRAWINGS REPRESENT DESIGNS PREPARED FOR USE BY THE ALABAMA DEPAR MENT OF TRANSPORTATION AND ARE NOT TO BE COPIED, REPRODUCED, ALTERED OR USED BY ANYONE, OR ANY ORGANIZATION, WITHOUT THE EXPRESSED WRITTE CONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE OF THESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

STANDARD PRESTRESSED GIRDER DETAILS

NOTES

RESPECT TO BEVELING.

PLATE PREPARATION REQUIREMENTS.

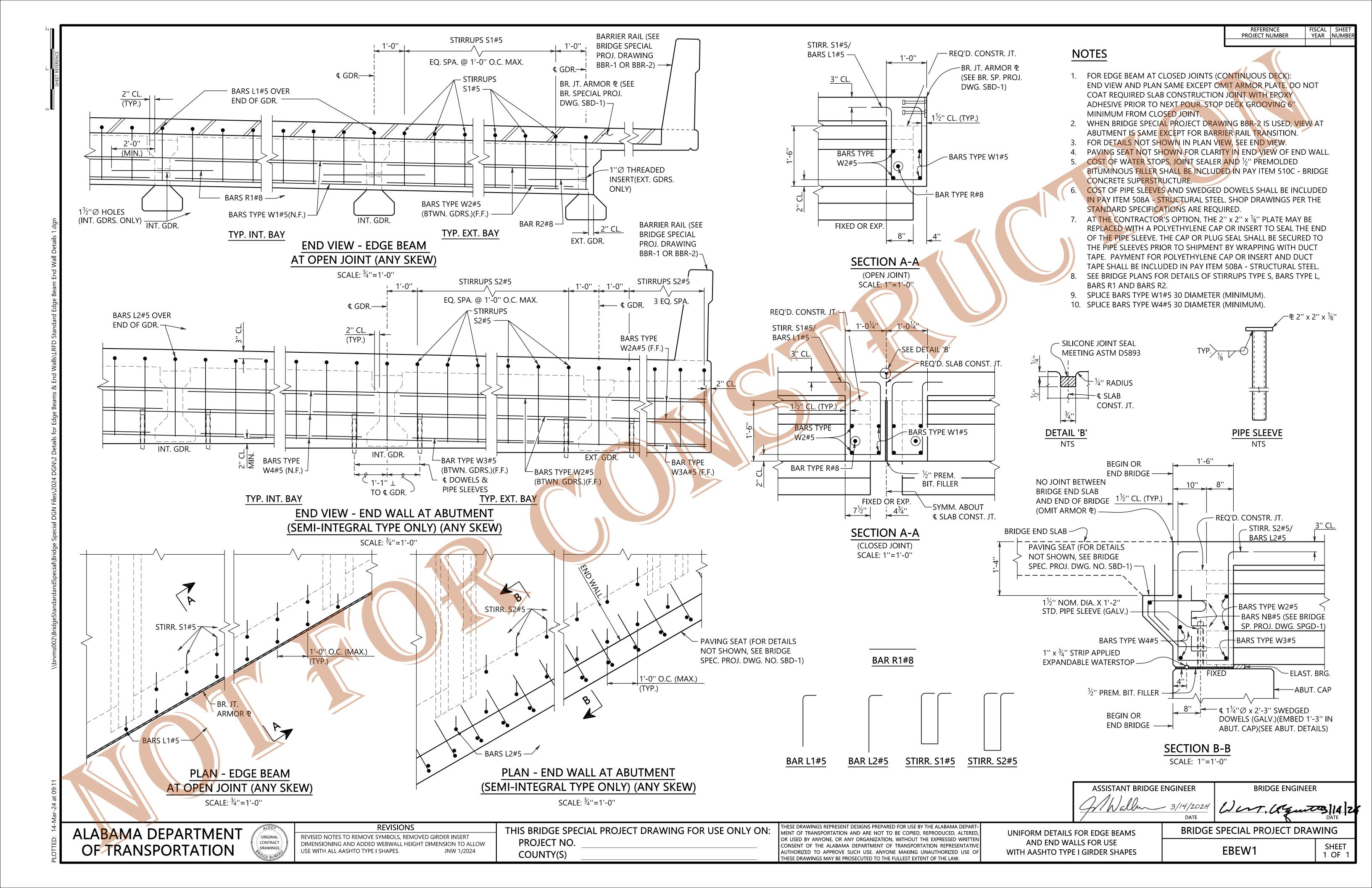
FURTHEST & EXPANSION BEARING.

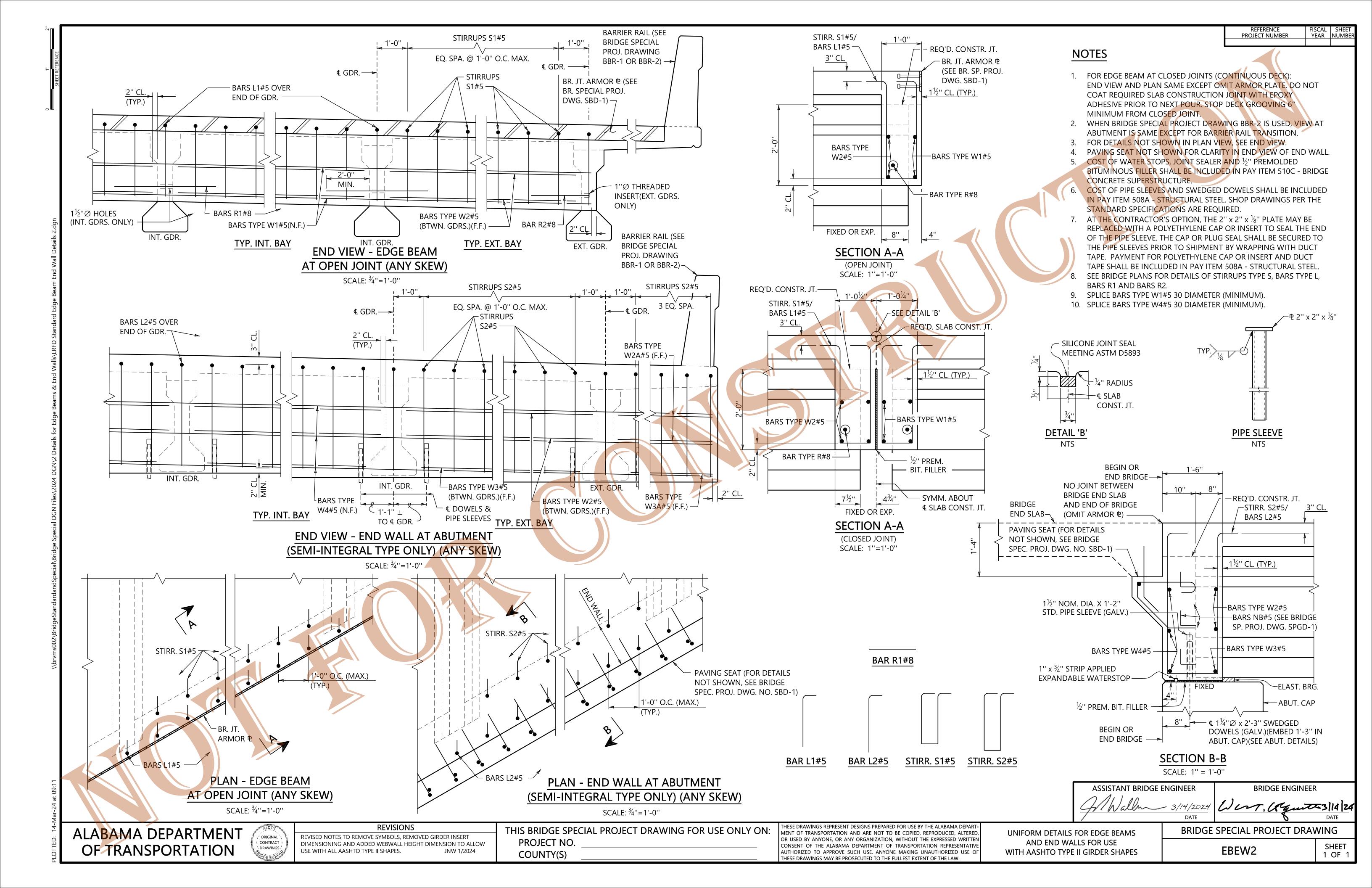
PLATE TO & SHIM PLATE.

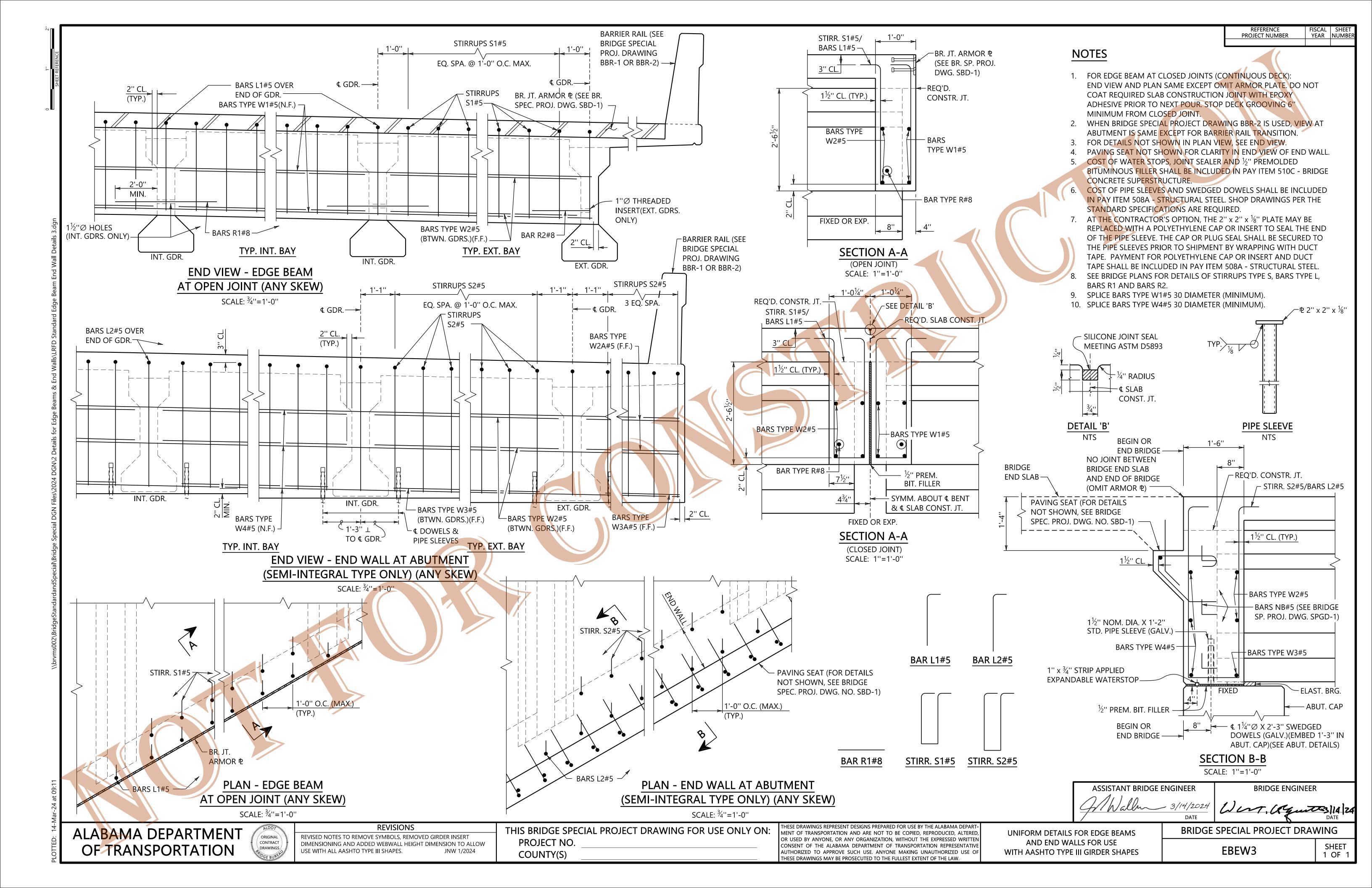
BRIDGE SPECIAL PROJECT DRAWING SHEET

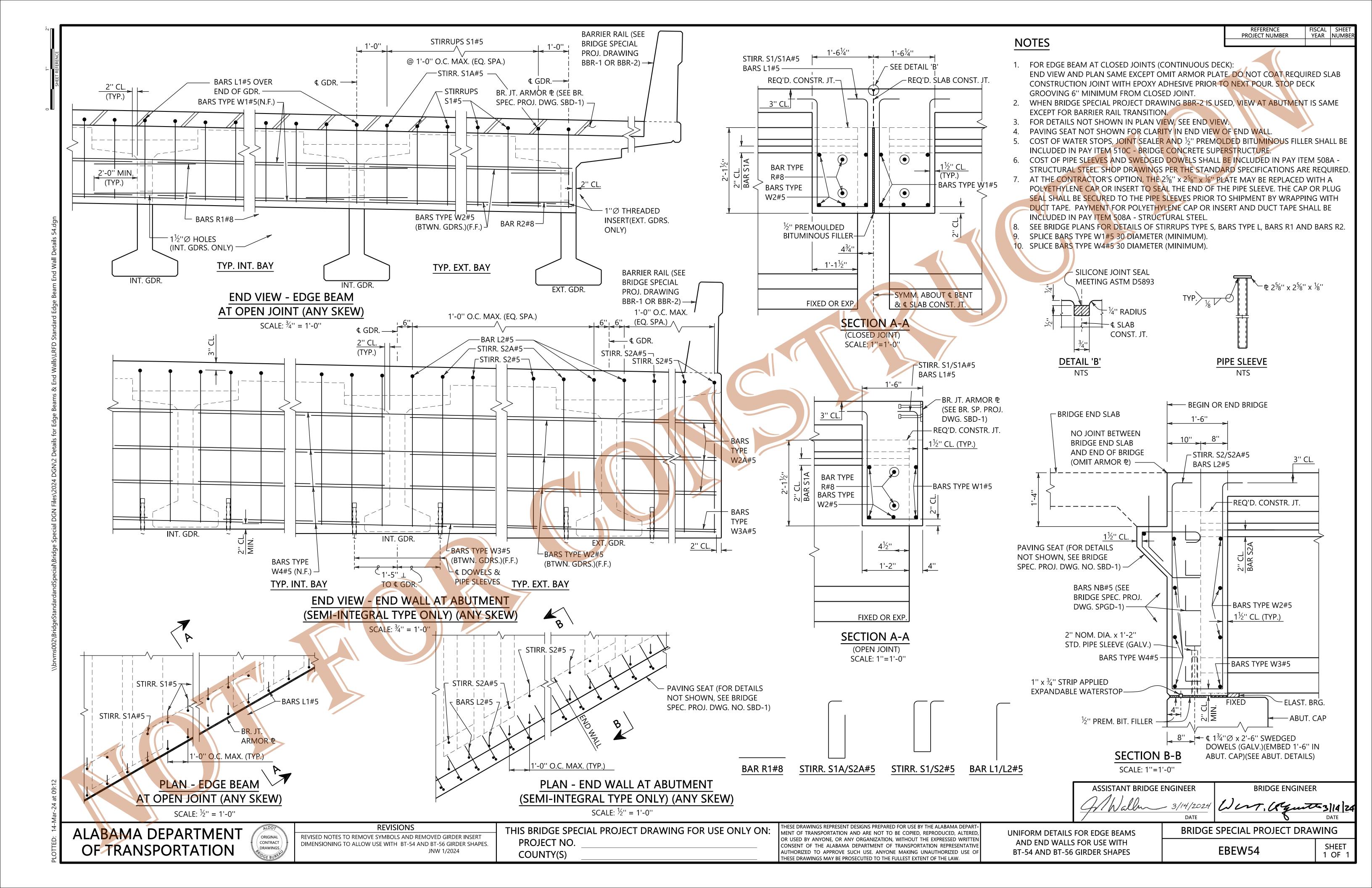
SPGD-1

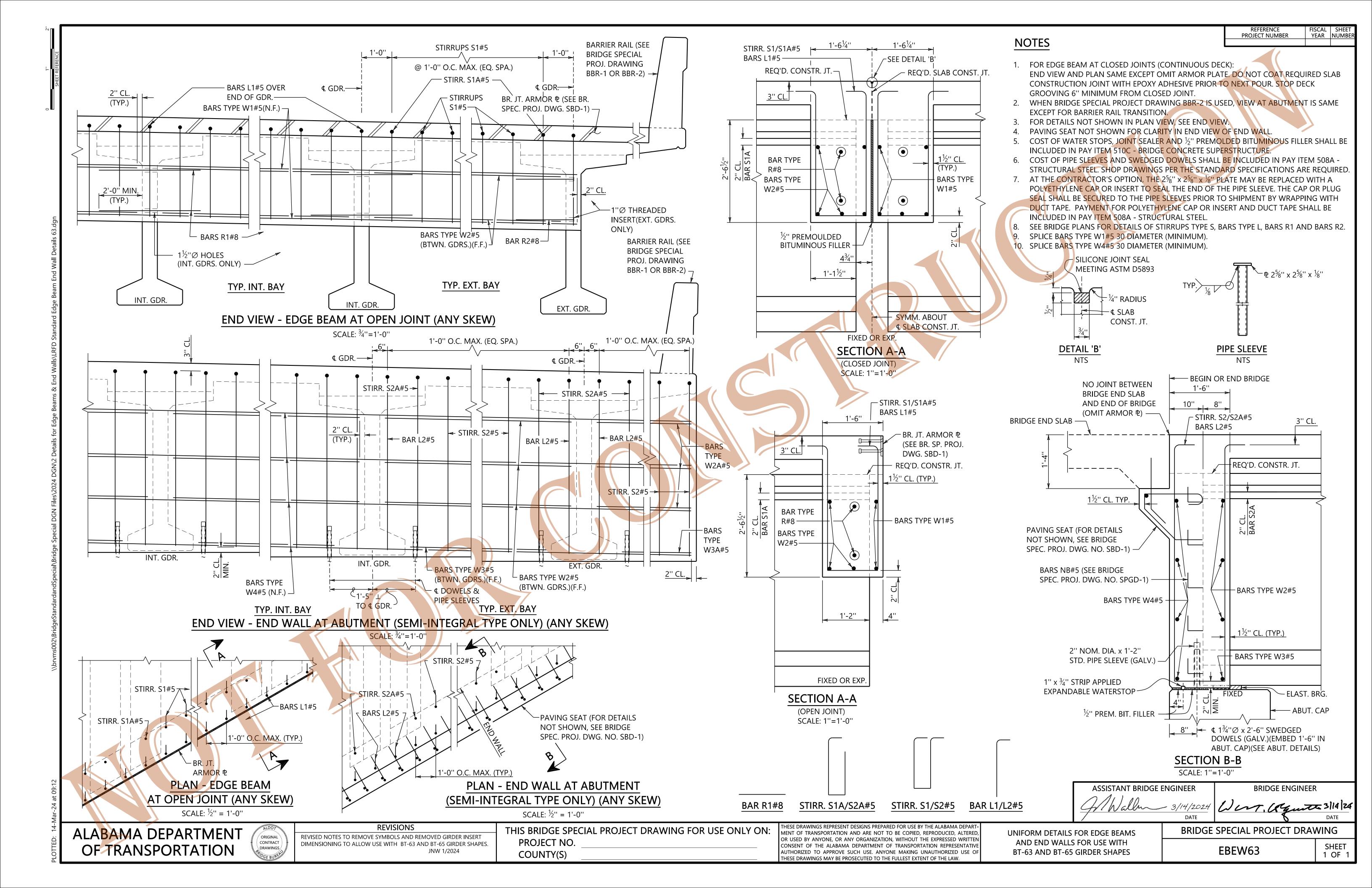
2 OF 2

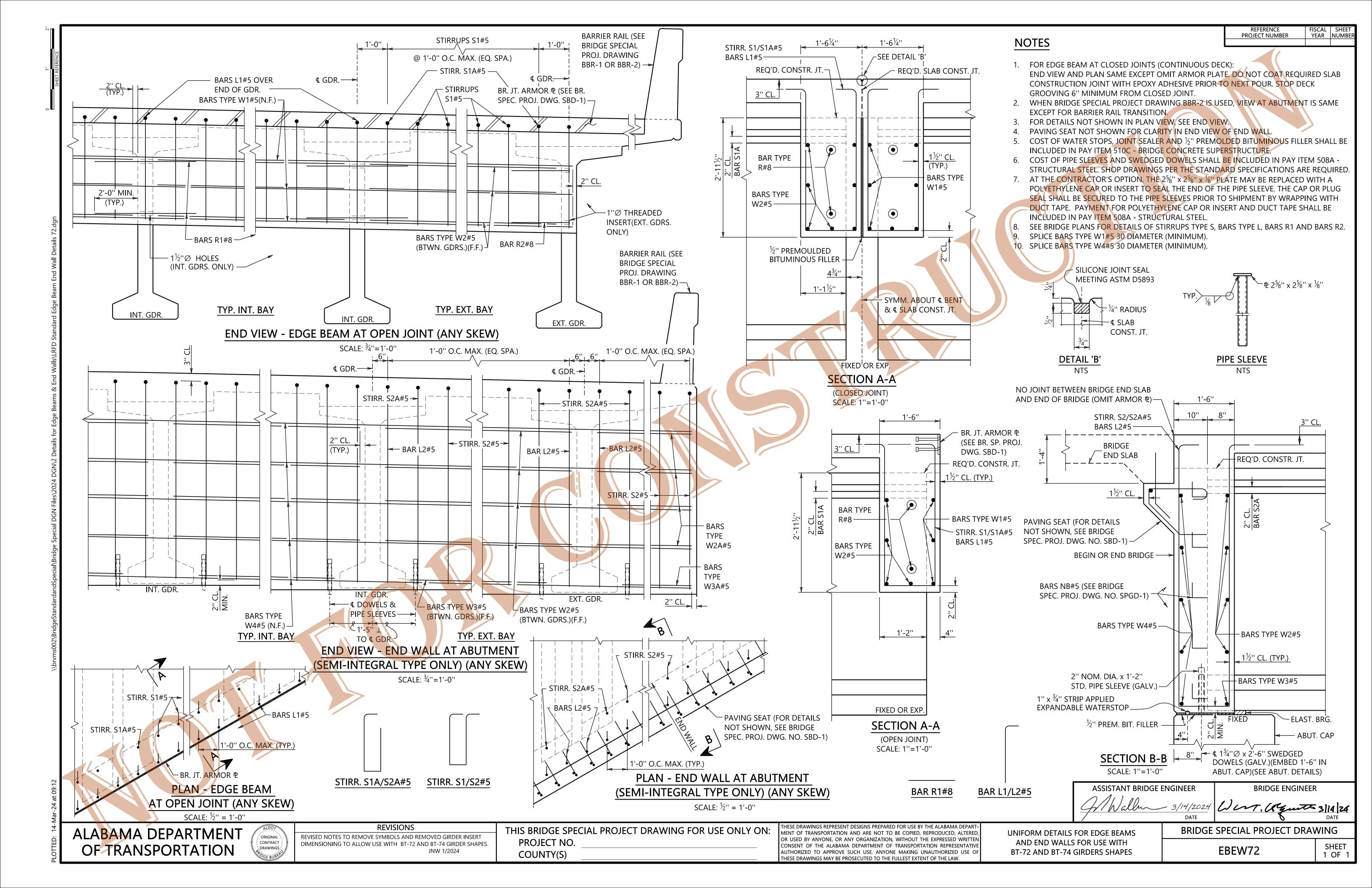


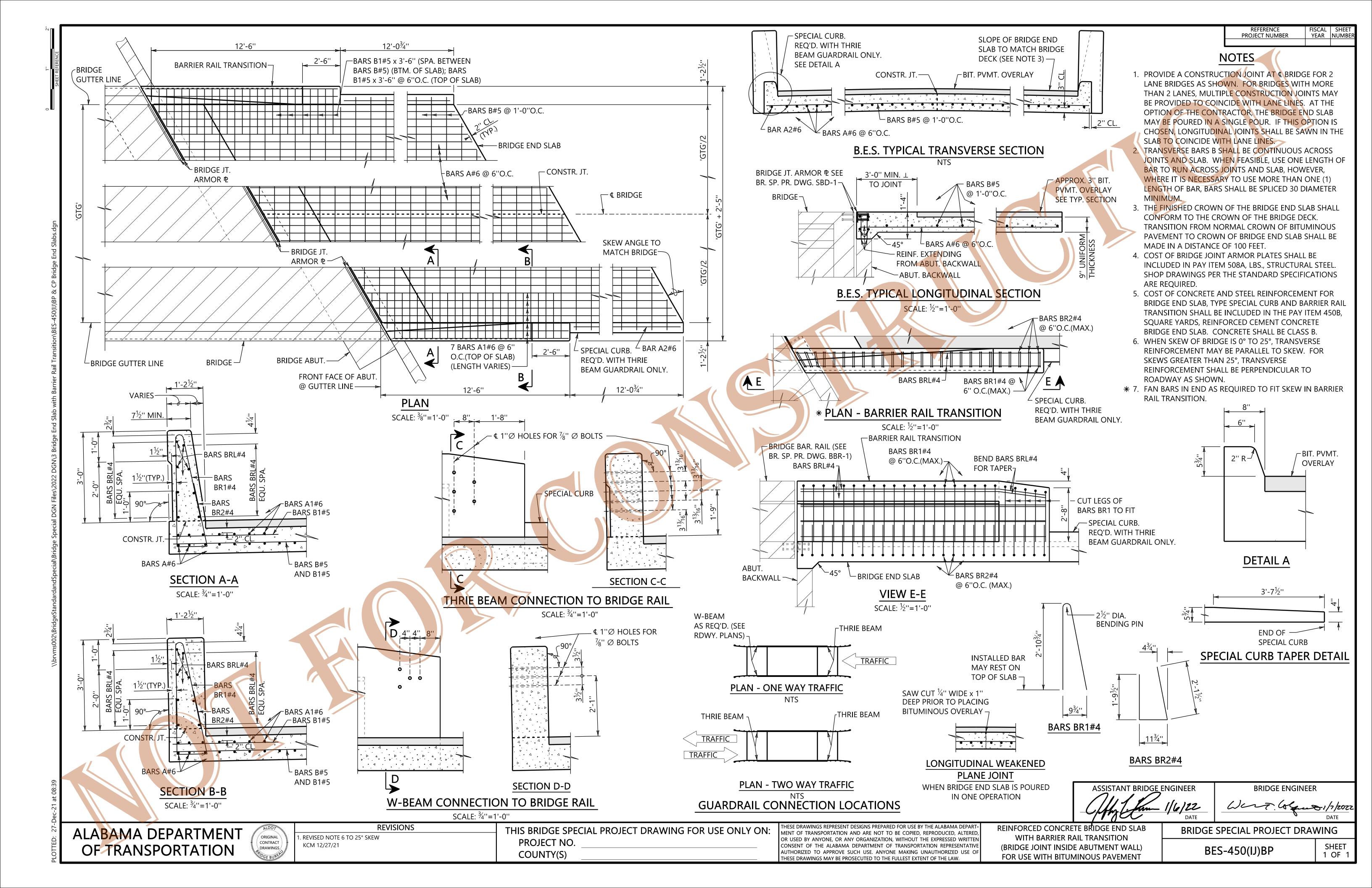


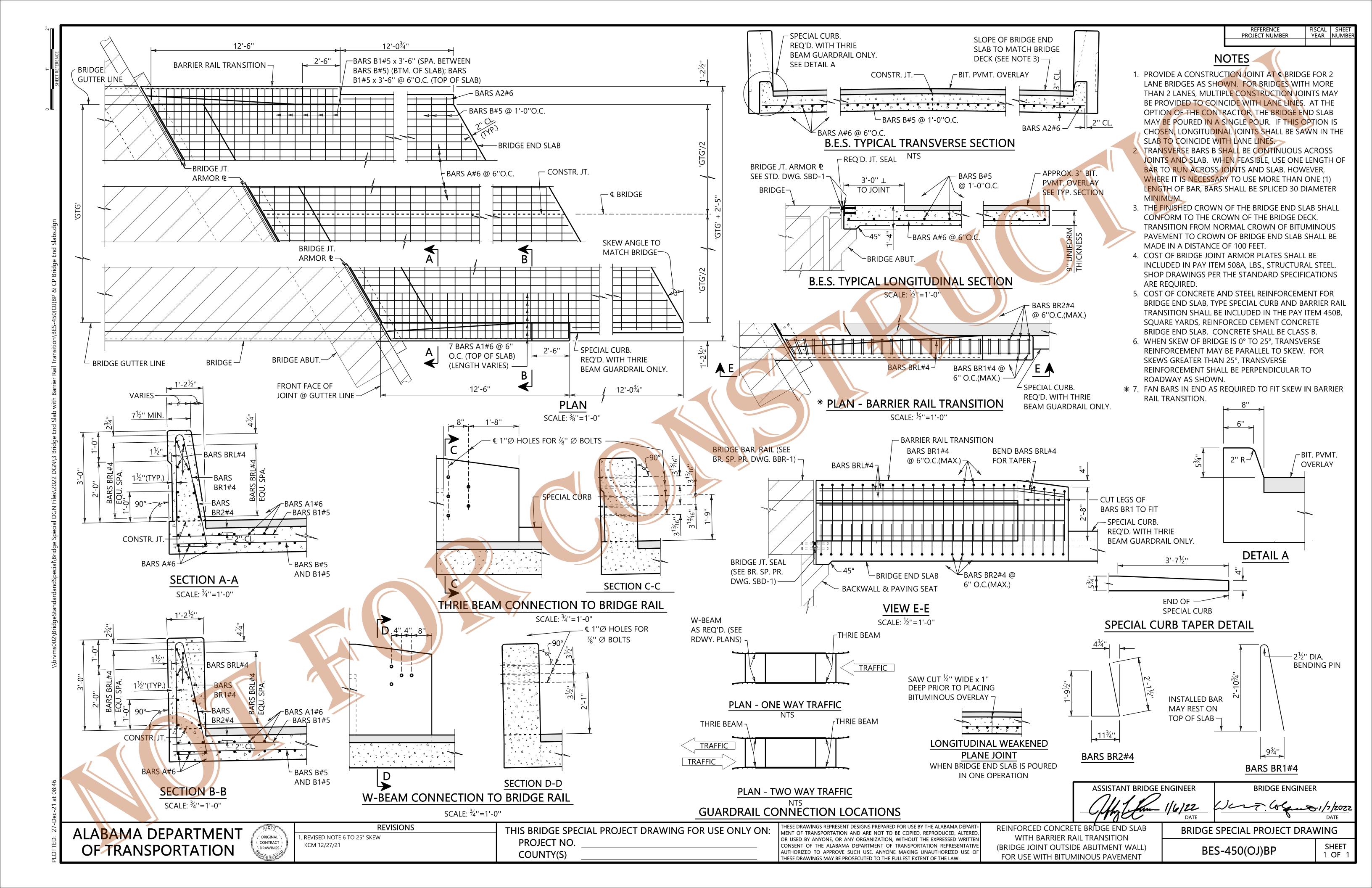


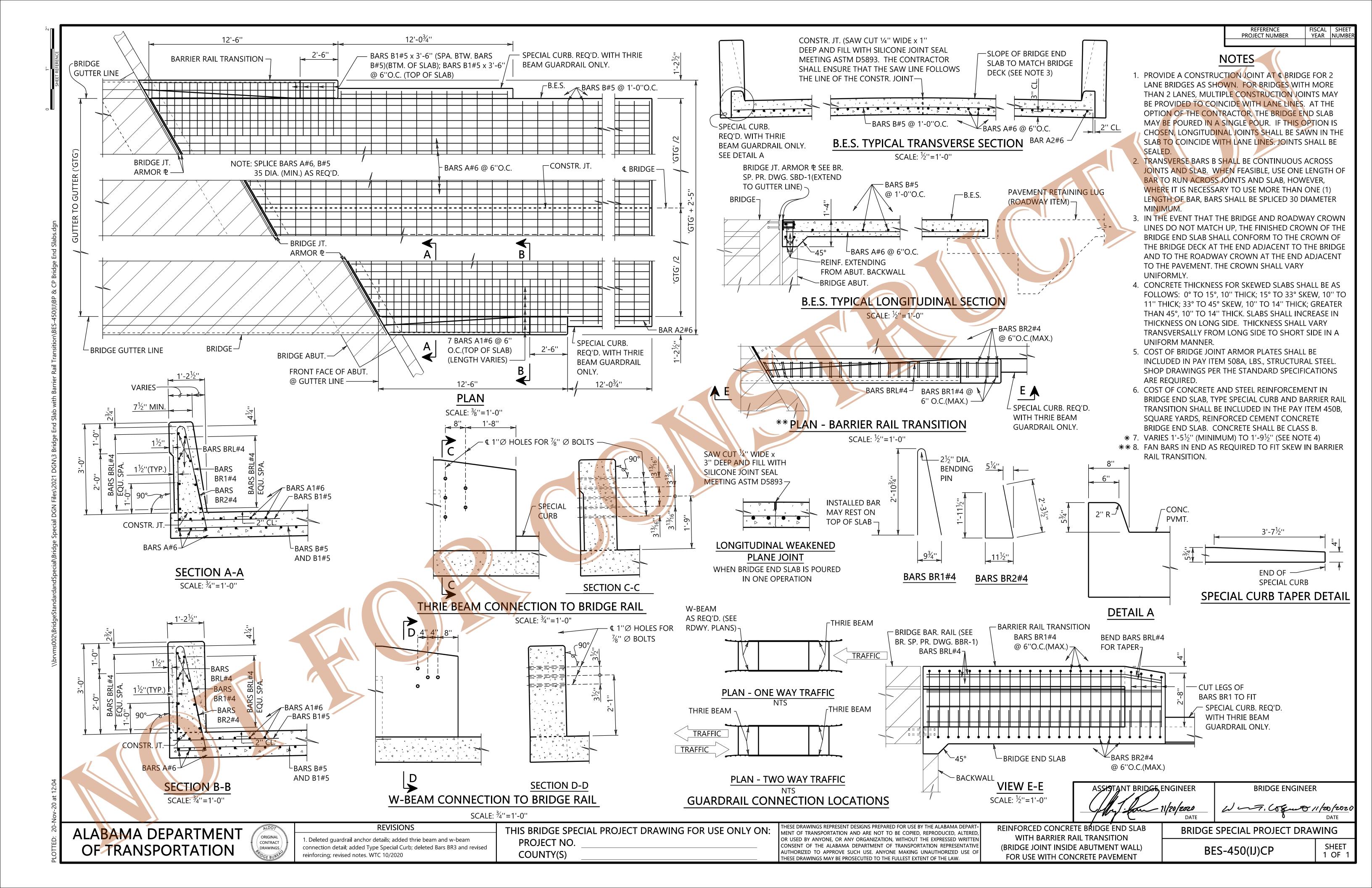


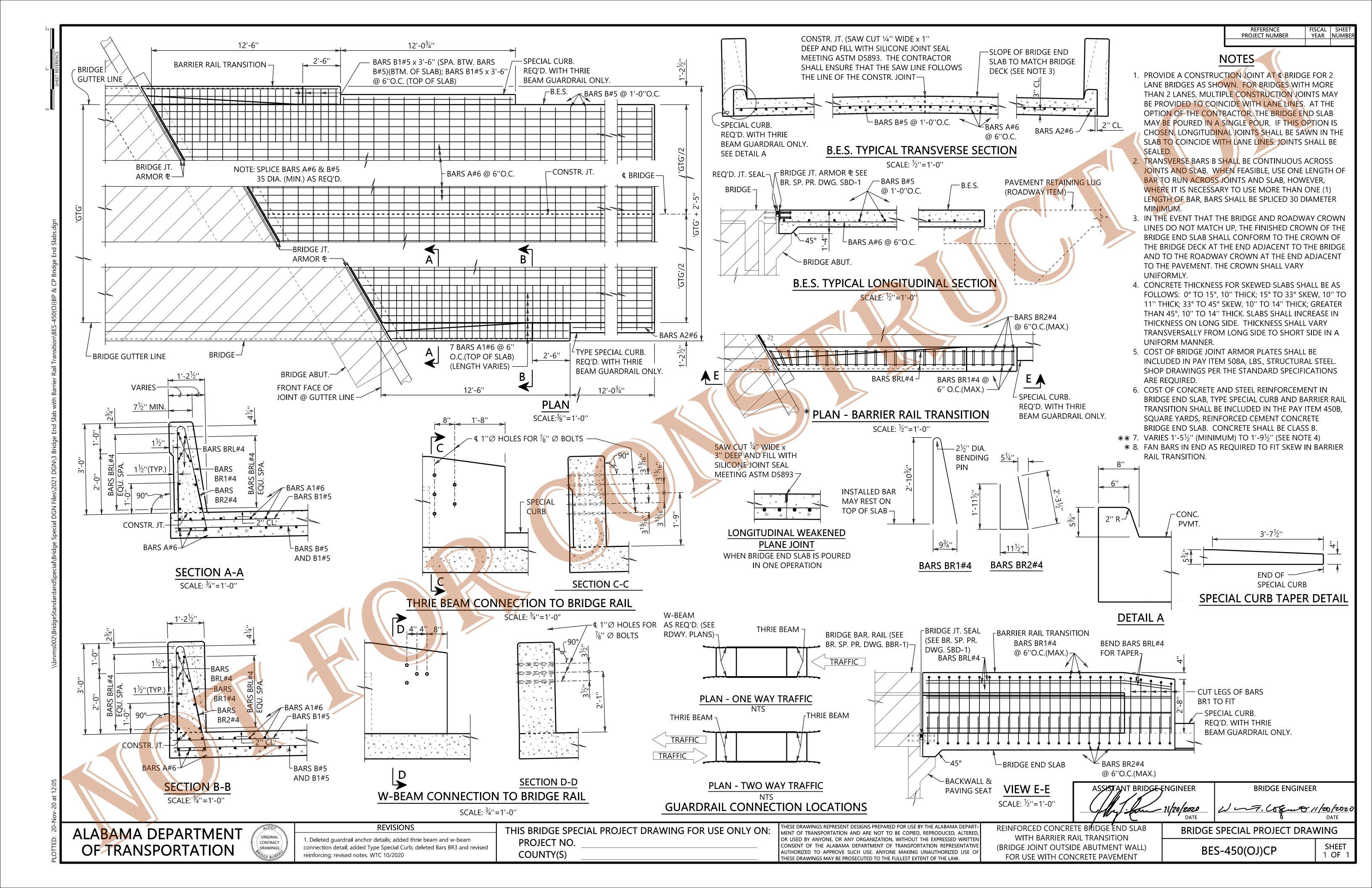


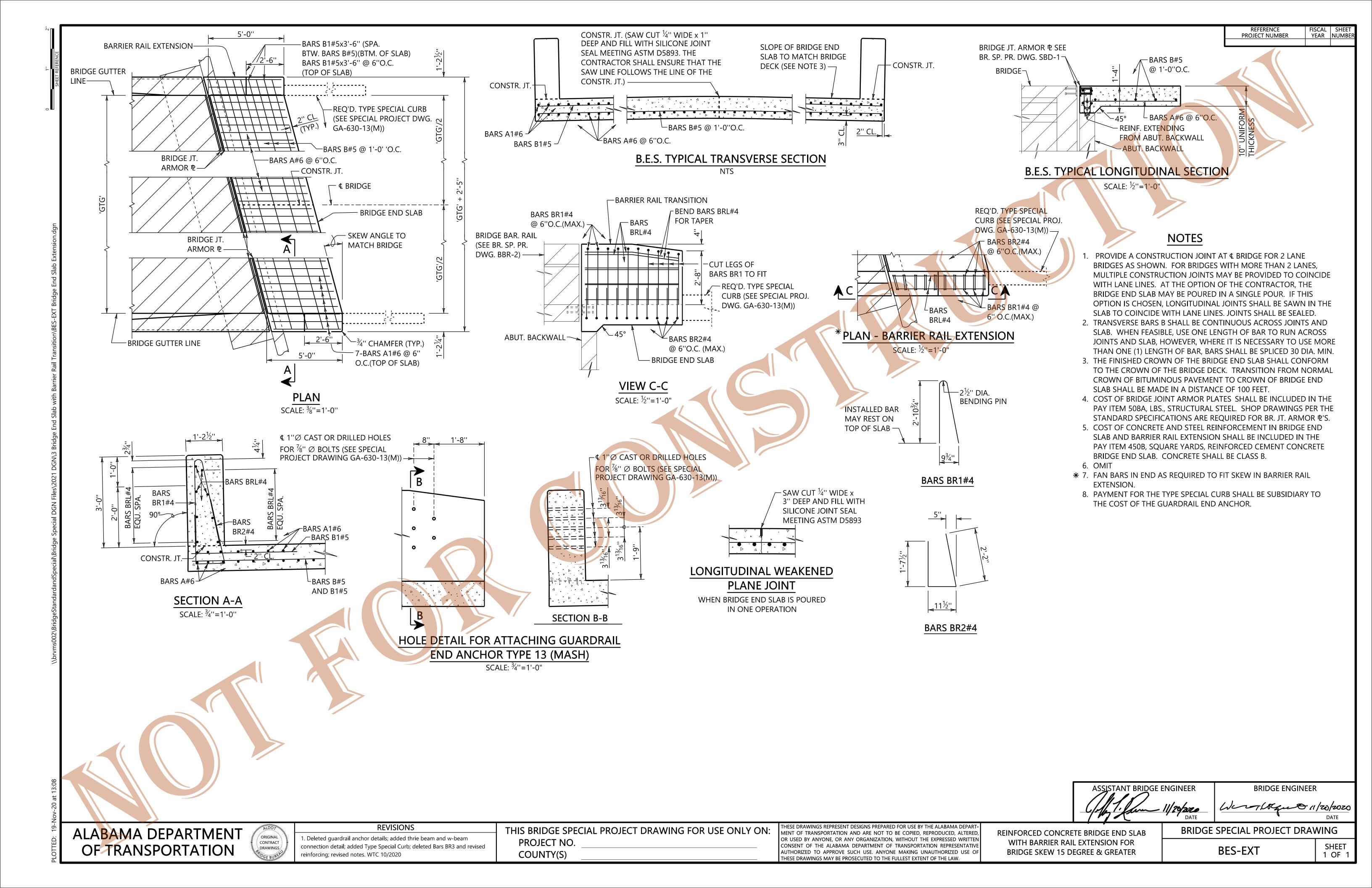


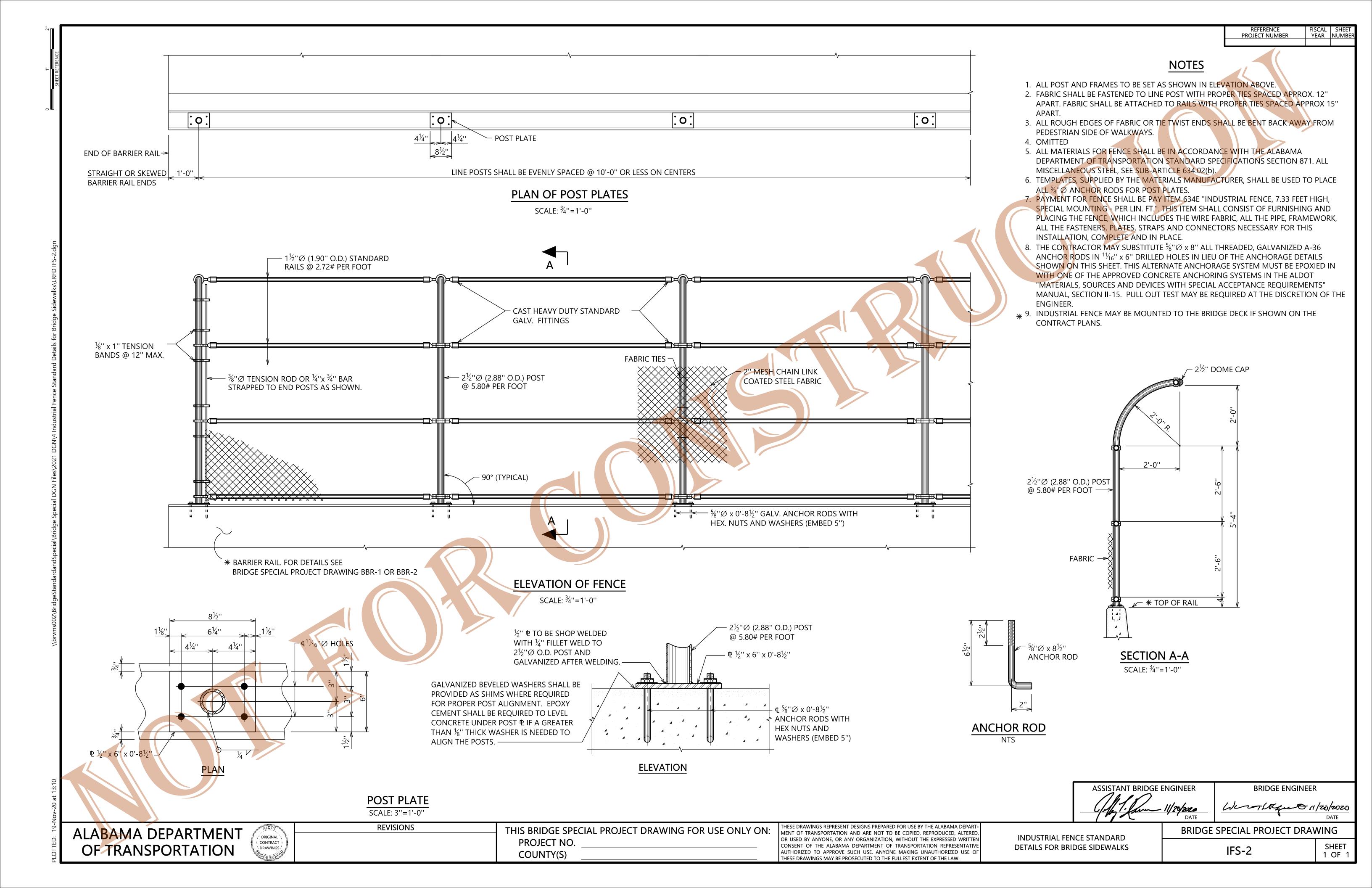


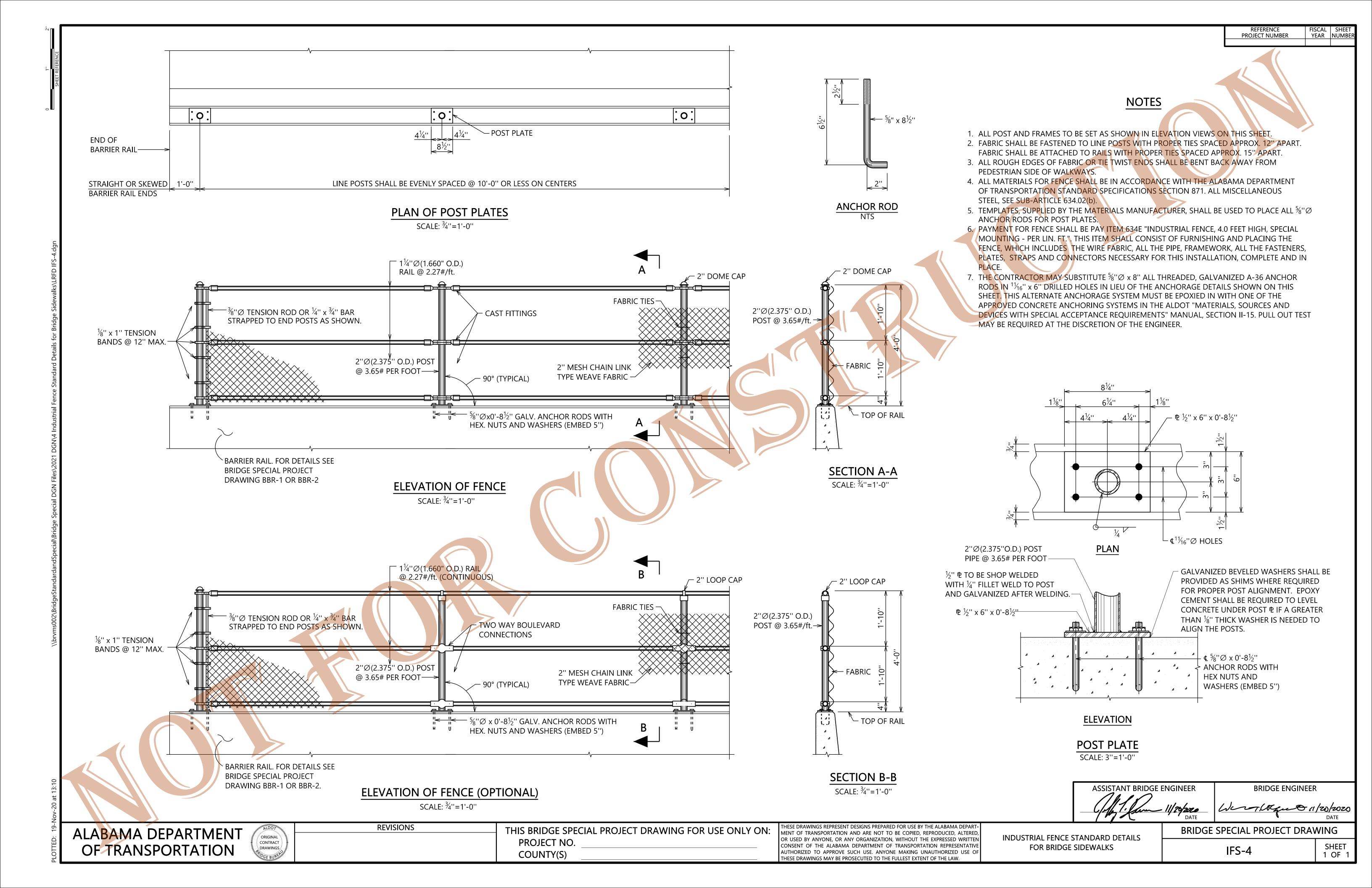


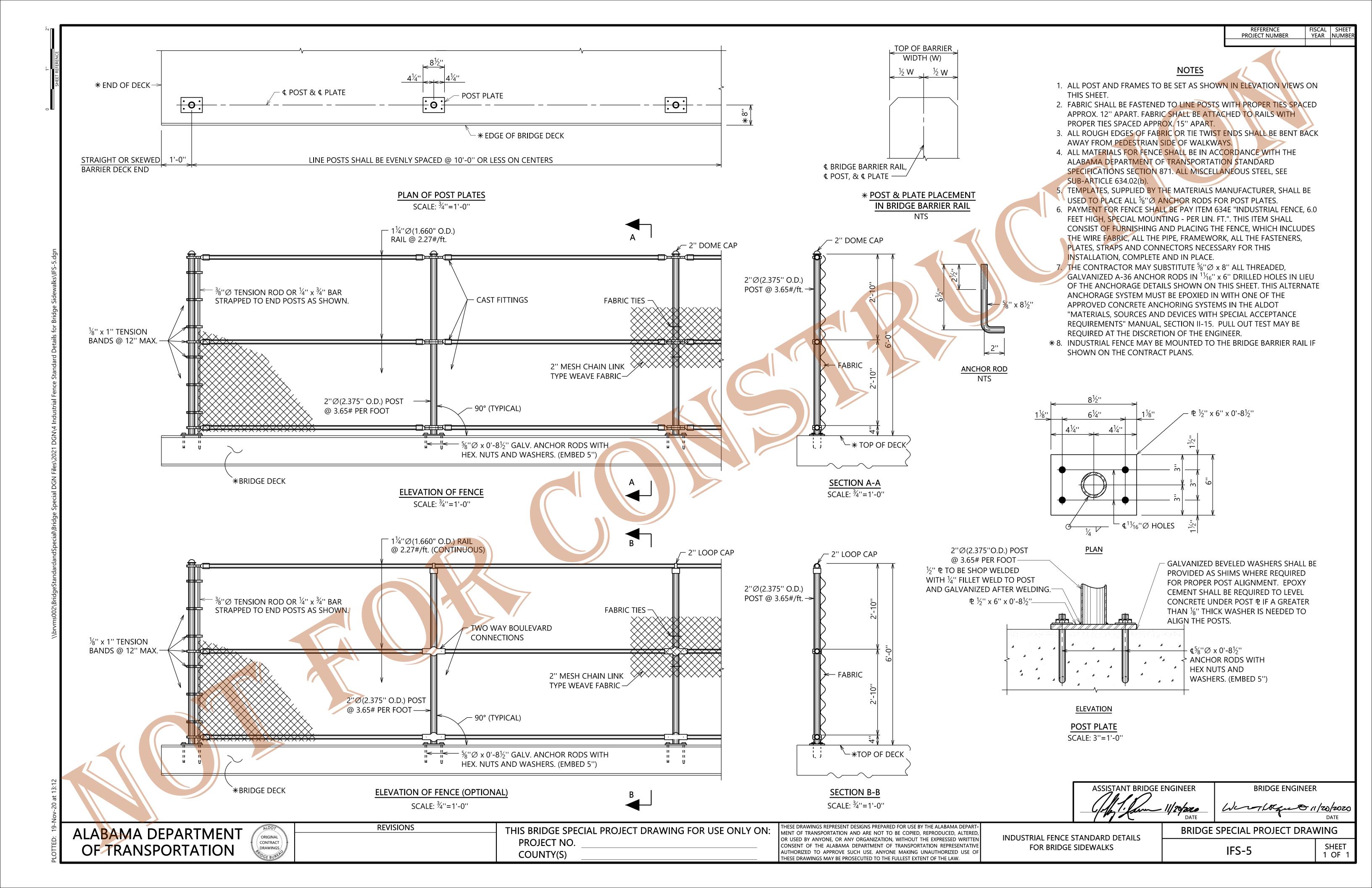


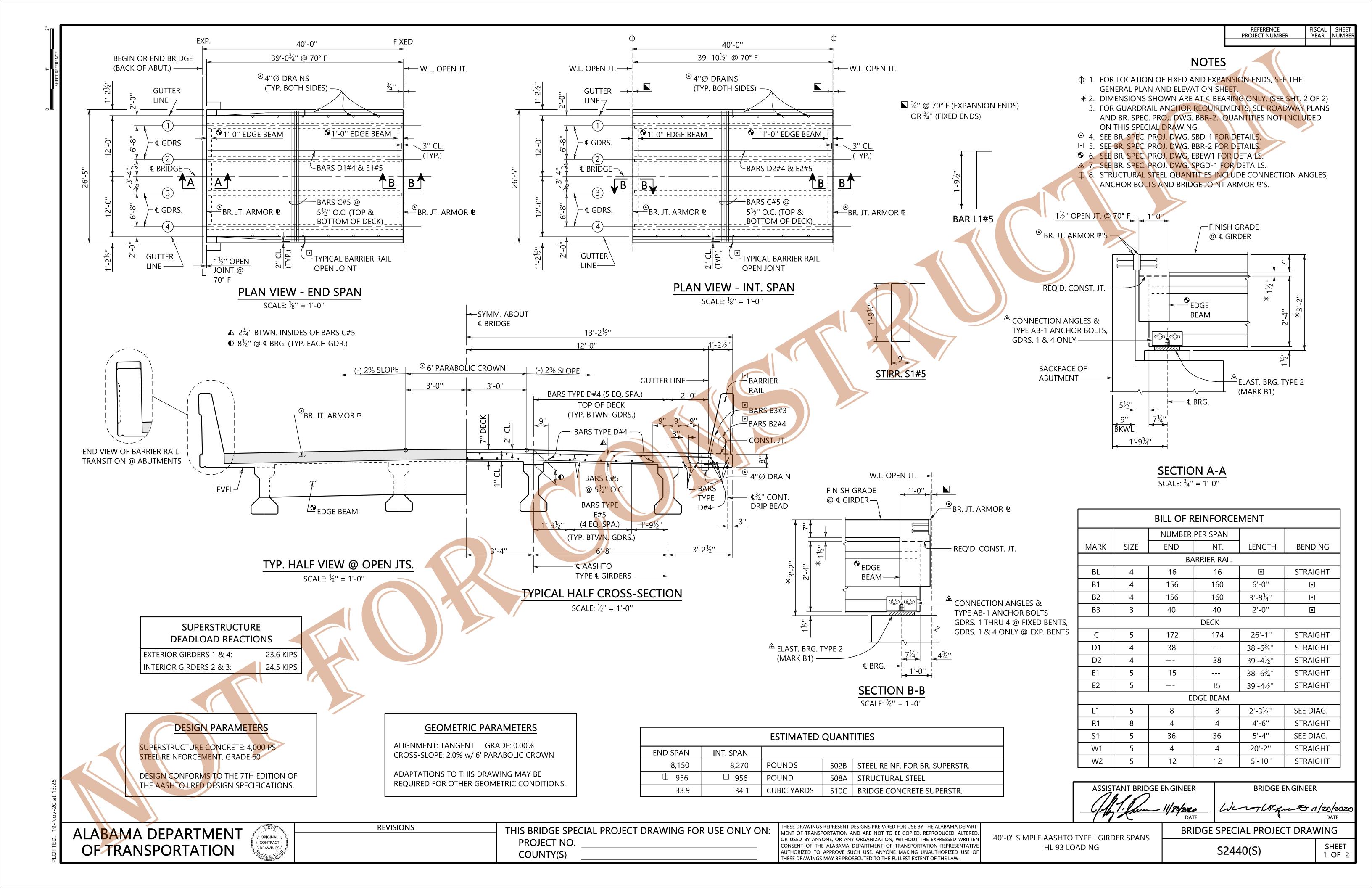






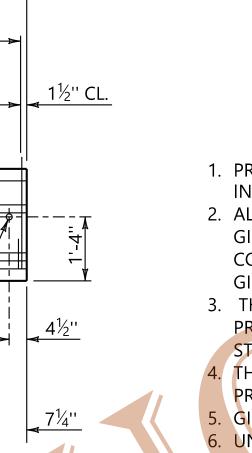








- 1. PRESTRESSING STRANDS SHALL BE $\frac{1}{2}$ " DIAMETER 270,000 PSI LOW RELAXTION WITH AN INITIAL TENSION OF 30,983 LBS./STRAND UNLESS OTHERWISE NOTED.
- 2. ALL STRANDS NOT TO BE ENCASED IN CONCRETE SHALL BE CUT FLUSH AT EACH END OF THE GIRDER. COAT GIRDER ENDS WHERE STRANDS ARE CUT WITH AN APPROVED EPOXY COATING. STRANDS TO BE ENCASED IN CONCRETE MAY EXTEND 2" FROM THE END OF THE
- 3. THE GIRDER CONCRETE SHALL HAVE A MINIMUM OF 4,500 PSI COMPRESSIVE STRENGTH PRIOR TO RECEIVING PRESTRESSING FORCE AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5,000 PSI.
- THREADED BARS R2 AND THREADED INSERTS SHALL BE INCLUDED IN THE BID ITEM 513B, PRETENSIONED-PRESTRESSED CONCRETE GIRDERS, TYPE I.
- GIRDER ENDS SHALL BE VERTICAL IN FINAL ERECTED POSITION.
- . UNLESS OTHERWISE SHOWN, STIRRUPS AND CONFINEMENT STEEL SHALL BE SECURELY TIED TO THE PRESTRESSING STRANDS TO PROVIDE A MINIMUM OF 1" CONCRETE COVER.
- CONNECTION ANGLES ARE REQUIRED ON BOTH FACES OF ALL GIRDERS AT THE FIXED END AND BOTH FACES OF THE EXTERIOR GIRDERS ONLY AT THE EXPANSION END. SEE BRIDGE SPECIAL PROJECT DWG. SPGD-1 FOR DETAILS.
- 3. THE ENGINEER WILL CONSIDER ALTERNATE GIRDER REINFORCING UTILIZING WELDED WIRE FABRIC IN LIEU REINFORCING FOR BARS B. THE EQUIVALENT AREA OF STEEL AND SPACING OF BARS SHALL BE MAINTAINED.



* TYPICAL GIRDER ELEVATION SCALE: ½'' = 1'-0''

▲ 2 STRAIGHT ½"Ø PRESTRESSED STRANDS WITH INITIAL TENSION OF 5,000 LBS. PER STRAND. STIRRUPS V#5 SHALL BE TIED IN PLACE TO THESE STRANDS.

6 @ 8''

=4'-0''

4 @ 1'-0''

=4'-0''

- BARS B#3 SPA. W/STIRRUPS V#5

5''

1'-4''

AASHTO TYPE I GIRDER

SCALE: 1'' = 1'-0''

[−]¾'' CHAMFER

■ MID-POINT OF GIRDER

DETAIL OF BUILD-UP BETWEEN BOTTOM

OF DECK AND TOP OF GDR. (ALONG & GDR.)

NO SCALE

OP OF GIRDER PRIOR

TOP OF GIRDER AFTER

/ 7" DECK

BUILD-UP (VARIES)

1½" @ **£** BRG. ONLY

DECK PLACEMENT

TO DECK PLACEMENT

(TYP.)

(10 PAIRS) TYP. EACH END

SPACING FOR

STIRRUPS V#51

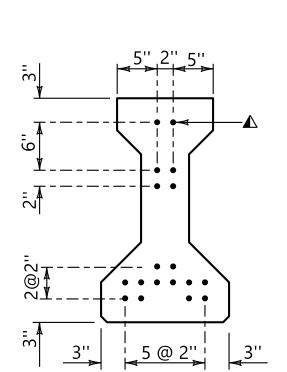
1½" CL.

71/4"

9 @ 4''

=3'-0''

└ BRG.



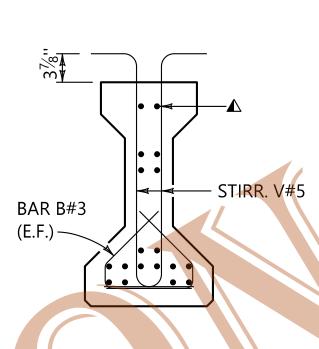
5 @ 1'-6''

=7'-6''

 $-2 - \frac{1}{2}$ "Ø TIE STRANDS



SCALE: 1'' = 1'-0''

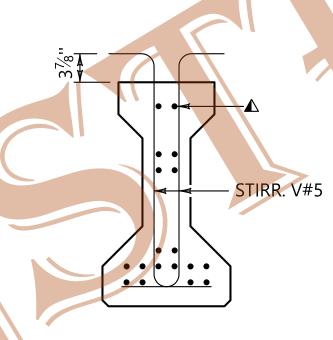


- 16 - $\frac{1}{2}$ "Ø STRAIGHT STRANDS

5 @ 1'-6''

=7'-6''





6 @ 8''

=4'-0''

9 @ 4''

=3'-0''

⊈ BRG.—>

 \blacksquare 1½"Ø HOLE (GDRS. 2 & 3) OR 1"Ø THREADED

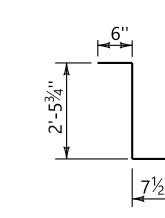
INSERT (INSIDE FACE ONLY, GDRS. 1 & 4)

* DIMENSIONS SHOWN ARE ALONG & GIRDER

4 @ 1'-0''

=4'-0''

SECTION ALONG GIRDER

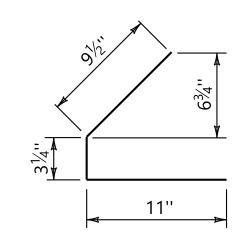


STIRRUPS V#5

	END SPANS	INT. SPANS
L	38'-4 ³ ⁄ ₄ ''	39'-2 ¹ ⁄2''
В	37'-2 ¹ ⁄ ₄ ''	38'-0''
Α	1'-1¾''	2 SPA. @ 11 ³ / ₄ ''
•		

NC THREAD _ 3" 2'-3''

BARS R2#8



BARS B#3

THEORETICAL CAMBER (UPWARD DEFLECTION) SHOWN. ACTUAL CAMBER OF GIRDER MAY VARY AND SHOULD BE DETERMINED BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS AND SETTING FORMS.

△ ADJUSTMENT TO BUILD-UP MAY BE REQUIRED IF USED FOR BRIDGES IN EXTREME CREST OR SAG VERTICAL CURVE GRADES.

ALABAMA DEPARTMENT **OF TRANSPORTATION**

REVISIONS PROJECT NO. CONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE OI HESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

HL 93 LOADING

BRIDGE SPECIAL PROJECT DRAWING

S2440(S)

SHEET 2 OF 2

TRUE GRADE AFTER

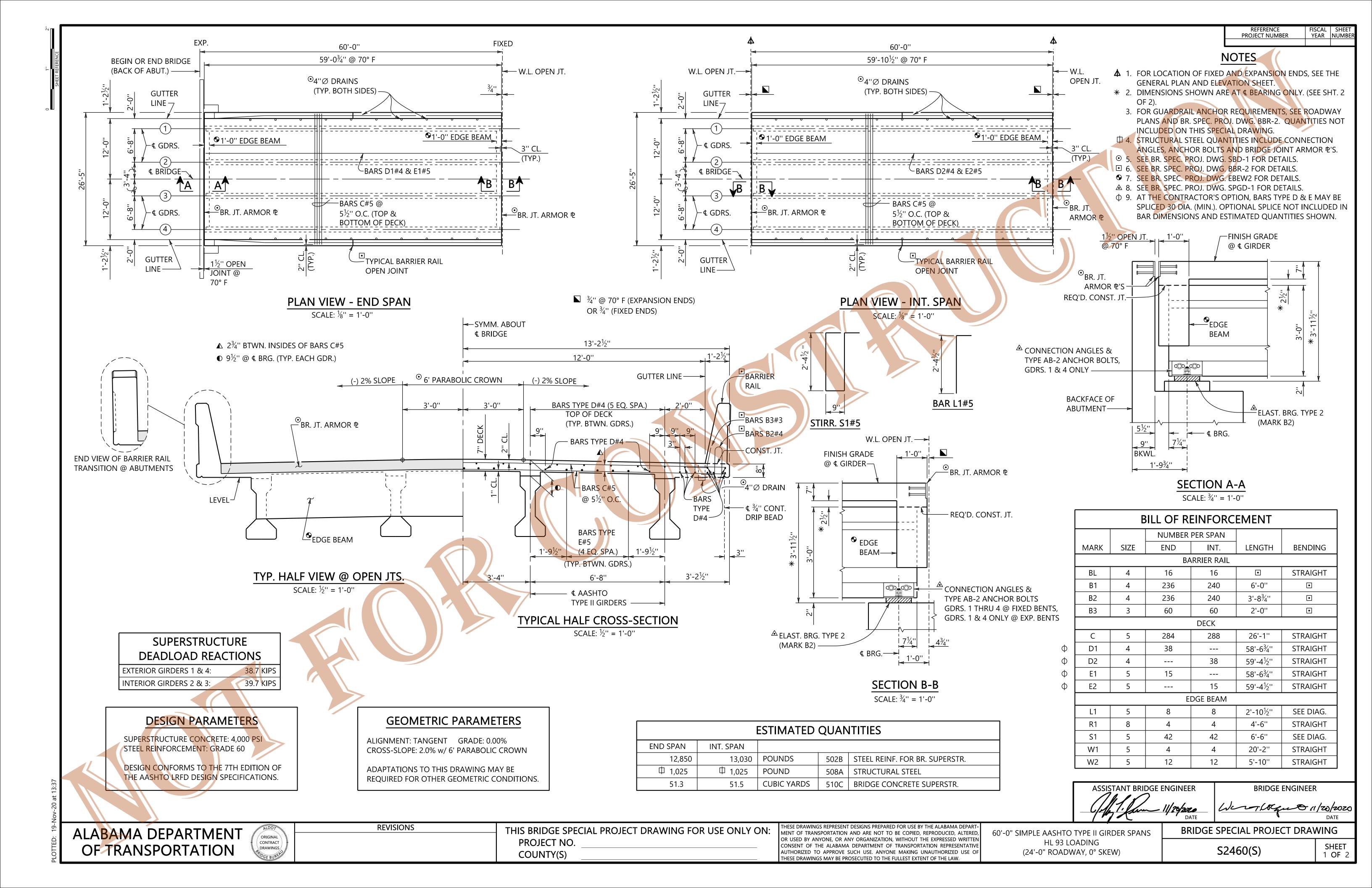
OF GIRDER—

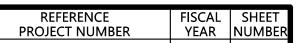
VARIES 8½" @ ⊄ BRG.

DEAD LOAD DEFLECTION

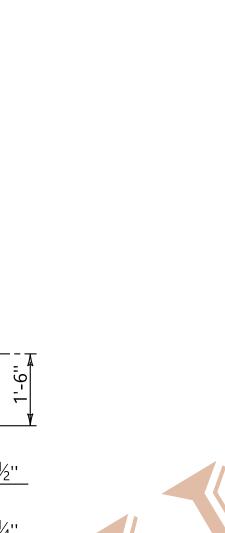
THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: COUNTY(S)

40'-0" SIMPLE AASHTO TYPE I GIRDER SPANS





- 1. PRESTRESSING STRANDS SHALL BE $\frac{1}{2}$ " DIAMETER 270,000 PSI LOW RELAXTION WITH AN INITIAL TENSION OF 30,983 LBS./STRAND UNLESS OTHERWISE NOTED.
- 2. ALL STRANDS NOT TO BE ENCASED IN CONCRETE SHALL BE CUT FLUSH AT EACH END OF THE GIRDER. COAT GIRDER ENDS WHERE STRANDS ARE CUT WITH AN APPROVED EPOXY COATING. STRANDS TO BE ENCASED IN CONCRETE MAY EXTEND 2" FROM THE END OF THE GIRDER.
- 3. THE GIRDER CONCRETE SHALL HAVE A MINIMUM OF 5,500 PSI COMPRESSIVE STRENGTH PRIOR TO RECEIVING PRESTRESSING FORCE AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6,000 PSI.
- 4. THREADED BARS R2 AND THREADED INSERTS SHALL BE INCLUDED IN THE BID ITEM 513B, PRETENSIONED-PRESTRESSED CONCRETE GIRDERS, TYPE II.
- 5. GIRDER ENDS SHALL BE VERTICAL IN FINAL ERECTED POSITION.
- 6. STRANDS SHOWN THUS SHALL REMAIN UNBONDED BY USING PLASTIC SHEATHES AROUND CABLES FOR A DISTANCE OF 6'-0" FROM THE ENDS OF THE GIRDER. UNLESS OTHERWISE SHOWN, STIRRUPS AND CONFINEMENT STEEL SHALL BE SECURELY TIED TO THE PRESTRESSING STRANDS TO PROVIDE A MINIMUM OF 1" CONCRETE
- CONNECTION ANGLES ARE REQUIRED ON BOTH FACES OF ALL GIRDERS AT THE FIXED END AND BOTH FACES OF THE EXTERIOR GIRDERS ONLY AT THE EXPANSION END. SEE BRIDGE SPECIAL PROJECT DWG. SPGD-1 FOR DETAILS.
- THE ENGINEER WILL CONSIDER ALTERNATE GIRDER REINFORCING UTILIZING WELDED WIRE FABRIC IN LIEU OF TIED REINFORCING FOR BARS B. THE EQUIVALENT AREA OF STEEL AND SPACING OF BARS SHALL BE MAINTAINED.



* TYPICAL GIRDER ELEVATION SCALE: ½'' = 1'-0''

▲ 2 STRAIGHT ½"Ø PRESTRESSED STRANDS WITH INITIAL TENSION OF 5,000 LBS. PER STRAND. STIRRUPS TYPE V SHALL BE TIED IN PLACE TO THESE STRANDS.

1'-0''

1'-6''

AASHTO TYPE II GIRDER

SCALE: 1'' = 1'-0''

┌─ 3 @ 4'' = 1'-0'' (V#6)

9 @ 8'' = 6'-0''

-BARS B#3 SPA. W/STIRRUPS TYPE V

 $-\frac{3}{4}$ " CHAMFER

(10 PAIRS) TYP. EACH END

- V1#5 -

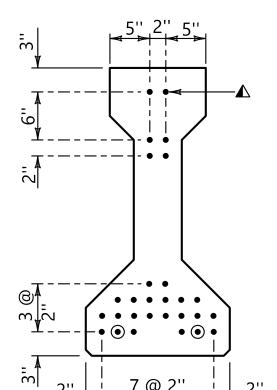
5 @ 4''

SPACING FOR

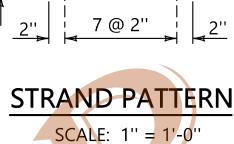
STIRR. TYPE V |

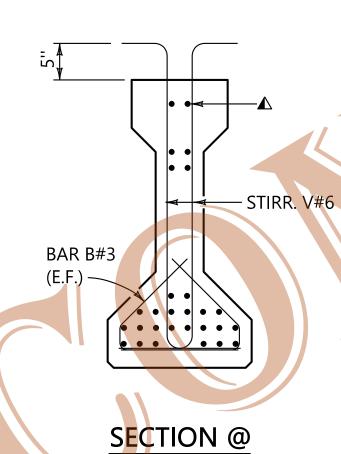
1½'' CL.

<u>7¹/₄''</u>▶



6 @ 1'-0'' = 6'-0''



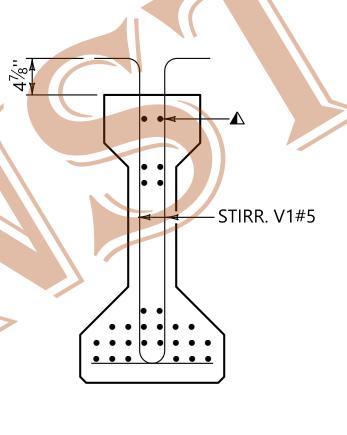


9 @ 1'-6" = 13'-6"

 $-2 - \frac{1}{2}$ "Ø TIE STRANDS

26 - ½"Ø STRAIGHT STRANDS





OMIT CENTER

STIRRUP IN

END SPANS —

SYMM. ABOUT

⊈ BRG.—>

■ $1\frac{1}{2}$ "Ø HOLE (GDRS. 2 & 3) OR 1"Ø THREADED

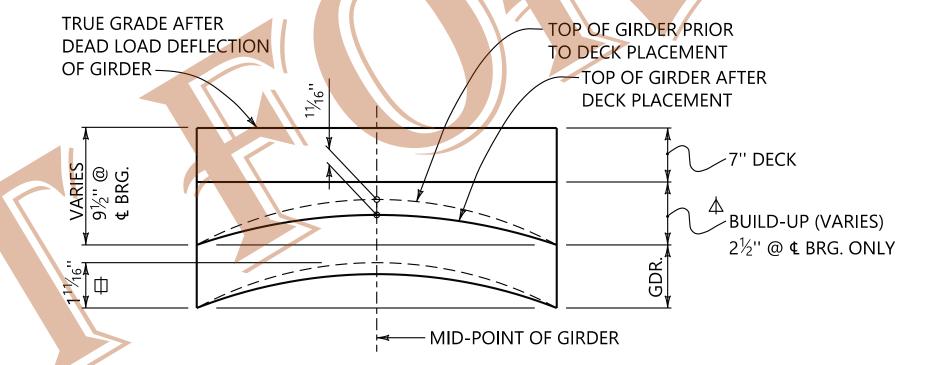
INSERT (INSIDE FACE ONLY, GDRS. 1 & 4)

***** DIMENSIONS SHOWN ARE ALONG **€** GIRDER

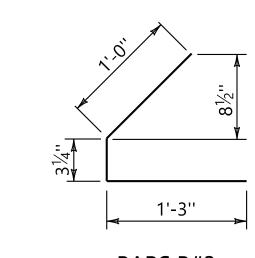
MID-POINT

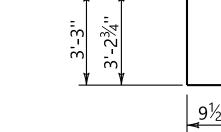
SECTION ALONG GIRDER SCALE: 1" = 1'-0"

	END SPANS	INT. SPANS
L	58'-4 ³ ⁄ ₄ ''	59'-2½''
В	57'-21/4''	58'-0''
A	1'-1¾''	2 SPA. @ 11¾''



- ☐ THEORETICAL CAMBER (UPWARD DEFLECTION) SHOWN. ACTUAL CAMBER OF GIRDER MAY VARY AND SHOULD BE DETERMINED BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS AND SETTING FORMS.
- △ ADJUSTMENT TO BUILD-UP MAY BE REQUIRED IF USED FOR BRIDGES IN EXTREME CREST OR SAG VERTICAL CURVE GRADES.





NC THREAD 2'-3''

BARS B#3

STIRRUPS TYPE V

BARS R2#8

NO SCALE

DETAIL OF BUILD-UP BETWEEN BOTTOM

OF DECK AND TOP OF GDR. (ALONG & GDR.)

ALABAMA DEPARTMENT **OF TRANSPORTATION**

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60'-0" SIMPLE AASHTO TYPE II GIRDER SPANS HL 93 LOADING (24'-0" ROADWAY, 0° SKEW)

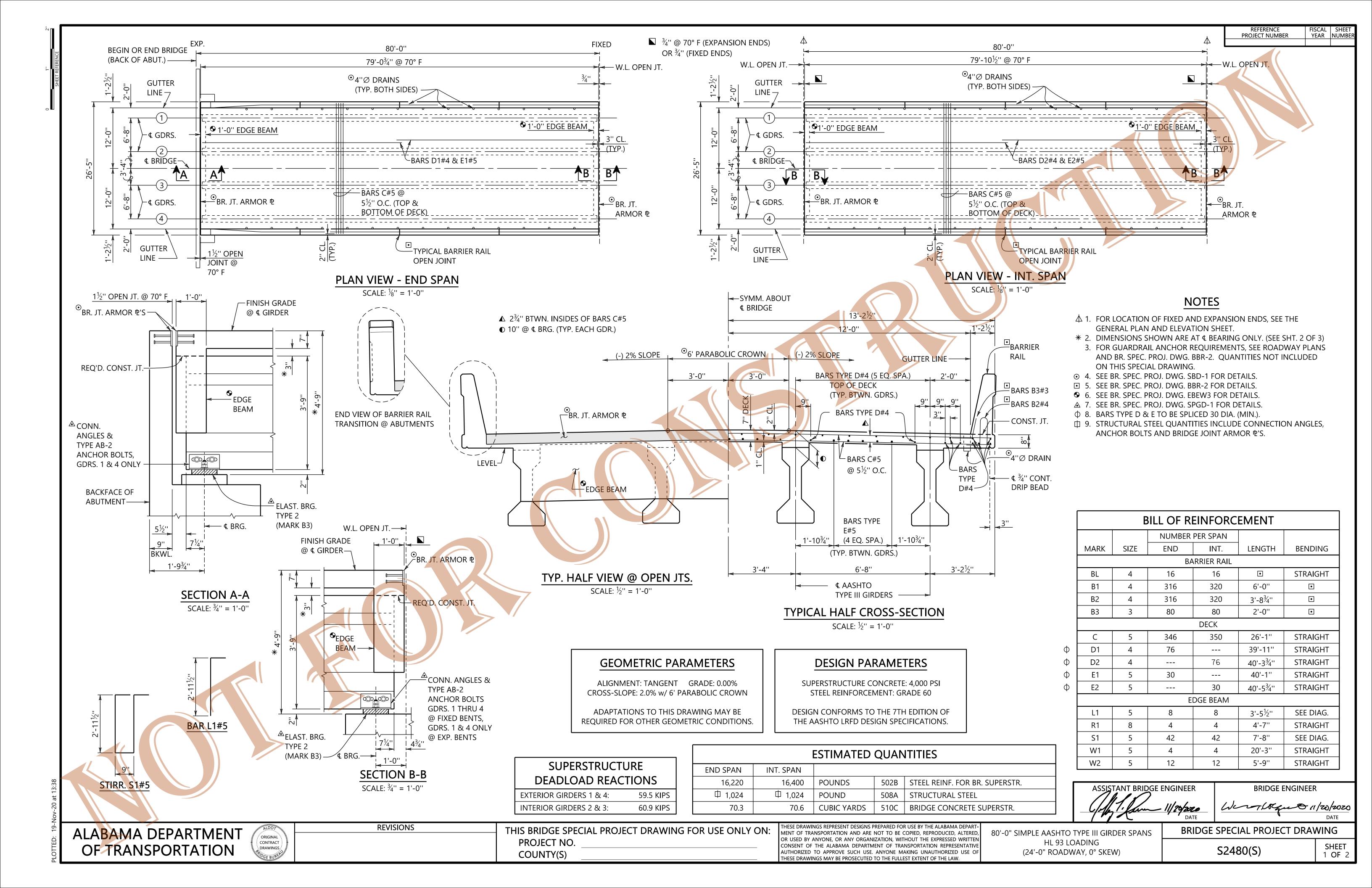
BRIDGE SPECIAL PROJECT DRAWING

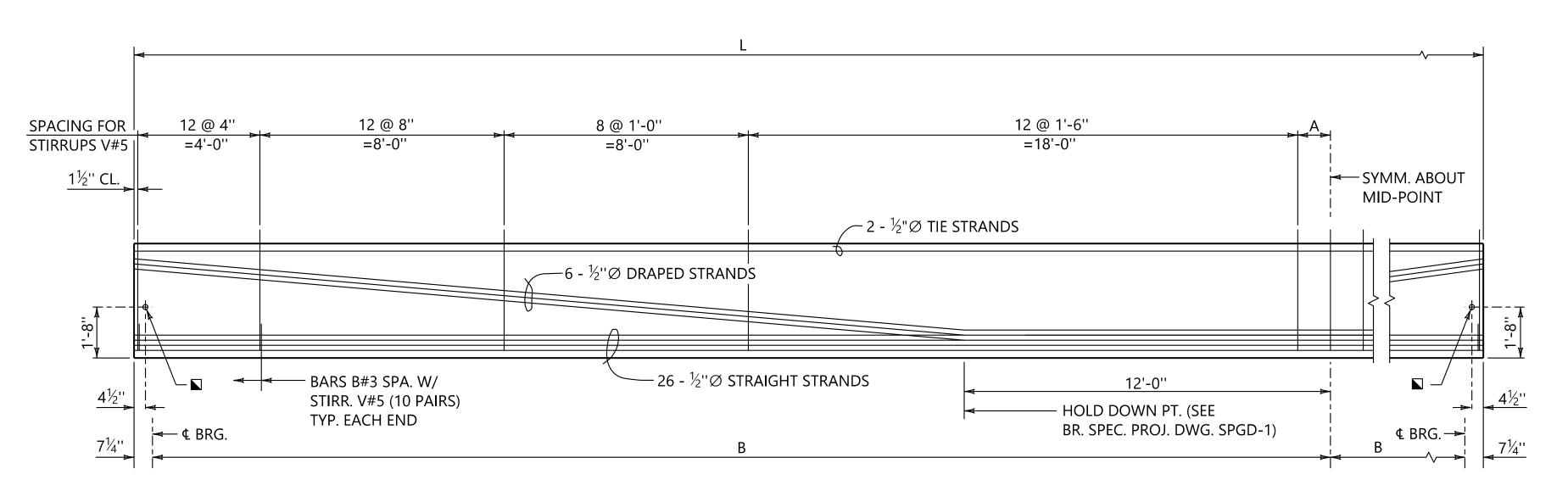
S2460(S)

SHEET 2 OF 2

CONTRACT

COUNTY(S)





*TYPICAL GIRDER ELEVATION SCALE: ³/₈'' = 1'-0''

▲ 2 STRAIGHT ½"Ø PRESTRESSED STRANDS WITH INITIAL TENSION OF 5,000 LBS. PER STRAND. STIRRUPS V#5 SHALL BE TIED IN PLACE TO THESE STRANDS.

1'-4''

1'-10''

1'-6''

BARS B#3

■ $1\frac{1}{2}$ "Ø HOLE (GDRS. 2 & 3) OR 1"Ø THREADED INSERT (INSIDE FACE ONLY, GDRS. 1 & 4)

★ DIMENSIONS SHOWN ARE ALONG & GIRDER

NOTES

- 1. PRESTRESSING STRANDS SHALL BE ½" DIAMETER 270,000 PSI LOW RELAXTION WITH AN INITIAL TENSION OF 30,983 LBS./STRAND UNLESS OTHERWISE NOTED.
- 2. ALL STRANDS NOT TO BE ENCASED IN CONCRETE SHALL BE CUT FLUSH AT EACH END OF THE GIRDER. COAT GIRDER ENDS WHERE STRANDS ARE CUT WITH AN APPROVED EPOXY COATING. STRANDS TO BE ENCASED IN CONCRETE MAY EXTEND 2" FROM THE END OF THE
- . THE GIRDER CONCRETE SHALL HAVE A MINIMUM OF 5,000 PSI COMPRESSIVE STRENGTH PRIOR TO RECEIVING PRESTRESSING FORCE AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5,500 PSI.
- 4. THREADED BARS R2 AND THREADED INSERTS SHALL BE INCLUDED IN THE BID ITEM 513B, PRETENSIONED-PRESTRESSED CONCRETE GIRDERS, TYPE III.
- 5. GIRDER ENDS SHALL BE VERTICAL IN FINAL ERECTED POSITION.
- 6. UNLESS OTHERWISE SHOWN, STIRRUPS AND CONFINEMENT STEEL SHALL BE SECURELY TIED TO THE PRESTRESSING STRANDS TO PROVIDE A MINIMUM OF 1" CONCRETE COVER.
- 7. CONNECTION ANGLES ARE REQUIRED ON BOTH FACES OF ALL GIRDERS AT THE FIXED END AND BOTH FACES OF THE EXTERIOR GIRDERS ONLY AT THE EXPANSION END. SEE BRIDGE SPECIAL PROJECT DWG. SPGD-1 FOR DETAILS.
- 8. THE ENGINEER WILL CONSIDER ALTERNATE GIRDER REINFORCING UTILIZING WELDED WIRE FABRIC IN LIEU OF TIED REINFORCING FOR BARS B. THE EQUIVALENT AREA OF STEEL AND SPACING OF BARS SHALL BE MAINTAINED.

	END SPANS	INT. SPANS
L	78'-4¾''	79'-2½''
В	77'-21/4''	78'-0''
Α	1'-07/8''	1'-5 ³ ⁄4''

2 @ 2''--– STIRR. V#5 — STIRR. V#5 BAR B#3 (E.F.) — -<u>••••</u> 9 @ 2"

SECTION @ **AASHTO TYPE III GIRDER** END OF GIRDER SCALE: $\frac{3}{4}$ " = 1'-0" SCALE: $\frac{3}{4}$ " = 1'-0"

STIRRUPS V#5

NC THREAD 3"

2'-3''

BARS R2#8

[™]¾'' CHAMFER

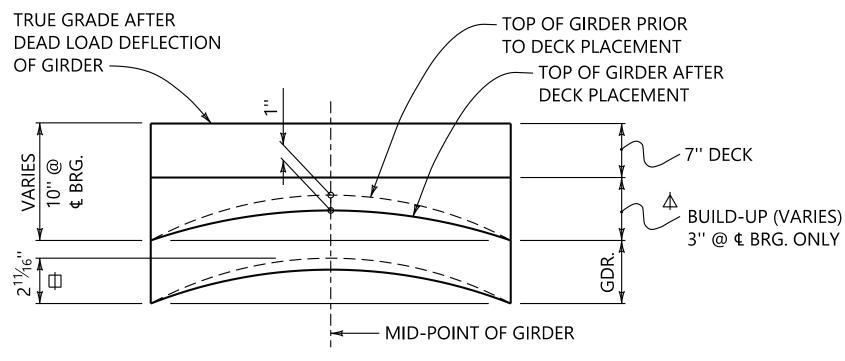
⋖ | STIRR. V#5 +<u>••••</u> m 2" 9 @ 2" 2" SECTION BETWEEN **SECTION BETWEEN**

☐ THEORETICAL CAMBER (UPWARD DEFLECTION) SHOWN. ACTUAL CAMBER OF GIRDER MAY VARY AND SHOULD BE DETERMINED BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS AND SETTING FORMS.

HOLD DOWN POINTS

SCALE: ³/₄'' = 1'-0''

△ ADJUSTMENT TO BUILD-UP MAY BE REQUIRED IF USED FOR BRIDGES IN EXTREME CREST OR SAG VERTICAL CURVE GRADES.



DETAIL OF BUILD-UP BETWEEN BOTTOM OF DECK AND TOP OF GDR. (ALONG | GDR.)

NTS

ALABAMA DEPARTMENT	
OF TRANSPORTATION	IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII

REVISIONS PROJECT NO.

END & HOLD DOWN

SCALE: $\frac{3}{4}$ " = 1'-0"

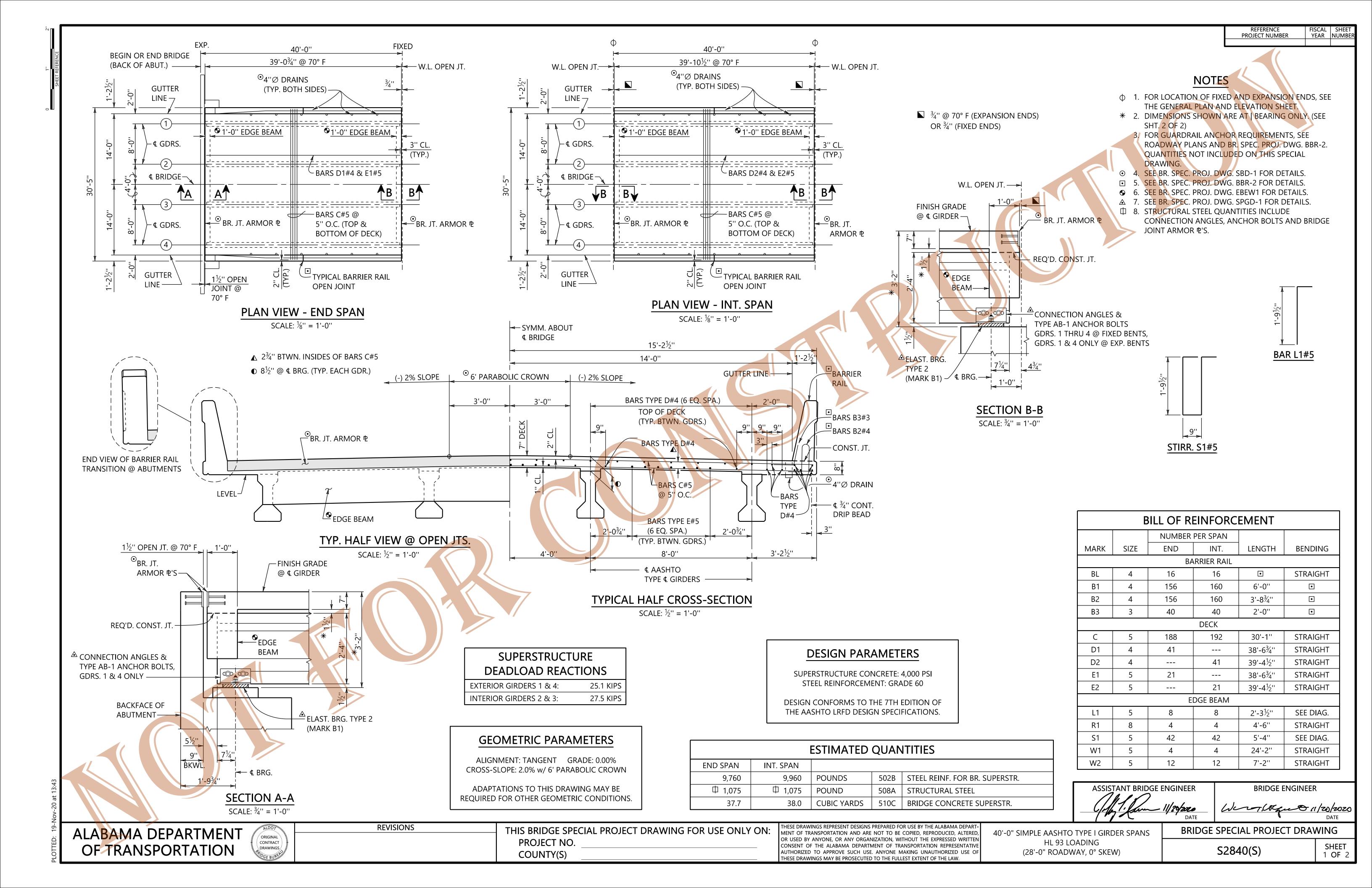
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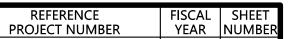
80'-0" SIMPLE AASHTO TYPE III GIRDER SPANS HL 93 LOADING (24'-0" ROADWAY, 0° SKEW)

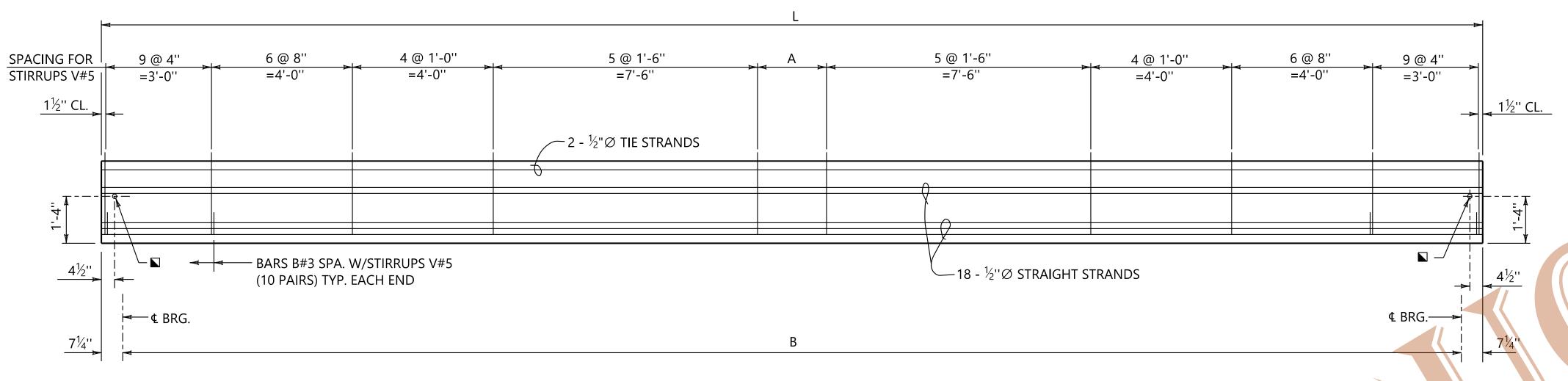
BRIDGE SPECIAL PROJECT DRAWING

ORIGINAL CONTRACT DRAWINGS.

THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: COUNTY(S)







*TYPICAL GIRDER ELEVATION

SCALE: ½'' = 1'-0''

- $1\frac{1}{2}$ "Ø HOLE (GDRS. 2 & 3) OR 1"Ø THREADED INSERT (INSIDE FACE ONLY, GDRS. 1 & 4)
- ***** DIMENSIONS SHOWN ARE ALONG **€** GIRDER

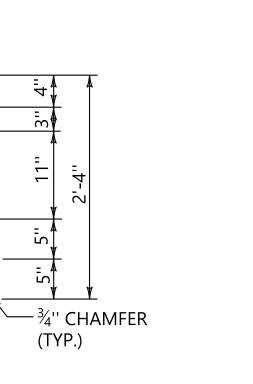
WITH INITIAL TENSION OF 5,000 LBS. PER STRAND. STIRRUPS V#5 SHALL BE TIED IN PLACE TO THESE STRANDS.

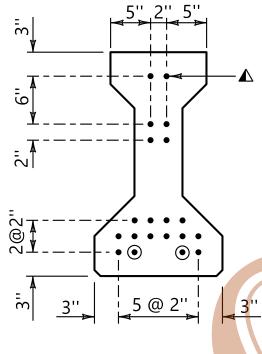
6''

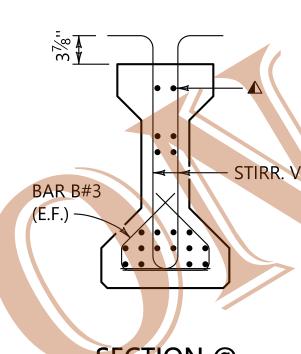
AASHTO TYPE I GIRDER

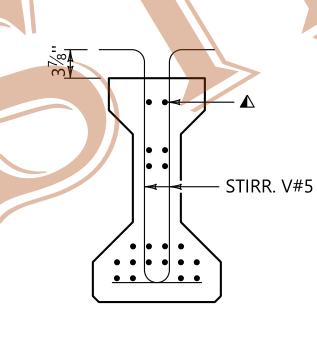
SCALE: 1'' = 1'-0''

▲ 2 STRAIGHT ½"Ø PRESTRESSED STRANDS





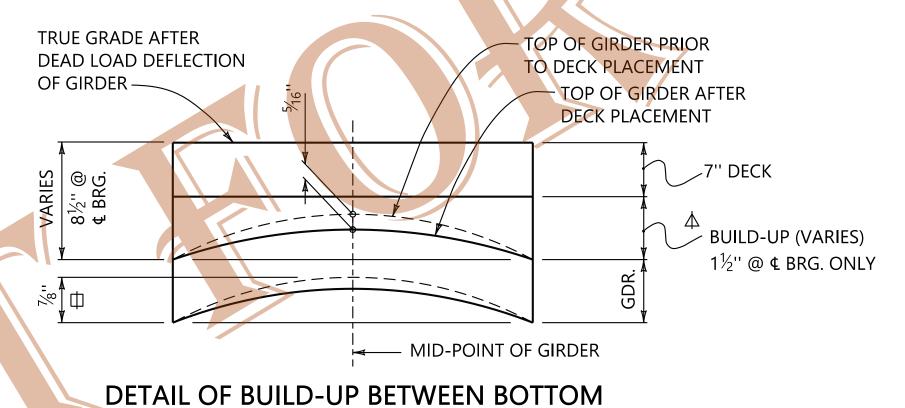




STRAND PATTERN SCALE: 1" = 1'-0"

SECTION @ END OF GIRDER SCALE: 1'' = 1'-0''

SECTION ALONG GIRDER SCALE: 1" = 1'-0"



OF DECK AND TOP OF GDR. (ALONG & GDR.)

NO SCALE

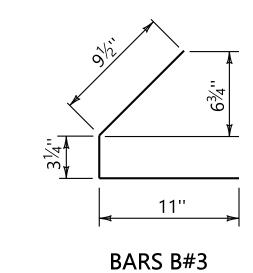
- THEORETICAL CAMBER (UPWARD DEFLECTION) SHOWN. ACTUAL CAMBER OF GIRDER MAY VARY AND SHOULD BE DETERMINED BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS AND SETTING FORMS.
- ADJUSTMENT TO BUILD-UP MAY BE REQUIRED IF USED FOR BRIDGES IN EXTREME CREST OR SAG VERTICAL CURVE GRADES.

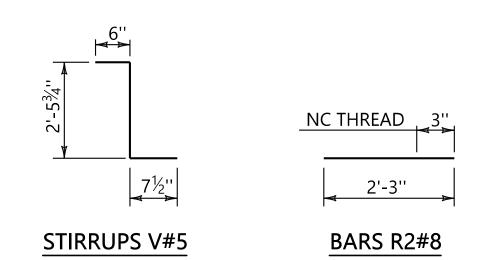
NOTES

- 1. PRESTRESSING STRANDS SHALL BE ½" DIAMETER 270,000 PSI LOW RELAXTION WITH AN INITIAL TENSION OF 30,983 LBS./STRAND UNLESS OTHERWISE NOTED.
- 2. ALL STRANDS NOT TO BE ENCASED IN CONCRETE SHALL BE CUT FLUSH AT EACH END OF THE GIRDER. COAT GIRDER ENDS WHERE STRANDS ARE CUT WITH AN APPROVED EPOXY COATING. STRANDS TO BE ENCASED IN CONCRETE MAY EXTEND 2" FROM THE END OF THE GIRDER.
- 3. THE GIRDER CONCRETE SHALL HAVE A MINIMUM OF 5,000 PSI COMPRESSIVE STRENGTH PRIOR TO RECEIVING PRESTRESSING FORCE AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 5,500 PSI.
- 4. THREADED BARS R2 AND THREADED INSERTS SHALL BE INCLUDED IN THE BID ITEM 513B, PRETENSIONED-PRESTRESSED CONCRETE GIRDERS, TYPE I.
- GIRDER ENDS SHALL BE VERTICAL IN FINAL ERECTED POSITION.
- 6. STRANDS SHOWN THUS

 SHALL REMAIN UNBONDED BY USING PLASTIC SHEATHES AROUND CABLES FOR A DISTANCE OF 4'-0" FROM THE ENDS OF THE GIRDER.
- UNLESS OTHERWISE SHOWN, STIRRUPS AND CONFINEMENT STEEL SHALL BE SECURELY TIED TO THE PRESTRESSING STRANDS TO PROVIDE A MINIMUM OF 1" CONCRETE COVER.
- 8. CONNECTION ANGLES ARE REQUIRED ON BOTH FACES OF ALL GIRDERS AT THE FIXED END AND BOTH FACES OF THE EXTERIOR GIRDERS ONLY AT THE EXPANSION END. SEE BRIDGE SPECIAL PROJECT DWG. SPGD-1 FOR DETAILS.
- 9. THE ENGINEER WILL CONSIDER ALTERNATE GIRDER REINFORCING UTILIZING WELDED WIRE FABRIC IN LIEU OF TIED REINFORCING FOR BARS B. THE EQUIVALENT AREA OF STEEL AND SPACING OF BARS SHALL BE MAINTAINED.

	END SPANS	INT. SPANS
L	38'-4¾''	39'-2½''
В	37'-2¼''	38'-0''
A	1'-1¾''	2 SPA. @ 11¾''





ALABAMA DEPARTMENT **OF TRANSPORTATION**

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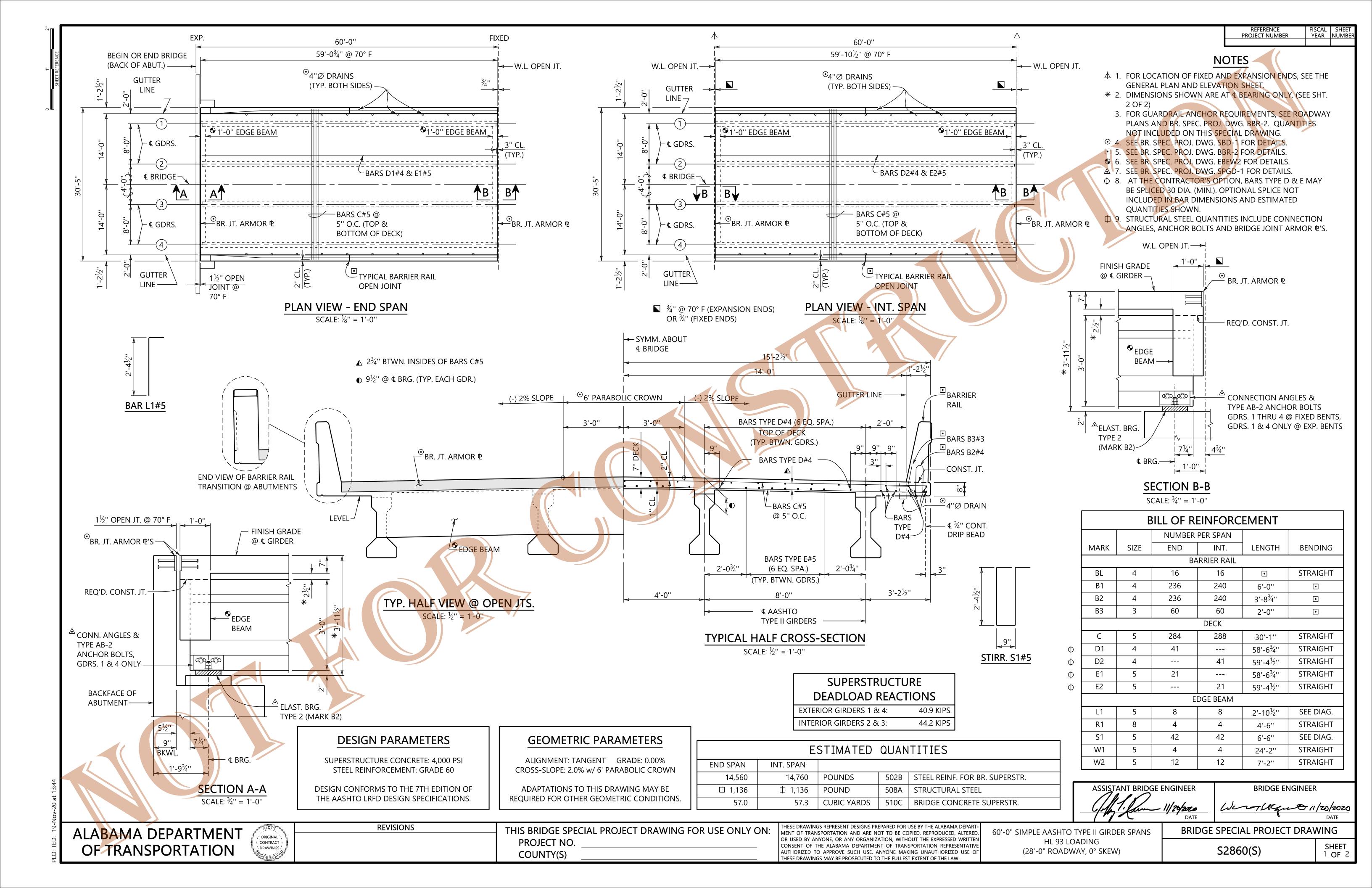
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40'-0" SIMPLE AASHTO TYPE I GIRDER SPANS HL 93 LOADING (28'-0" ROADWAY, 0° SKEW)

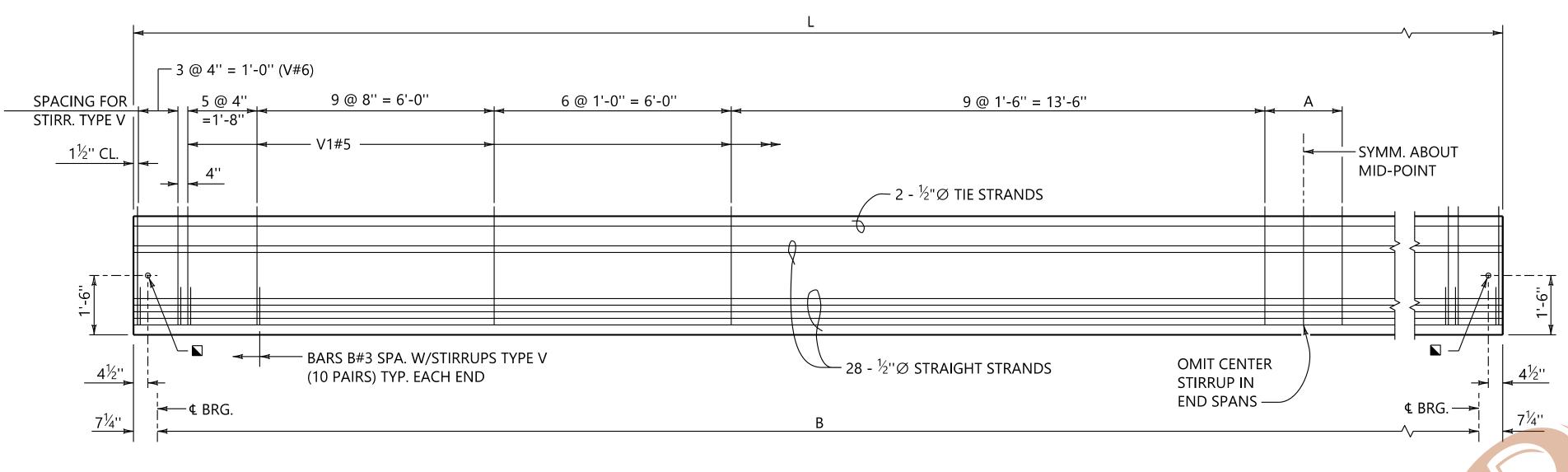
BRIDGE SPECIAL PROJECT DRAWING

SHEET 2 OF 2 S2840(S)

COUNTY(S)

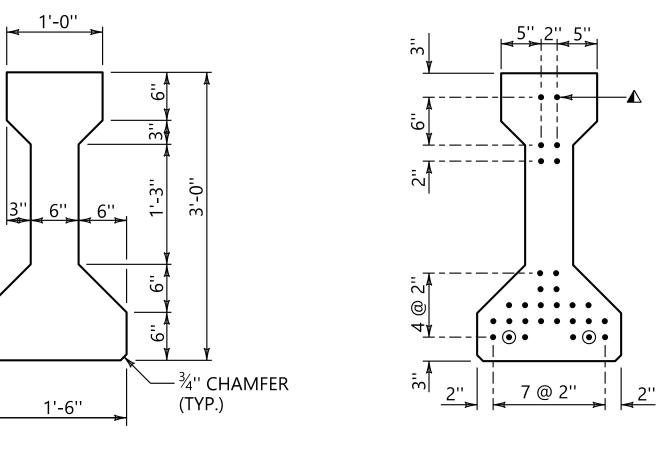


- 1. PRESTRESSING STRANDS SHALL BE 1/2" DIAMETER 270,000 PSI LOW RELAXTION WITH AN INITIAL TENSION OF 30,983 LBS./STRAND UNLESS OTHERWISE NOTED.
- 2. ALL STRANDS NOT TO BE ENCASED IN CONCRETE SHALL BE CUT FLUSH AT EACH END OF THE GIRDER. COAT GIRDER ENDS WHERE STRANDS ARE CUT WITH AN APPROVED EPOXY COATING. STRANDS TO BE ENCASED IN CONCRETE MAY EXTEND 2" FROM THE END OF THE
- 3. THE GIRDER CONCRETE SHALL HAVE A MINIMUM OF 6,500 PSI COMPRESSIVE STRENGTH PRIOR TO RECEIVING PRESTRESSING FORCE AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 7,000 PSI.
- 4. THREADED BARS R2 AND THREADED INSERTS SHALL BE INCLUDED IN THE BID ITEM 513B, PRETENSIONED-PRESTRESSED CONCRETE GIRDERS, TYPE II.
- 5. GIRDER ENDS SHALL BE VERTICAL IN FINAL ERECTED POSITION.
- 6. STRANDS SHOWN THUS ⊙ 2 SHALL REMAIN UNBONDED BY USING PLASTIC SHEATHES AROUND CABLES FOR A DISTANCE OF 6'-0" FROM THE ENDS OF THE GIRDER.
- 7. UNLESS OTHERWISE SHOWN, STIRRUPS AND CONFINEMENT STEEL SHALL BE SECURELY TIED TO THE PRESTRESSING STRANDS TO PROVIDE A MINIMUM OF 1" CONCRETE COVER.
- 8. CONNECTION ANGLES ARE REQUIRED ON BOTH FACES OF ALL GIRDERS AT THE FIXED END AND BOTH FACES OF THE EXTERIOR GIRDERS ONLY AT THE EXPANSION END. SEE BRIDGE SPECIAL PROJECT DWG. SPGD-1 FOR DETAILS.
- 9. THE ENGINEER WILL CONSIDER ALTERNATE GIRDER REINFORCING UTILIZING WELDED WIRE FABRIC IN LIEU OF TIED REINFORCING FOR BARS B. THE EQUIVALENT AREA OF STEEL AND SPACING OF BARS SHALL BE MAINTAINED.



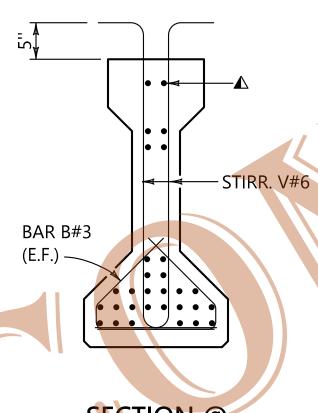
* TYPICAL GIRDER ELEVATION

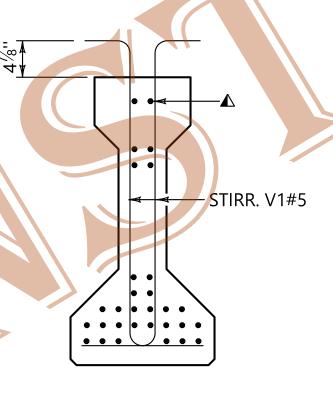
SCALE: $\frac{1}{2}$ " = 1'-0"



MID-POINT OF GIRDER







END SPANS INT. SPANS 58'-4³⁄₄'' 59'-2½'' 57'-2¹⁄₄'' 58'-0'' 1'-1³⁄4'' 2 SPA. @ 11³/₄''

AASHTO TYPE II GIRDER SCALE: 1'' = 1'-0''

DETAIL OF BUILD-UP BETWEEN BOTTOM

OF DECK AND TOP OF GDR. (ALONG & GDR.)

NTS

STRAND PATTERN SCALE: 1" = 1'-0"

BUILD-UP (VARIES)

2½" @ **\$** BRG. ONLY

OP OF GIRDER PRIOR

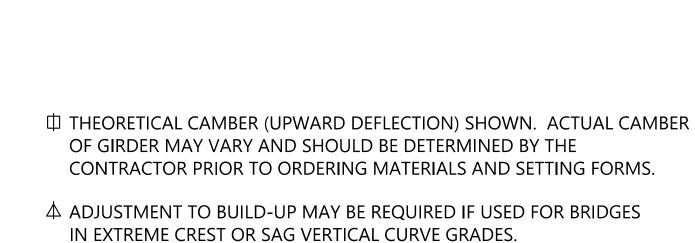
DECK PLACEMENT

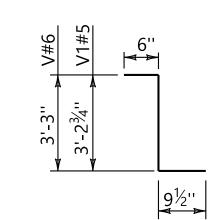
TOP OF GIRDER AFTER

TO DECK PLACEMENT

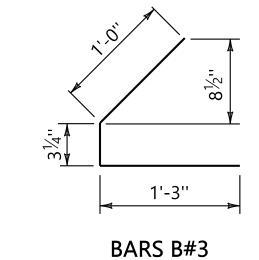
SECTION @ END OF GIRDER SCALE: 1" = 1'-0"

SECTION ALONG GIRDER SCALE: 1'' = 1'-0''





STIRRUPS TYPE V



NC THREAD 2'-3''

■ 1½"Ø HOLE (GDRS. 2 & 3) OR 1"Ø THREADED INSERT (INSIDE FACE ONLY, GDRS. 1 & 4)

* DIMENSIONS SHOWN ARE ALONG & GIRDER

▲ 2 STRAIGHT ½"Ø PRESTRESSED STRANDS WITH INITIAL TENSION OF 5,000 LBS. PER STRAND. STIRRUPS TYPE V SHALL BE TIED

IN PLACE TO THESE STRANDS.

BARS R2#8

ALABAMA DEPARTMENT **OF TRANSPORTATION**

TRUE GRADE AFTER

OF GIRDER —

DEAD LOAD DEFLECTION

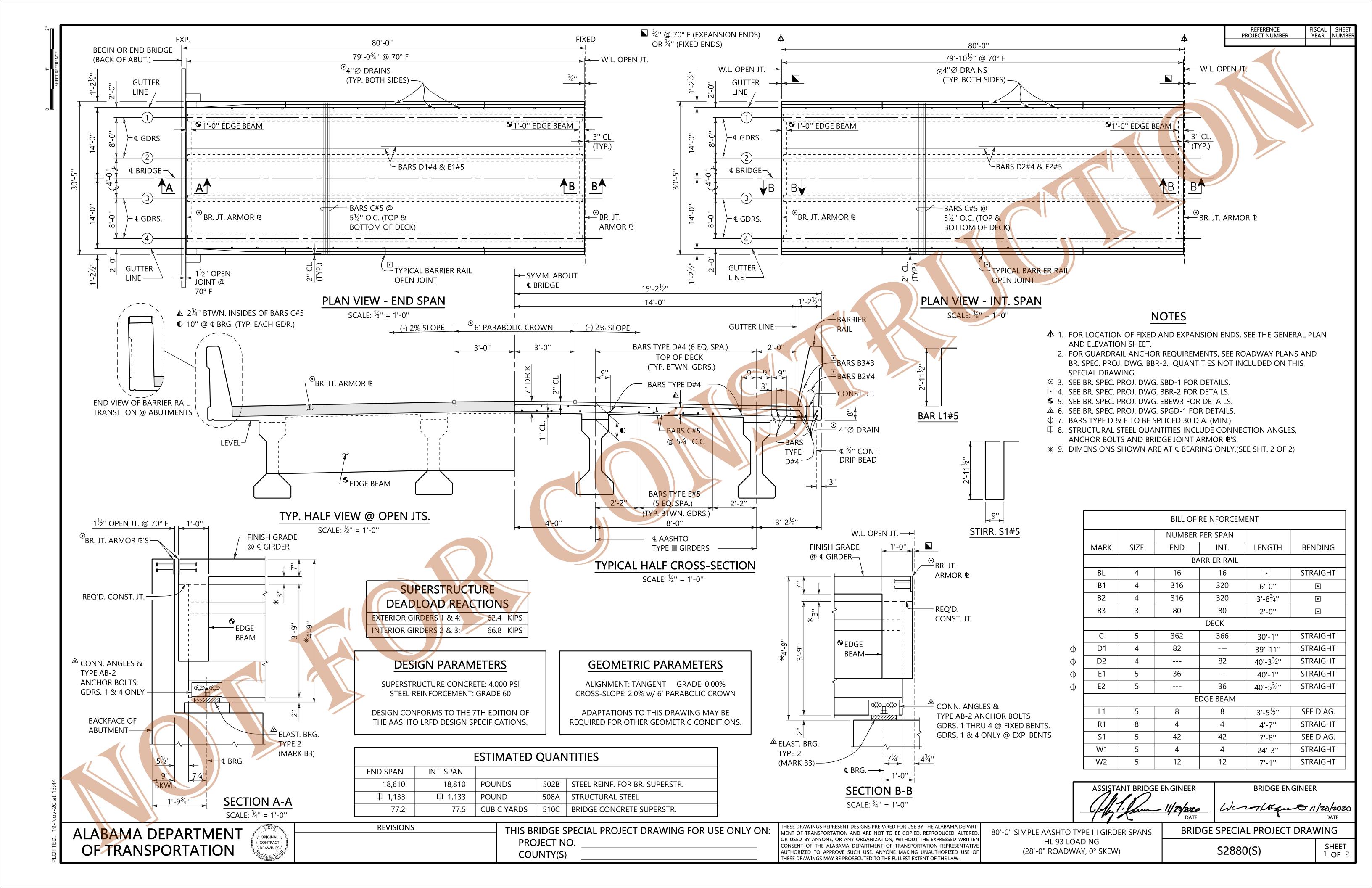
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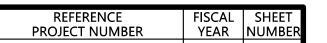
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60'-0" SIMPLE AASHTO TYPE II GIRDER SPANS HL 93 LOADING (28'-0" ROADWAY, 0° SKEW)

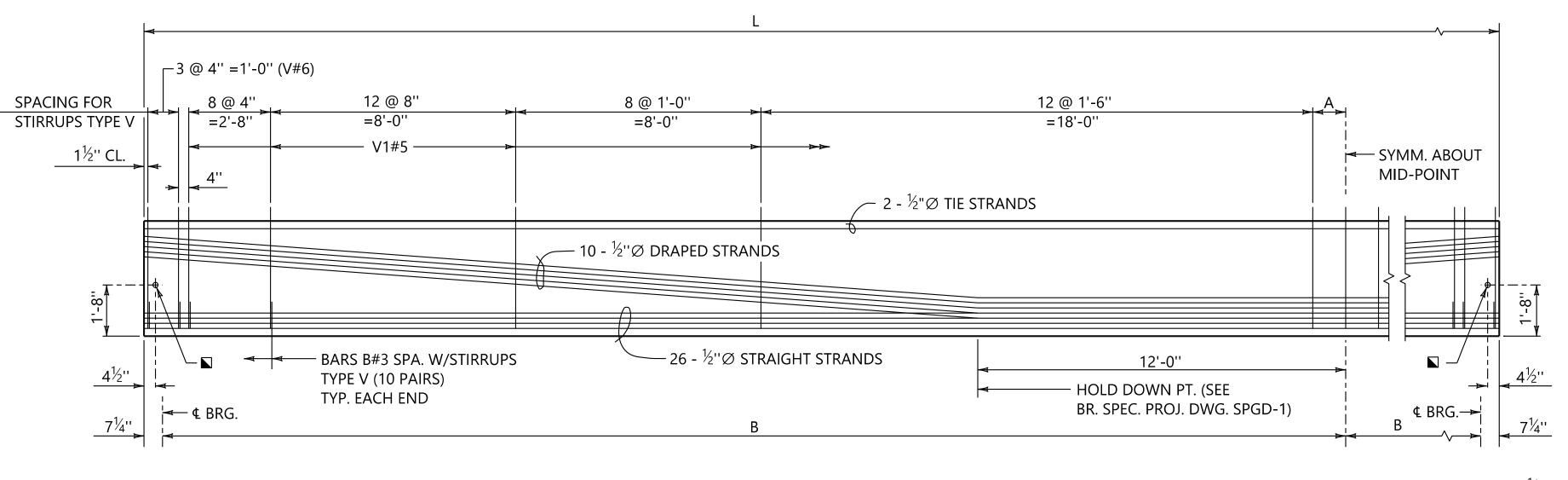
BRIDGE SPECIAL PROJECT DRAWING

SHEET 2 OF 2





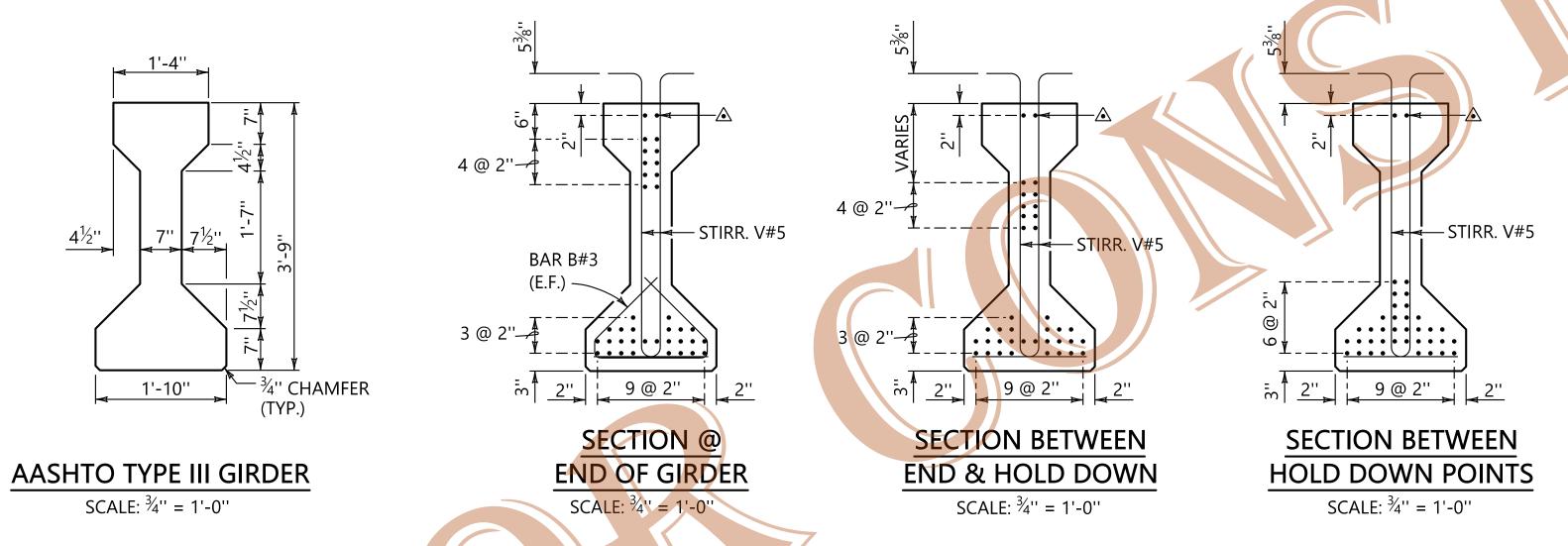
- 1. PRESTRESSING STRANDS SHALL BE $\frac{1}{2}$ " DIAMETER 270,000 PSI LOW RELAXTION WITH AN INITIAL TENSION OF 30,983 LBS./STRAND UNLESS OTHERWISE NOTED.
- 2. ALL STRANDS NOT TO BE ENCASED IN CONCRETE SHALL BE CUT FLUSH AT EACH END OF THE GIRDER. COAT GIRDER ENDS WHERE STRANDS ARE CUT WITH AN APPROVED EPOXY COATING. STRANDS TO BE ENCASED IN CONCRETE MAY EXTEND 2" FROM THE END OF THE GIRDER.
- 3. THE GIRDER CONCRETE SHALL HAVE A MINIMUM OF 5,500 PSI COMPRESSIVE STRENGTH PRIOR TO RECEIVING PRESTRESSING FORCE AND A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6,000 PSI.
- 4. THREADED BARS R2 AND THREADED INSERTS SHALL BE INCLUDED IN THE BID ITEM 513B, PRETENSIONED-PRESTRESSED CONCRETE GIRDERS, TYPE III.
- 5. GIRDER ENDS SHALL BE VERTICAL IN FINAL ERECTED POSITION.
- 6. UNLESS OTHERWISE SHOWN, STIRRUPS AND CONFINEMENT STEEL SHALL BE SECURELY TIED TO THE PRESTRESSING STRANDS TO PROVIDE A MINIMUM OF 1" CONCRETE COVER.
- 7. CONNECTION ANGLES ARE REQUIRED ON BOTH FACES OF ALL GIRDERS AT THE FIXED END AND BOTH FACES OF THE EXTERIOR GIRDERS ONLY AT THE EXPANSION END. SEE BRIDGE SPECIAL PROJECT DWG. SPGD-1 FOR DETAILS.
- 8. THE ENGINEER WILL CONSIDER ALTERNATE GIRDER REINFORCING UTILIZING WELDED WIRE FABRIC IN LIEU OF TIED REINFORCING FOR BARS B. THE EQUIVALENT AREA OF STEEL AND SPACING OF BARS SHALL BE MAINTAINED.



*TYPICAL GIRDER ELEVATION SCALE: $\frac{3}{8}$ " = 1'-0"

▲ 2 STRAIGHT ½"Ø PRESTRESSED STRANDS WITH INITIAL TENSION OF 5,000 LBS. PER STRAND. STIRRUPS TYPE V SHALL BE TIED IN PLACE TO THESE STRANDS.

- \blacksquare 1½"Ø HOLE (GDRS. 2 & 3) OR 1"Ø THREADED INSERT (INSIDE FACE ONLY, GDRS. 1 & 4)
- ***** DIMENSIONS SHOWN ARE ALONG **⊈** GIRDER



TOP OF GIRDER PRIOR

TO DECK PLACEMENT

TOP OF GIRDER AFTER

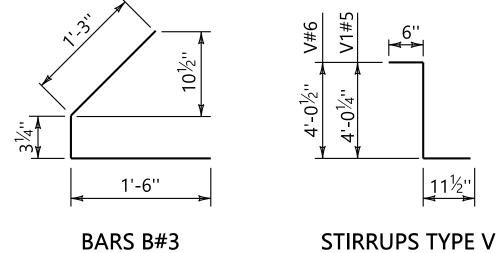
√ 7" DECK

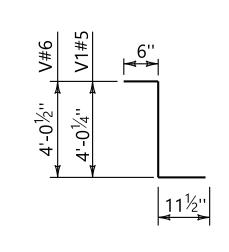
BUILD-UP (VARIES)

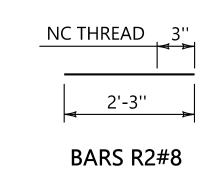
3" @ & BRG. ONLY

DECK PLACEMENT

	END SPANS	INT. SPANS
L	78'-4 ³ ⁄ ₄ ''	79'-2½''
В	77'-21/4''	78'-0''
Α	1'-0 ⁷ %''	1'-5 ³ ⁄ ₄ ''







THEORETICAL CAMBER (UPWARD DEFLECTION) SHOWN. ACTUAL CAMBER OF GIRDER MAY VARY AND SHOULD BE DETERMINED BY THE CONTRACTOR PRIOR TO ORDERING MATERIALS AND SETTING FORMS.

△ ADJUSTMENT TO BUILD-UP MAY BE REQUIRED IF USED FOR BRIDGES IN EXTREME CREST OR SAG VERTICAL CURVE GRADES.

DETAIL OF BUILD-UP BETWEEN BOTTOM OF DECK AND TOP OF GDR. (ALONG & GDR.) NTS

MID-POINT OF GIRDER

REVISIONS

TRUE GRADE AFTER

OF GIRDER -

DEAD LOAD DEFLECTION

ALABAMA DEPARTMENT CONTRACT **OF TRANSPORTATION**

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MENT OF TRANSPORTATION AND ARE NOT TO BE COPIED, REPRODUCED, ALTERED, OR USED BY ANYONE, OR ANY ORGANIZATION, WITHOUT THE EXPRESSED WRITTEN CONSENT OF THE ALABAMA DEPARTMENT OF TRANSPORTATION REPRESENTATIVE AUTHORIZED TO APPROVE SUCH USE. ANYONE MAKING UNAUTHORIZED USE OF HESE DRAWINGS MAY BE PROSECUTED TO THE FULLEST EXTENT OF THE LAW.

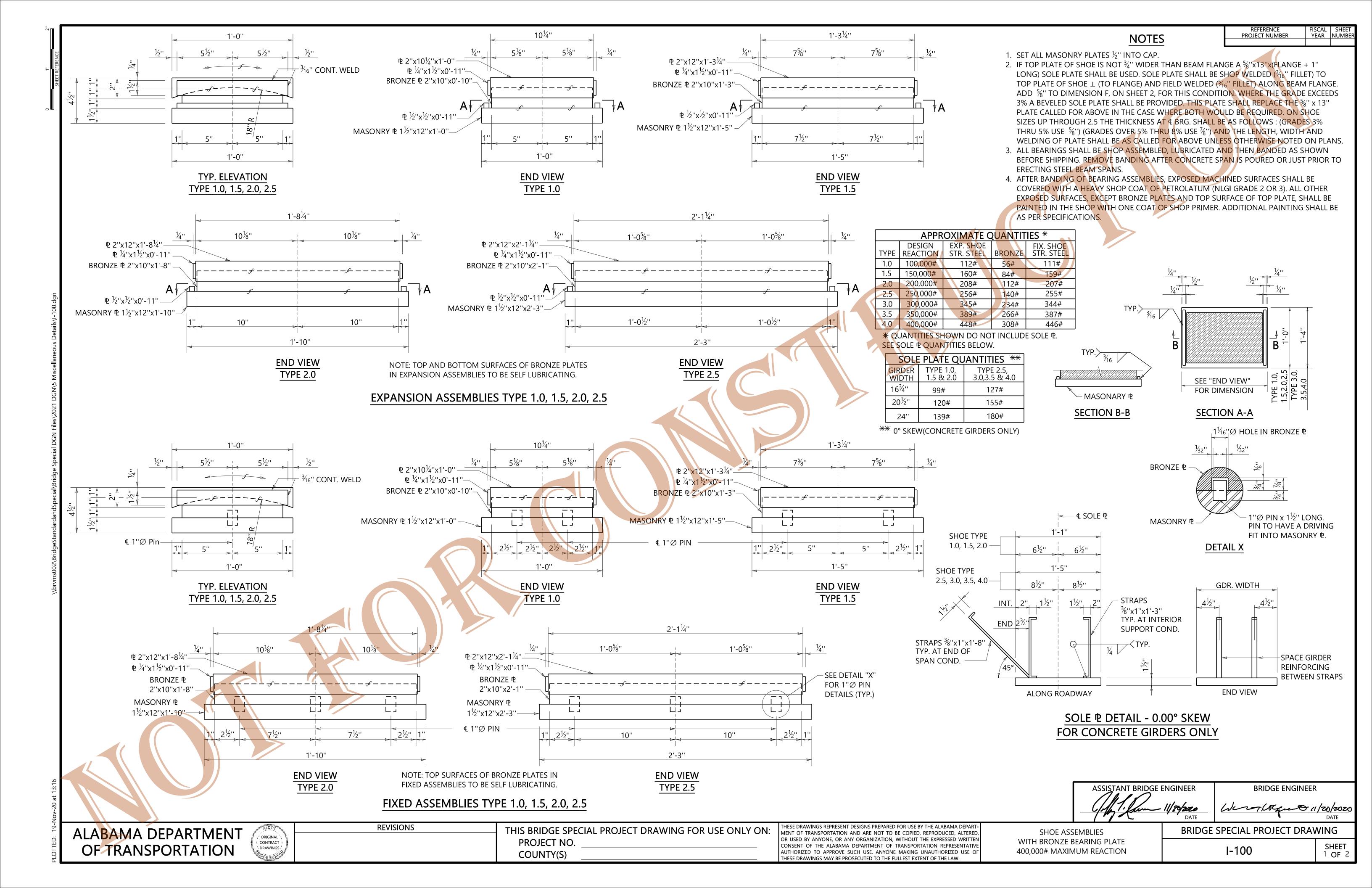
80'-0" SIMPLE AASHTO TYPE III GIRDER SPANS HL 93 LOADING (28'-0" ROADWAY, 0° SKEW)

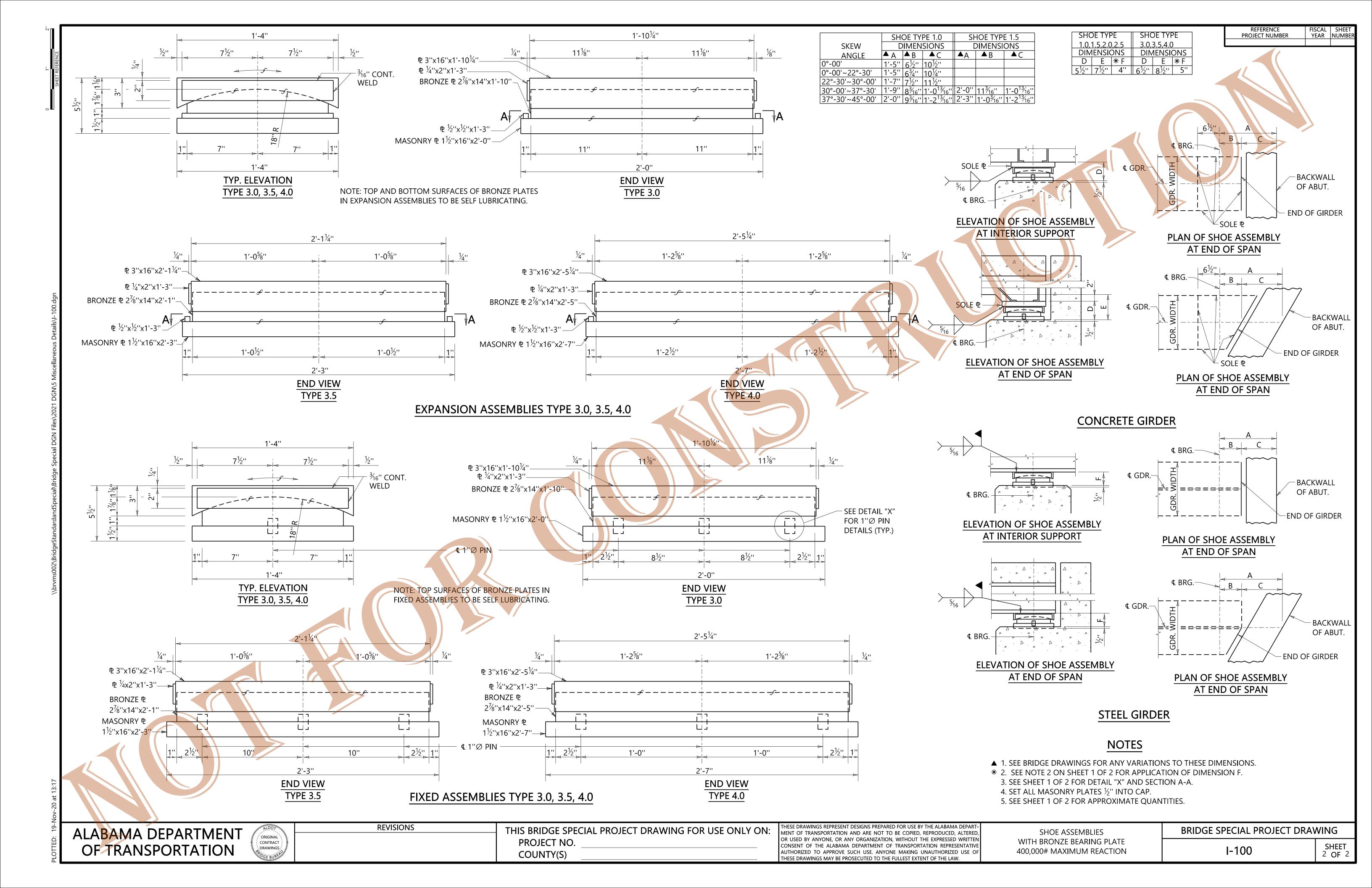
BRIDGE SPECIAL PROJECT DRAWING

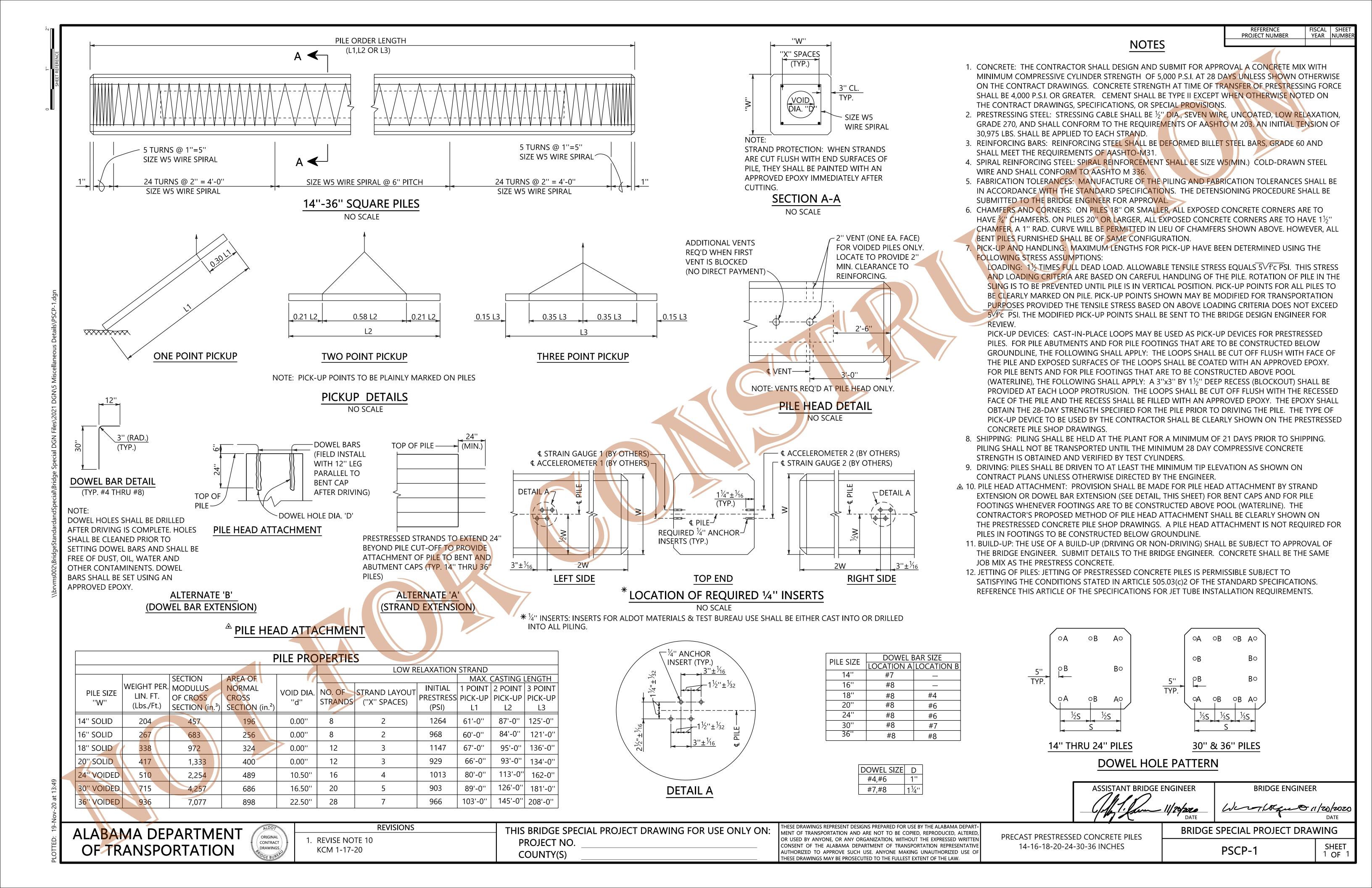
S2880(S)

SHEET 2 OF 2

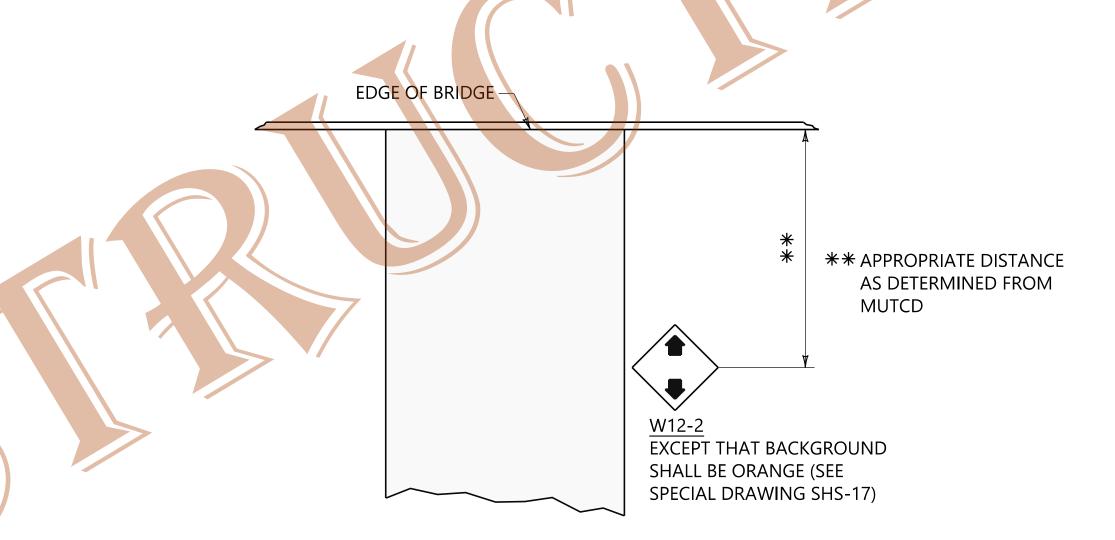
COUNTY(S)







- 1. TRAFFIC PROTECTION SHALL BE TYPE 1 OR 2 AS APPROPRIATE. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH STANDARD DRAWING BGN-1 NOTE NO. 13 OR BRIDGE SPECIAL PROJECT DRAWING SBN-1 NOTE NO. 13.
- 2. CLEARANCE FOR TRAFFIC PROTECTORS: THE CONTRACTOR WILL BE REQUIRED TO MAINTAIN THE CLEARANCES SUBMITTED UNDER REQUIREMENT OF STANDARD DRAWING BGN-1 NOTE NO. 13 OR BRIDGE SPECIAL PROJECT DRAWING SBN-1 NOTE NO. 13 DURING CONSTRUCTION.
- 3. TRAFFIC PROTECTORS SHALL BE ERECTED AHEAD OF THE FORMWORK FOR THE SUPERSTRUCTURE AND SHALL REMAIN IN PLACE UNTIL ALL POURING OVER THE PROTECTION HAS BEEN COMPLETED. FLAGMEN SHALL BE USED AS NEEDED TO PROTECT TRAFFIC DURING ERECTION AND REMOVAL OF THE PROTECTORS. FLAGMEN SHALL ALSO BE USED AS NEEDED DURING FINISHING AND PAINTING OPERATIONS OVER THE ROADWAY.

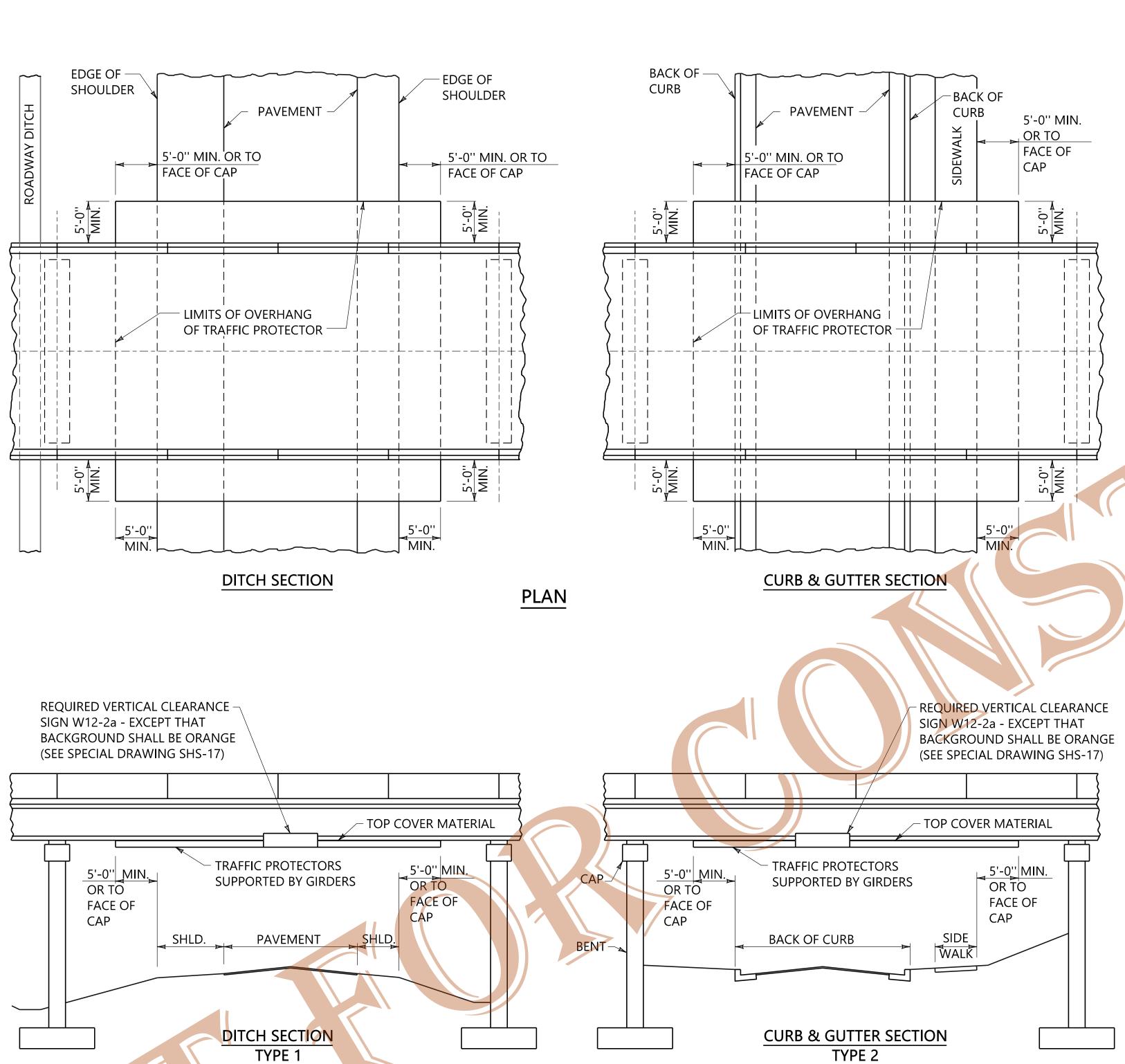


SIGN LOCATION DETAIL

NOTE: VERTICAL CLEARANCE SIGNS SHOWN AT LEFT SHALL BE INSTALLED IN ADDITION TO THE NORMAL REQUIRED ADVANCE WARNING SIGNS WHEN THE VERTICAL CLEARANCE IS LESS THAN 17'-0".

LEGEND

NO.'s & LETTERS----BLACK (NON-REFL.) BACKGROUND-----ORANGE (REFL.)



TRAFFIC PROTECTION ELEVATION

ASSISTANT BRIDGE ENGINEER **BRIDGE ENGINEER** Wer 100 que 11/20/2020

ALABAMA DEPARTMENT OF TRANSPORTATION

REVISIONS THIS BRIDGE SPECIAL PROJECT DRAWING FOR USE ONLY ON: PROJECT NO. COUNTY(S)

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TRAFFIC PROTECTION

BRIDGE SPECIAL PROJECT DRAWING

SHEET 1 OF 1

TP-1